

LIST OF ACRONYMS AND ABBREVIATIONS

Acronym	Full Name
ADA	Americans with Disabilities Act
BFE	Base flood elevation
CDFW	California Department of Fish and Wildlife
cfs	Cubic feet per second
CLOMR	Conditional Letter of Map Revision
CPIOZ	Community Plan Implementation Overlay Zone
du/ac	Dwelling unit per acre
FEMA	Flood Emergency Management Administration
FIS	Flood Insurance Study
Grantville Station	Grantville Trolley Station
HGLs	Hydraulic grade lines
I-8	Interstate 8
LOMR	Letter of Map Revision
Master Program	2013 Master Storm Water System Maintenance Program
МНРА	Multi-Habitat Planning Area
MSCP	San Diego Multiple Species Conservation Program
MTS	Metropolitan Transit System
RCB	Reinforced concrete box
RWQCB	Regional Water Quality Control Board
SANDAG	San Diego Association of Governments
SDR	Supplemental development regulations
SFHA	Special Flood Hazard Area
SGIP	Smart Growth Incentive Program
T&SW	City of San Diego Transportation & Storm Water Department
TOD	Transit Oriented Development
USACE	U.S. Army Corps of Engineers

STUDY MATERIALS CITED

Biological Constraints Report

Prepared by RECON Environmental Inc. for the Grantville Trolley Station/Alvarado Creek Enhancement Study (Oct. 2016).

Available at the City of San Diego's Navajo Community Profile webpage: www.sandiego.gov/planning/community/profiles/navajo. The report may also be directly accessed here.

Existing Conditions Report

Prepared by Dyett & Bhatia, Rick Engineering Company, and RECON Environmental for the Grantville Trolley Station/Alvarado Creek Enhancement Study (Jan. 2017).

Available at the City of San Diego's Navajo Community Profile webpage: www.sandiego.gov/planning/community/profiles/navajo. The report may also be directly accessed here.

Feasibility Study

See Appendix C.

Hydrology Background Report

See Appendix D.

Also available at the City of San Diego's Navajo Community Profile webpage: www.sandiego.gov/planning/community/profiles/navajo.



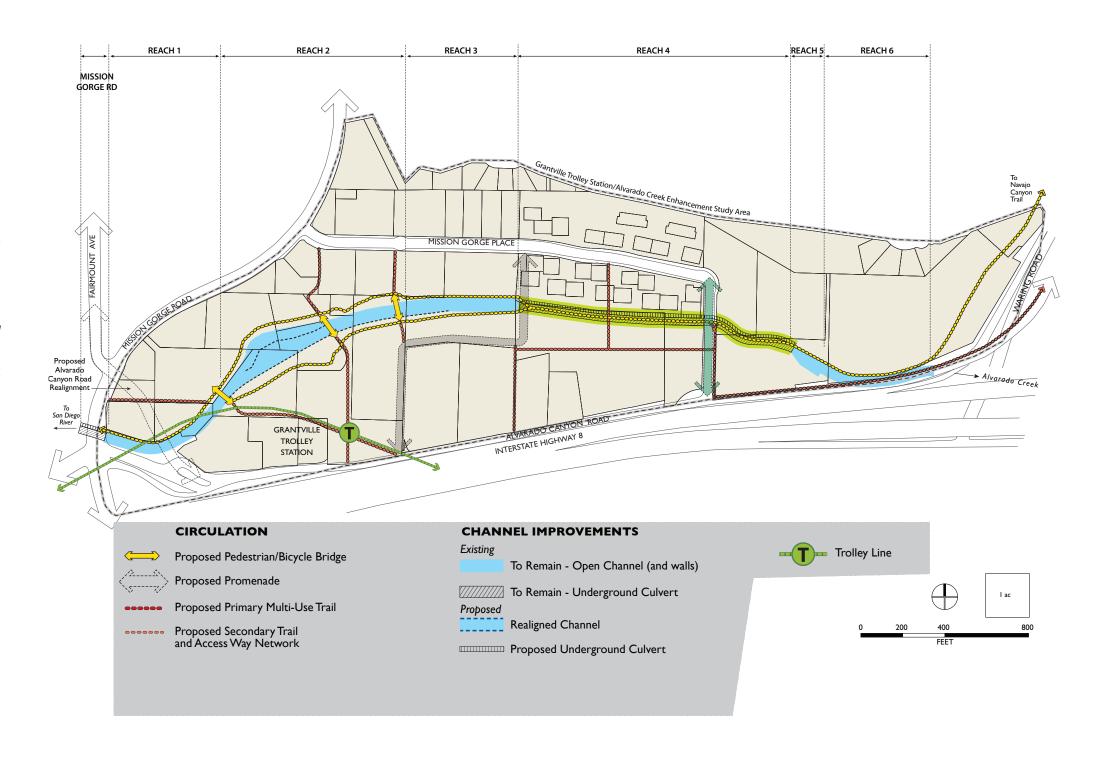
INTRODUCTION PURPOSE AND INTENT

The purpose of the following Design Palette is to provide non-binding recommendations and guidance to aid the design of the Multi-Use Trail, as well as lighting and educational signage along the Multi-Use Trail. The recommendations and guidance are intended to assist property owners in achieving the following goals:

- Safe Environment. Developing a Multi-Use
 Trail system that is safe for a wide variety of
 community members of all ages and abilities,
 in the daytime and in the evening, as well as
 during large storm events; and
- Cohesive Appearance of the Multi-Use Trail System. Developing a Multi-Use Trail system that has a cohesive look and feel throughout the Study Area.

APPLICABILITY

The Design Palette provides recommendations and guidance regarding the Multi-Use Trail system, the recommended alignment of which is shown in the adjacent figure. Specifically, the Design Palette provides non-binding guidance concerning the design of the Trail itself, lighting to illuminate the Trail, and educational signage along the Trail.



MULTI-USE TRAIL

The Multi-Use Trail should be designed to accommodate a variety of users, including walkers, joggers, and cyclists. Below are recommendations to aid the design and installation of a safe and cohesive Multi-Use Trail.

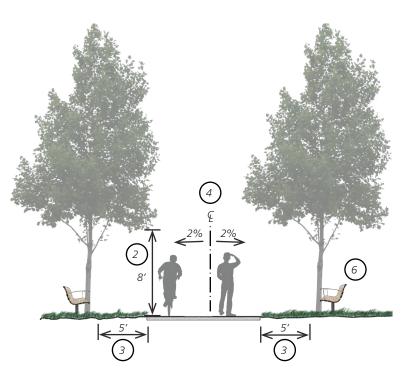
- (1) Width: The Trail should, at minimum, be 10 feet wide.
- (2) Vertical Clearance: Eight feet of vertical clearance from the finished grade of trail should be provided.
- Width Clearance: In general, a minimum of five feet of clearance should be provided from the outside edge of the trail to any object, including fences, poles, signs, benches, and the top of slope at the creek channel's edge. Site constraints such as steep slopes and proximity to the channel edge may make a five-foot width clearance infeasible; in such situations, a safe and pleasant Trail segment for users of all ages and abilities should be provided.
- 4 Cross-Slope: Two percent cross-slope should be provided. If the Trail is to be crowned for grading and drainage purposes, such as on the Recreation Corridor, the maximum and minimum slope should be two percent from the centerline of the Trail.

- Surface Material: As the Trail may be constructed, in part, in the floodplain, a hard-paved concrete surface should be used, so the Trail can withstand high velocity flows. Permeable concrete should be considered. Permeable concrete has the benefit of increasing filtration and reducing runoff of pollution into the creek; if selected, occasional sweeping and pressure washing may be required to keep the surface maintained and permeable. The selected concrete color should blend in with the Trail's surroundings and be relatively light, to minimize the urban heat island effect.
- Furnishings: The installation of furnishings adjacent to the Multi-Use Trail should be carefully considered where the Trail is located in the floodplain. The installation of fencing, benches, signage, bike racks, and trash receptacles should be designed and installed in a manner that minimizes their effect on flood waters and protects the amenities from flood damage.
- Fences: Limit fencing of the creek corridor except in those areas where safety presents a concern, or where fencing is required to protect sensitive habitat. Fencing should allow for wildlife movement. When needed, a unified fence type that is at least 75 percent open with appropriate access points can aid in making the creek a community asset.

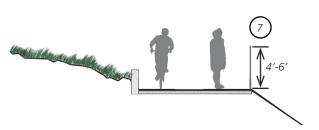
ILLUSTRATIVE SECTIONS

Typical Trail $\begin{array}{c|c}
4 \\
2\% \\
\hline
8'
\end{array}$ $\begin{array}{c|c}
10' \\
\hline
3
\end{array}$





Trail with Site Constraints



EXAMPLES

Multi-Use Trails



Trail adjacent to Guadalupe River, San Jose, CA



Floodable trail along Buffalo Bayou Park, Houston, Texas



Trail along the Los Angeles River, Studio City, CA

Concrete Colors



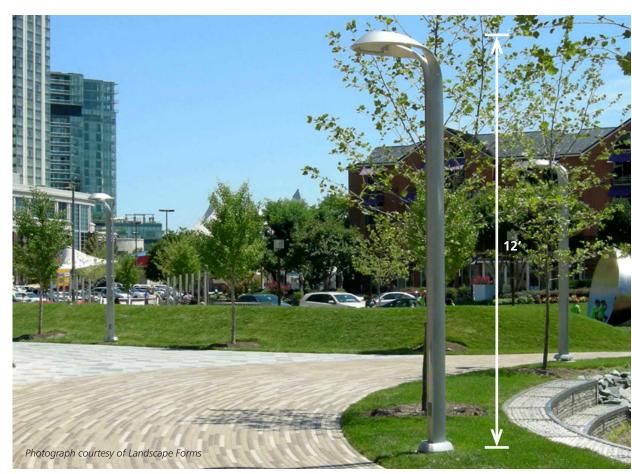




PEDESTRIAN-SCALED TRAIL LIGHTING

The overall conceptual approach to illuminating the Multi-Use Trail should balance safety and security with nighttime visibility and function through light color selection and reduction of glare. Below are recommendations to aid the design and implementation of a uniform Multi-Use Trail lighting plan.

- Color of Light Source: Light color should provide true color rendering.
- Standards and Fixtures: Fixtures should create an unobtrusive appearance that allows the focus to remain on the creek and trail, rather than the fixture. Where lighting is appropriate, it should be treated consistently throughout the Multi-Use Trail System, in terms of light source, fixture type, fixture finish, and color.
- *Energy Efficient*: Lighting should be energy efficient. Solar powered lighting should be considered as a sustainable alternative.
- *Illumination*: Illumination levels should meet the standards for Class I bike trails as defined by Caltrans Highway Design Manual.
- Pedestrian-scaled: Lighting should be pedestrian-scaled.

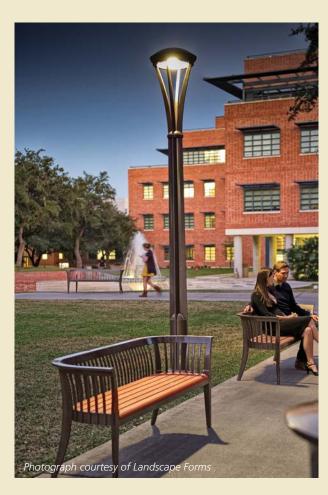


Pedestrian-scaled lighting example

EXAMPLES



One example of attractive and efficient solar-powered lighting is along D.C.'s Metropolitan Branch Trail.





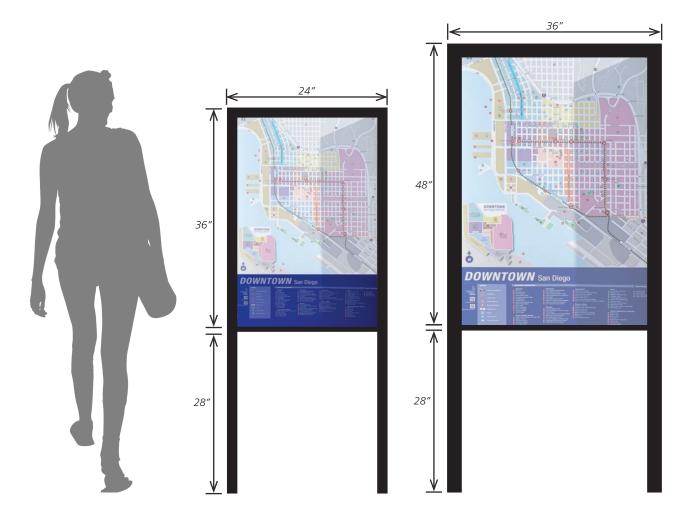


EDUCATIONAL SIGNAGE ALONG THE MULTI-USE TRAIL

The following provides guidance for implementation of a cohesive educational signage program along the Multi-Use Trail system. For further assistance, refer to the National Park Service's Wayside Exhibits: A Guide to Developing Outdoor Interpretive Exhibits.

FREESTANDING TRAILSIDE INFORMATION SIGNS

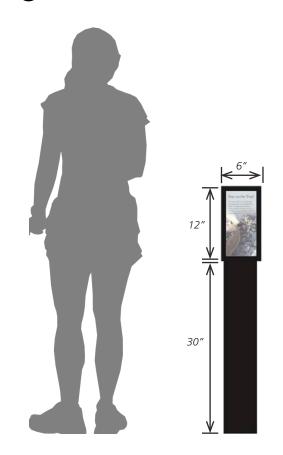
Generally, freestanding trailside information signs are placed at a visitor's decision point. They provide information to inform that decision, such as a map and particulars about what to expect along and what destinations can be accessed via the route. Maps, when used, should be simple, oriented in the direction of view, indicate where the visitor is, and help visitors get to a point B. Signs should be designed for an attention span of 30 to 45 seconds.



TRAILSIDE IDENTIFICATION SIGNS

Trailside identification signs are often used to identify a plant or provide a brief piece of helpful information. These signs generally include:

- \bigcirc A short title
- \bigcirc 25 words of text, or fewer
- © One, simple graphic



EXAMPLES

Freestanding Trailside Information Sign



Trailside Identification Signs





TABLETOP INTERPRETIVE SIGNS

Tabletop interpretive signs provide an interpretation of a significant landscape feature that visitors can readily see, and which is generally located in front of the sign. The sign should tell a simple story about what is unique about the landscape, and why visitors should care. A site-specific, bold graphic should be the primary tool for conveying the story. A 36 inch by 24 inch panel has room for the following elements:

- (A) Compelling title, used to grab the visitor's attention and interest
- \bigcirc 100 words of interpretive text, maximum
- (c) A large, bold interpretive graphic
- D Small inset graphics
- (E) Short captions or labels, which made be used to label the graphics



EXAMPLES

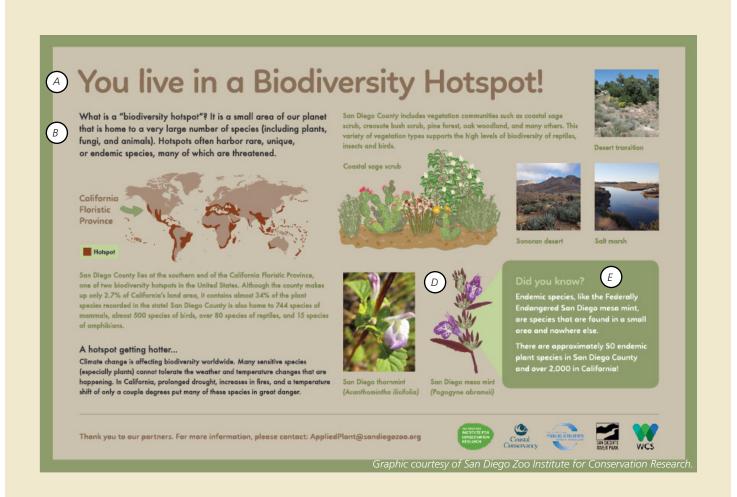
Tabletop Interpretive Signs



Bold graphics highlight the main features of the area, while also sharing additional information to engage and educate visitors.



This tabletop interpretive sign, located along the Los Angeles River, has a decorative, stone base.



GRANTVILLE TROLLEY STATION/ALVARADO CREEK REVITALIZATION STUDY

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APPENDIX C FEASIBILITY REPORT

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Introduction and Study Area Location

1.1 INTRODUCTION

The purpose of this Feasibility Report for the Grantville Trolley Station/Alvarado Creek Enhancement Study is to describe and document the process and analysis undertaken to arrive at the preferred concepts for the alignment and geometry for an enhanced Alvarado Creek channel and flood management system, route for the Alvarado Creek Multi-Use Trail system, and locations of channel crossings for bicyclists and pedestrians.

This Feasibility Report is organized as follows:

- **Chapter 1** provides an overview of the Study Area location.
- **Chapter 2** reviews the planning objectives and constraints. It also discusses the feasibility criteria used to evaluate the extent to which the preliminary, alternative revitalization strategies meet Study objectives.
- **Chapter 3** presents the preliminary, alternative revitalization strategies. Rejected alternatives are also presented.
- Chapter 4 presents the preferred design strategy and how the channel improvements and trail and crossing system were refined.

1.2 STUDY AREA LOCATION

The Study Area is located in the easterly portion of the City of San Diego, within the Grantville community in the Navajo Community Planning Area, in close proximity to Mission Valley, Allied Gardens, and San Diego State University (Figure 1-1). The Study Area is encompassed by Twain Avenue to the north, Mission Gorge Road/Fairmount Avenue to the west, Alvarado Canyon Road to the south, and Waring Road to the east. The Grantville Station and Interstate 8 (I-8) are situated in the southern portion of the Study Area (Figure 1-2).

Alvarado Creek runs through the Study Area before its confluence with the San Diego River to the west. Alvarado Creek is a low-lying area of Grantville that gradually increases in elevation in the eastern and northern portions of the Study Area. Within the Study Area, water in Alvarado Creek is conveyed by a series of open channels and box culverts that extend approximately 4,400 feet upstream from directly east of Mission Gorge Road to the southbound Waring Road onramp to Westbound I-8. For the purposes of this Study, the creek is divided into several reaches, identified as reaches 1 through 6. The location and photographic description of the reaches are presented in Figure 1-3.

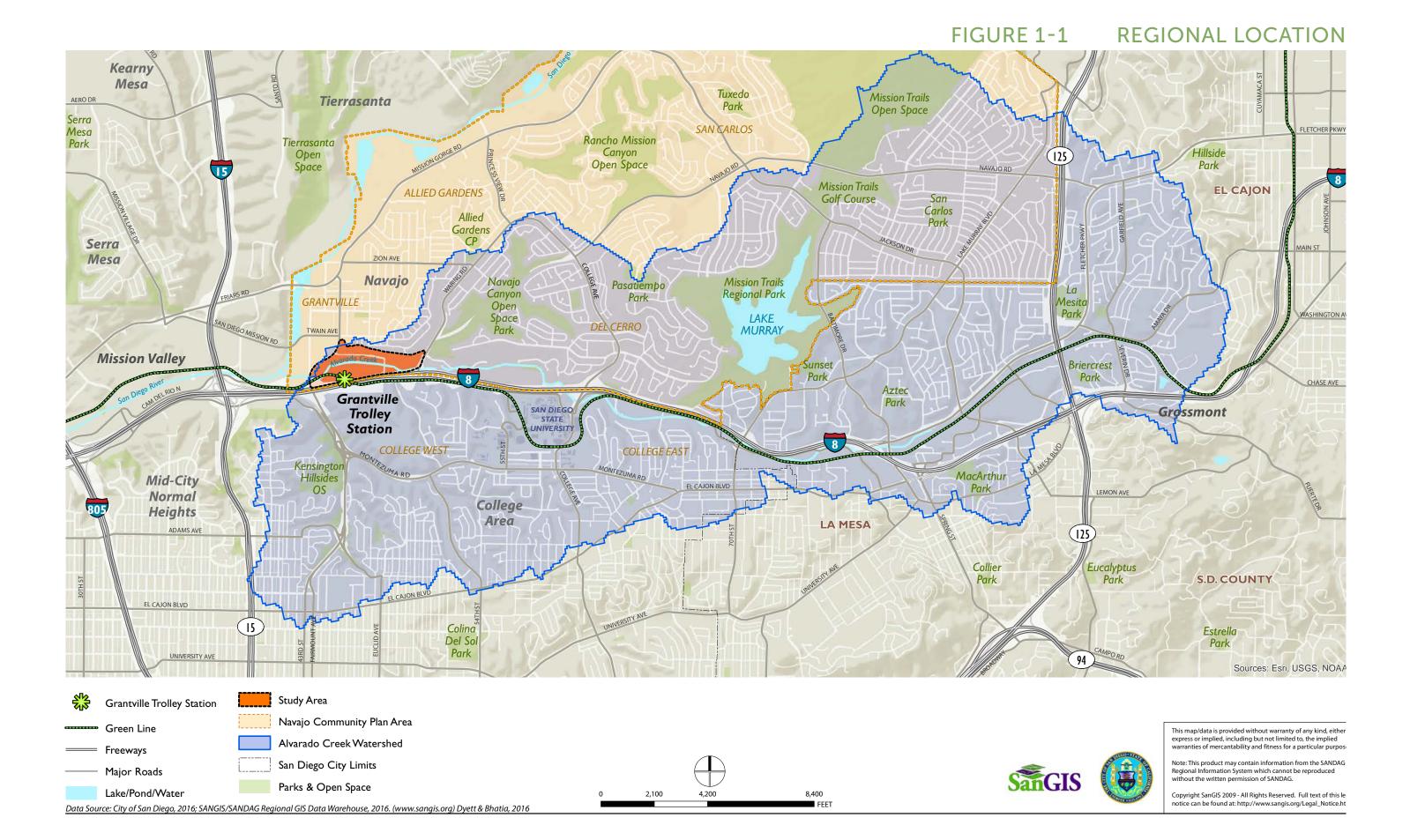
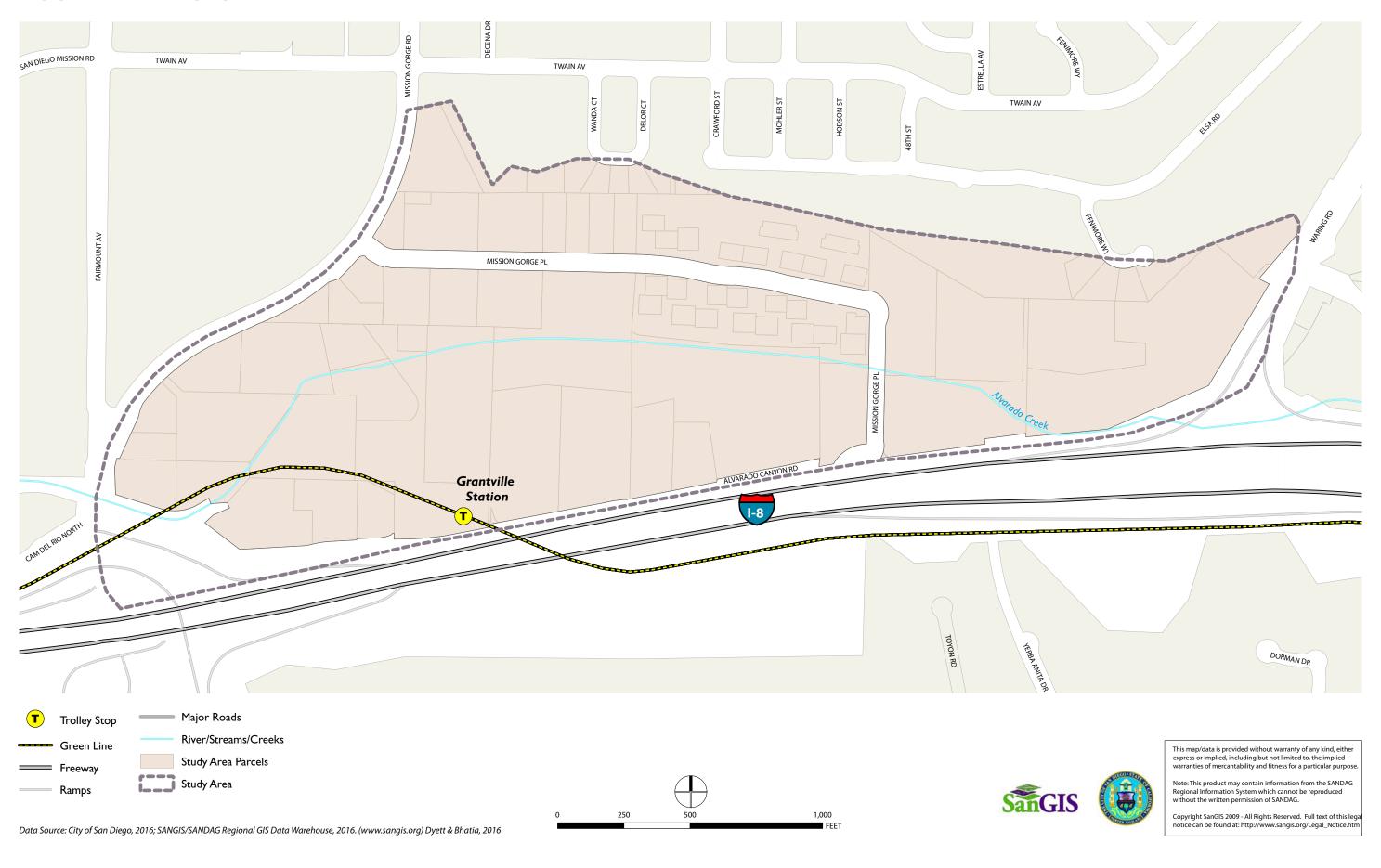
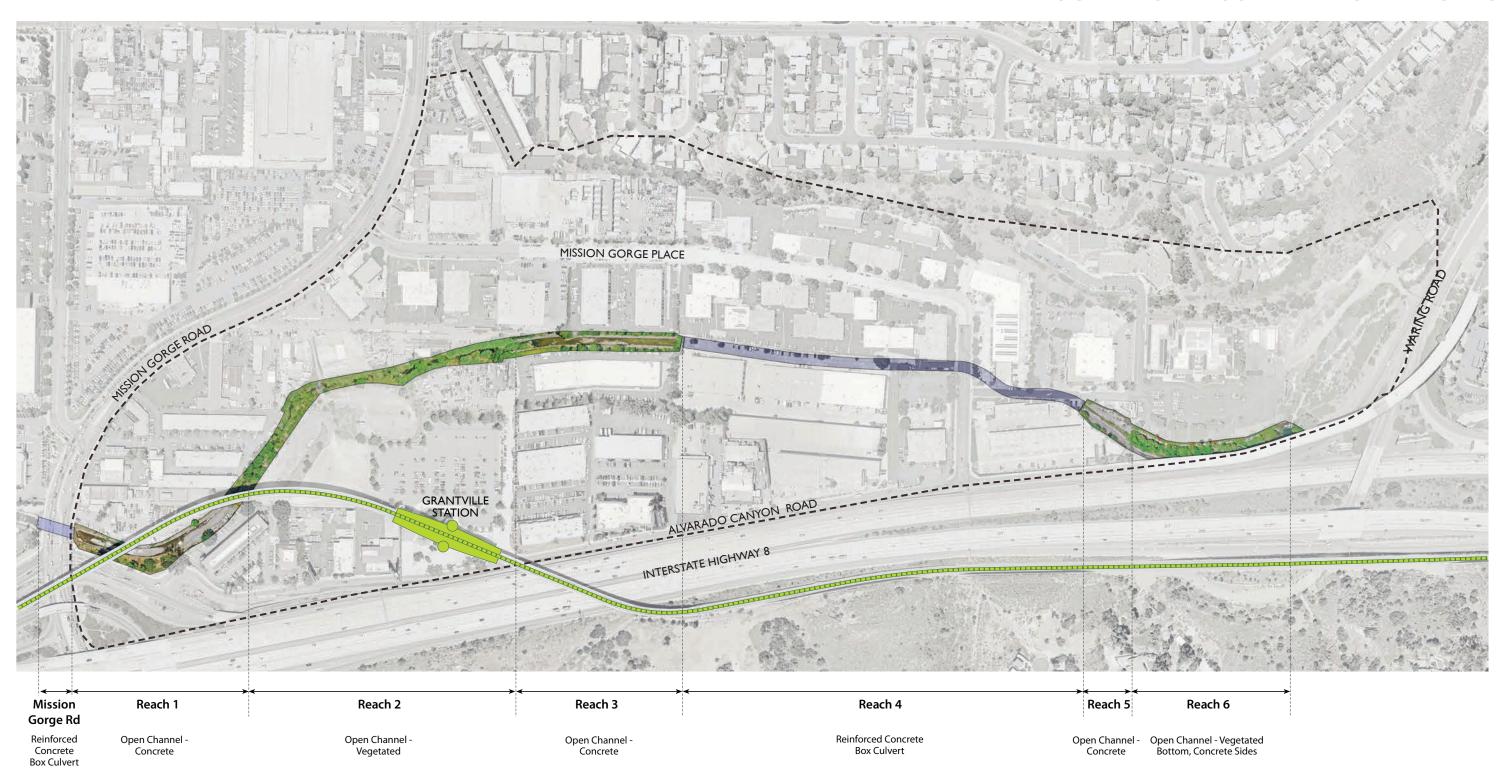


FIGURE 1-2 STUDY AREA

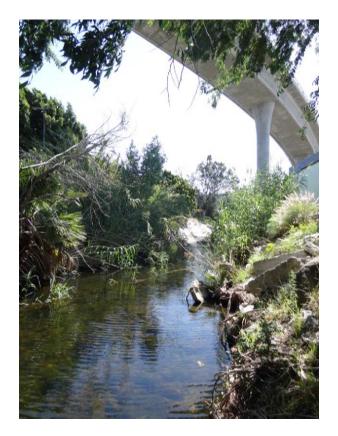




Green Line
---- Study Area

0 200 400 800 FEET

Sources: City of San Diego, 2016; SANGIS/SANDAG Regional GIS Data Warehouse, 2016; Dyett& Bhatia, 2017





Flooding in the Study Area



Alvarado Creek's water quality is negatively impacted by adjacent parking and hardscape.

2 Planning Objectives

2.1 SPECIFIC OBJECTIVES

In 2015, the City of San Diego adopted a Focused Plan Amendment for the Navajo Community Plan, which envisions transformation of the primarily industrial area around the Grantville Trolley Station (Grantville Station) into a vibrant, mixed-use community. The Community Plan seeks to promote housing and a mix of other uses in a transit-oriented setting, as well as new bicycle and pedestrian trails along open spaces.

To facilitate this transformation, the City of San Diego applied for and was awarded a Smart Growth Incentive Program (SGIP) grant from San Diego Association of Governments (SANDAG). The purpose of this grant is to develop concepts and assess the feasibility of alternatives to transform Alvarado Creek into an amenity that catalyzes redevelopment and improves access and connections to Grantville Station for new residents, local businesses, and the community as a whole.

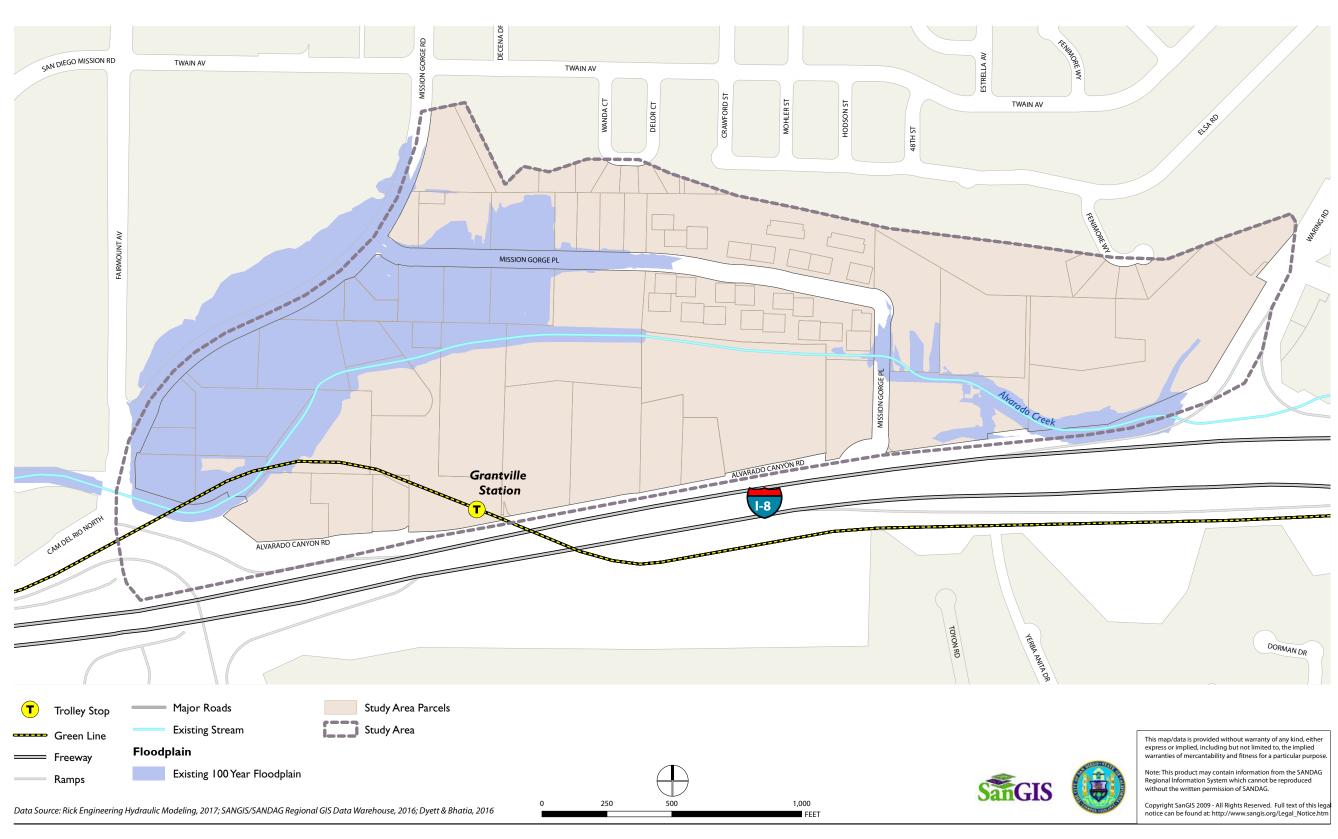
The specific planning objectives of the Grantville Trolley Station/Alvarado Creek Enhancement Study (Study) are discussed below.

IMPROVE FLOODING, STORMWATER, AND WATER QUALITY ISSUES

A primary planning objective is to develop strategies to manage the flooding that currently impacts surrounding properties and is an obstacle to transit-oriented development, particularly housing, in the Study Area. A significant portion of the Study Area is within a Special Flood Hazard Area (SFHA), the 100-year floodplain, as shown on Figure 2-1. The area north of Alvarado Creek along reaches 1 through 4, a limited area south of Alvarado Creek at Reach 2, and an area west of reaches 5 and 6 is shown within the 100-year floodplain. This is consistent with the hydraulic capacity analysis performed for this Study, which indicates that the majority of the channel reaches lack the hydraulic capacity to convey the 100-year storm event flow, as well as the flow from much smaller storm events.

Another planning objective is to improve Alvarado Creek's water quality, in part by minimizing stormwater runoff into the creek. Currently, Alvarado

FIGURE 2-1 EXISTING 100-YEAR FLOODPLAIN





Barricades around Alvarado Creek



Fencing surrounding Alvarado Creek





Creek's water quality is negatively impacted by its location in a low-lying area and the presence of industrial uses, open storage, vehicular parking, and hardscape immediately adjacent to it.

INCREASE ACCESS TO GRANTVILLE STATION AND ALVARADO CREEK

Pedestrian access to Grantville Station is impeded due to the significant obstacle to north-south connectivity presented by Alvarado Creek and the fact that there is only one north-south street connection within the nearly one-mile long Study Area. Moreover, very little of the creek is itself publicly accessible. Within the Study Area, the creek is frequently surrounded by fencing or concrete barriers, as well as by surface parking lots and outdoor storage.

ENHANCE ALVARADO CREEK AS AN AMENITY THAT INCLUDES BICYCLE AND PEDESTRIAN TRAILS

The Navajo Community Plan, as amended in 2015 by the Grantville Focused Plan Amendment, designates the planned land uses for the Study Area (see Figure 2-2). This figure shows that approximately 75 percent of the Study Area is designated as either Urban Village or Business Park-Residential, permitting housing to be built at densities up to 109 and 73 dwelling units per acre, respectively. This indicates an intention for the Study Area to transform from largely an industrial and service-commercial district to a mixed-use and residential

neighborhood that accommodates medium- and high-density housing. Indeed, the Focused Plan Amendment to the Navajo Community Plan Update is intended to facilitate approximately 8,280 new housing units in the planning area¹, approximately 6,260² of which are expected to be accommodated in the Study Area. Residents of these new housing units will benefit from areas to recreate outside their homes and pathways on which to travel, to access Grantville Station or stores to the north, for example. Although Alvarado Creek is currently an underutilized asset and an obstacle to mobility within the Study Area, it has the potential to operate as a natural linkage and a recreational resource for the community. In recognition of that, the Navajo Community Plan Update requires, among other things, that pedestrian and bicycle connectivity be enhanced and that a 10-foot wide multi-use pedestrian and bicycle trail directly adjacent to the Alvarado Creek be established.

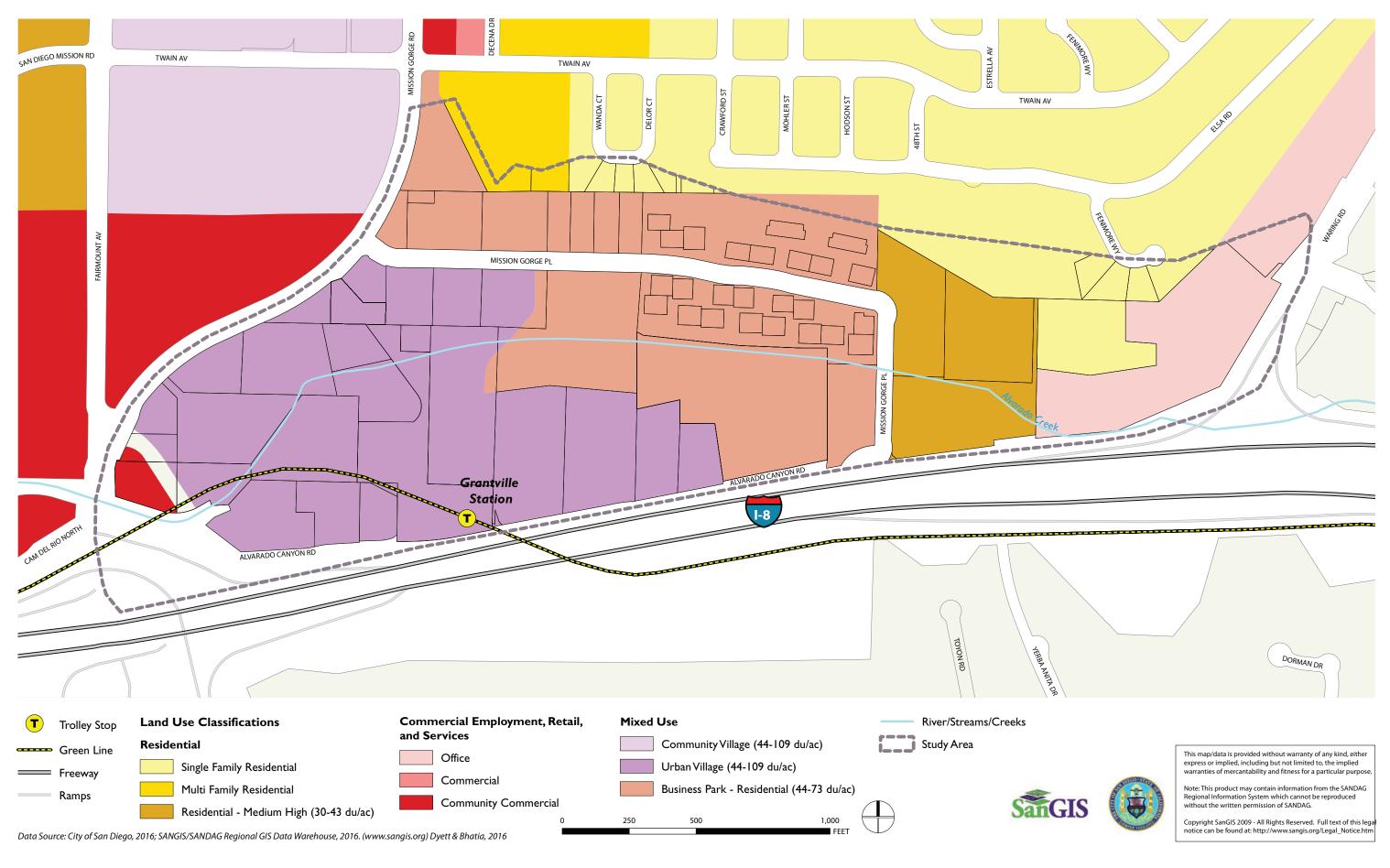
FOSTER TRANSIT-ORIENTED DEVELOPMENT ADJACENT TO THE GRANTVILLE STATION

Lastly, one of the primary Study objectives is to identify concepts to catalyze transit-oriented development in the Study Area. Essential to this objective is proposing strategies that ensure that homes built on the privately-owned parcels around Grantville Station are out of the 100-year floodplain, removing the flood insurance requirement there, while still leaving these parcels with sufficient developable space in which to construct mediumand high-density residential and mixed-use buildings.

¹Grantville Focused Plan Amendment Final Environmental Impact Report, April 2015.

²Based on residential and mixed-use land use designations fully developing at the highest allowable densities, with Office, Urban Village, and Business Park designations developing with 75 percent residential uses and 25 percent commercial uses.

FIGURE 2-2 PLANNED LAND USE (NAVAJO COMMUNITY PLAN)



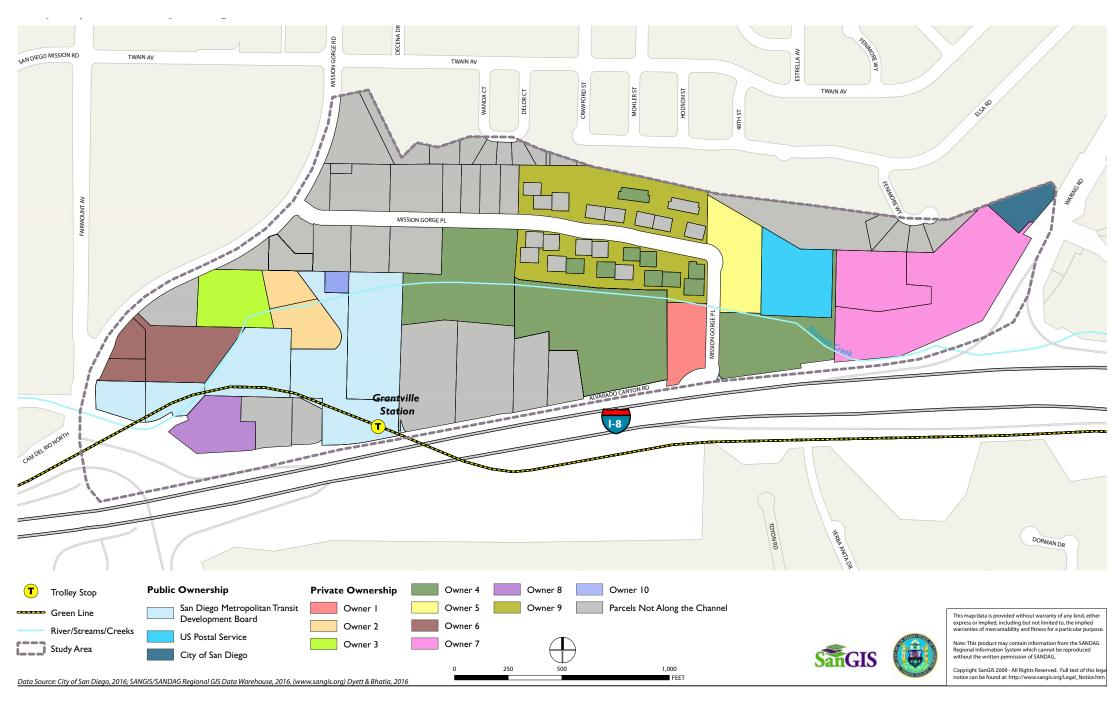
2.2 PLANNING CONSTRAINTS

Planning constraints are those that represent significant barriers that limit the physical or policyrelated aspects of formulated plans. The location of the Study Area results in several constraints that required consideration during the development of alternatives for the revitalization of Alvarado Creek and the enhancement of the area surrounding Grantville Station. The following are key issues that were encountered and represent significant constraints that apply to the Study.

LAND OWNERSHIP

As shown in Figure 2-3, most of the parcels through which Alvarado Creek flows are owned by private parties. Indeed, of the approximately 48 acres of parcels that are located adjacent to the creek in the Study Area, 35 acres are privately owned. In addition, the San Diego Metropolitan Transit System (MTS), a non-City public agency, owns several parcels adjacent to the creek; MTS has plans to further develop these parcels as housing near Grantville Station.

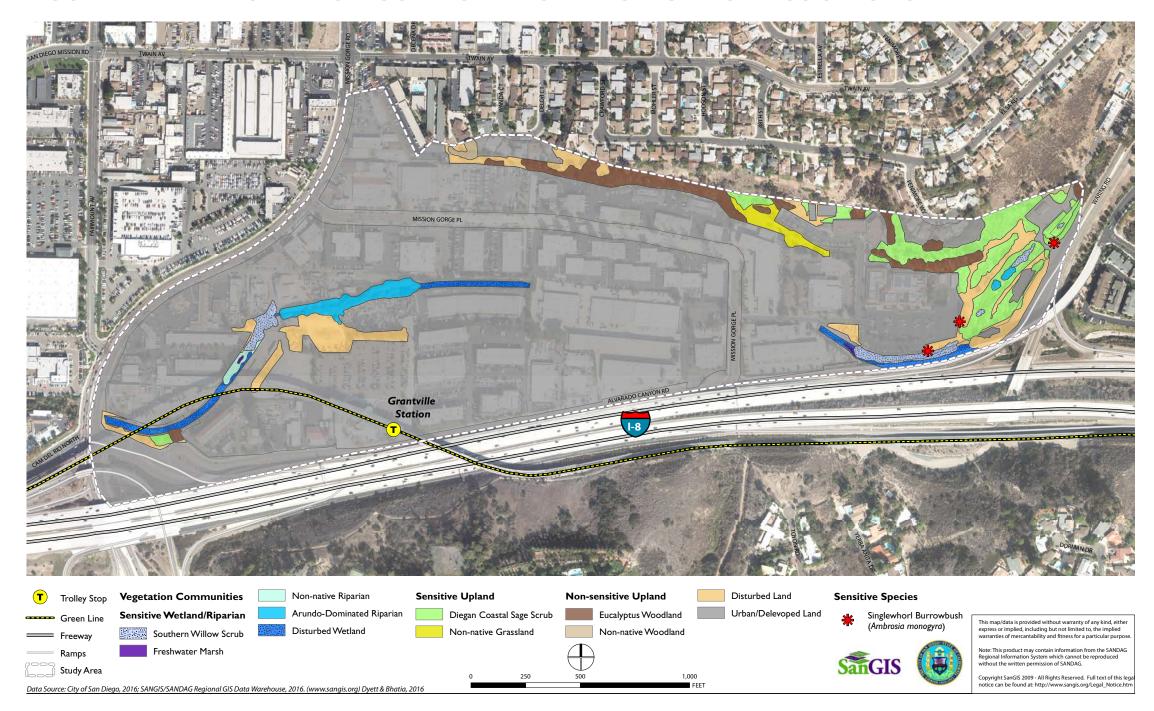
FIGURE 2-3 PROPERTY OWNERSHIP ALONG THE CHANNEL



BIOLOGICAL RESOURCES

There are approximately eight acres of sensitive vegetation communities³ within the Study Area, approximately two acres of which are wetland communities within the Alvarado Creek channel in Reach 2 (Figure 2-4). Impacts to these wetlands should be avoided. To the extent that impacts to these communities cannot be avoided, they would require mitigation at a ratio of 1:1 up to 3:1 depending on the type and sensitivity of the community, based on negotiations with the resource agencies and in conformance with San Diego's Environmentally Sensitive Lands Regulations and Biology Guidelines, as well as in compliance with federal "no net loss" policies for wetlands. In addition, although there is currently development adjacent to the existing vegetation, a habitat buffer would likely be required to be maintained around existing, enhanced, and created wetlands, as needed to protect the functions and values of these wetlands.

FIGURE 2-4 VEGETATION COMMUNITIES AND SPECIAL STATUS SPECIES



³Sensitive vegetation communities are those identified by Holland (1986) or identified by the City of San Diego in its Biology Guidelines (2012). The Biology Guidelines were formulated to aid in the implementation and interpretation of the Environmentally Sensitive Lands Regulations (ESL), San Diego Land Development Code, Chapter 14, Division 1, Section 143.0101. Section III of the Guidelines (Biological Impact Analysis and Mitigation Procedures) also serve as standards for the determination of impacts and mitigation under California Environmental Quality Act (CEQA). The ESL defines sensitive biological resources as those lands included within the Multi-Habitat Planning Area (MHPA) as identified in the City of San Diego's Multiple Species Conservation Program (MSCP) Subarea Plan (City of San Diego 1997) and other lands outside the MHPA that contain wetlands; vegetation communities classifiable as Tier I, II, IIIA or IIIB; habitat for rare, endangered or threatened species; or narrow endemic species.

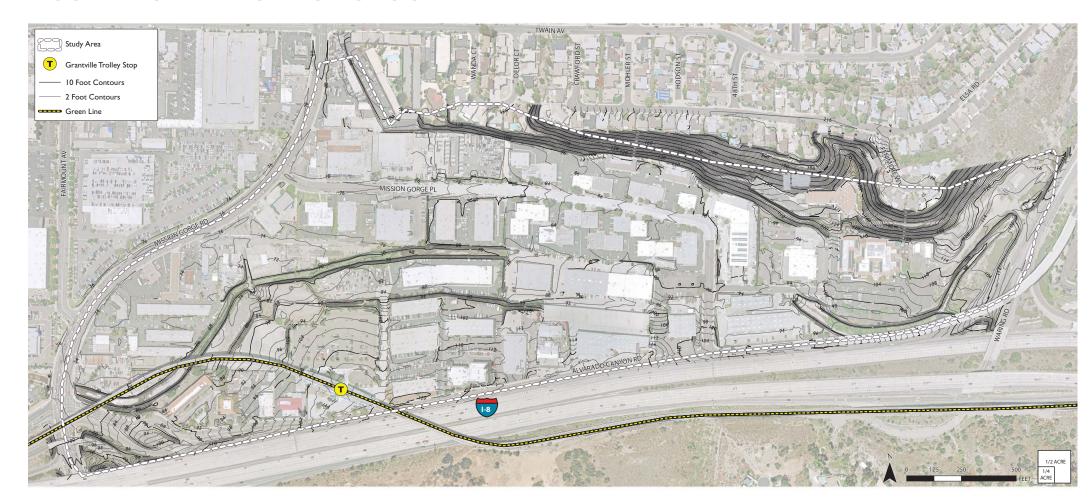
TOPOGRAPHY

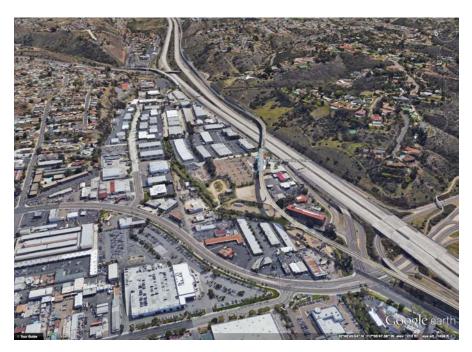
Alvarado Creek is located in a low-lying area of Grantville that gradually increases in elevation in the western and northern portions of the Study Area. In addition, Alvarado Creek's confluence with the San Diego River is located nearby and to the west of the Study Area. This limited elevation range and close proximity to the San Diego River limit the options to achieve additional channel capacity by deepening the channel.

UTILITIES AND STRUCTURES

There are a number of existing utilities and structures within the Study Area (see Figure 2-6). These include support columns associated with the above-grade Grantville Station near reaches 1 and 2, a low-water crossing approximately 500 feet northwest of Grantville Station, a triple box culvert located in Reach 4, a triple box culvert under Mission Gorge Road, and various utilities.

FIGURE 2-5 EXISTING TOPOGRAPHY







2.3 FEASIBILITY CRITERIA

In light of the project objectives and constraints, project alternatives are analyzed and evaluated based on the following feasibility criteria.

The flooding is managed

- 100-year storm is accommodated (via channel geometry, floodable green space, fill north of the channel, building design, etc.)
- At minimum, properties that are designated for residential use are raised out of the 100year frequency base flood elevation

Water quality is improved and stormwater is managed

• The amount of stormwater that flows into the creek is reduced and quality of water is improved (via green infrastructure, biofiltration areas, etc.)

Impacts to private property are minimized

- Proposed creek alignment minimizes, to the extent feasible, impacts on adjacent private property
- To the extent the proposed creek alignment impacts private property, owners maintain the ability to meaningfully develop their property
- Opportunities for partnerships are maximized
- Concerns of property owners and the surrounding community are taken into account

Biological impacts are avoided to the extent possible

- Impacts to the sensitive vegetative and wetland communities in the Study Area are minimized or mitigated
- There is no net loss to wetland communities and a wetland buffer is provided
- The ability to secure needed permits based on existing regulations

Connections within the study area are increased

- Access to the Grantville Trolley Station, particularly from north of the creek, is increased
- Bicycle/pedestrian bridge(s) are provided over the creek to increase north-south connections
- A 10-foot-wide continuous Multi-Use Trail is incorporated into creek corridor concepts for a consistent approach to the Grantville Supplemental Development Regulations
- Connections are accessible per the Americans with Disabilities Act (ADA), and ADA accessibility in the Study Area is improved
- Connections to Navajo Canyon and the San Diego River are provided

Alvarado Creek is transformed into an amenity

- Connections to Alvarado Creek are increased
- Open spaces are provided for the new residents planned for the area
- Trail design integrates opportunities for environmental education
- Views to the creek are opened up

The proposed design is context-sensitive

- Design provides safe and well-lit trails and crossings
- Porous material is given priority over impermeable materials
- Materials in the channel have a natural appearance (e.g., gabion, stones, blockcrete, berms) or use existing infrastructure
- To the extent the existing channel is substantially altered, such alteration is limited as necessary to either improving habitat functioning or controlling flooding, where no other feasible method of protecting existing public or private development exists and where such protection is necessary for public safety⁴
- Estimated cost of improvements (soft and hard costs) are minimized
- The location of existing utilities and structures are considered



Existing culvert under Mission Gorge Road

3 Preliminary Alternatives

3.1 VIABLE ALTERNATIVES

Three viable, preliminary alternatives were developed and analyzed to assess which alternative, or combination of interventions depicted in the alternatives, was the best potential strategy for achieving the Study objectives. Below, the common elements of all three viable, preliminary alternatives are described, and then each alternative is described, in turn. Then, the alternatives are evaluated and compared based on the feasibility criteria.

COMMON ELEMENTS

All three preliminary, viable alternatives shared certain elements, as described below.

NEW CULVERT UNDER MISSION GORGE ROAD

Preliminary hydraulic analysis indicated that events in excess of a 10-year storm event would exceed the current capacity of the existing triple reinforced-concrete box (triple RCB) culvert underneath Mission Gorge Road, located at the western edge of the Study Area. In other words, this is a

water flow choke point. As a result, all three preliminary alternatives propose that the existing culvert be supplemented with an additional 15-footwide by 8-foot-tall box, to increase capacity in that location and decrease the likelihood of upstream flooding.

CHANNEL WIDENING IN REACHES 1, 2, AND 3

Preliminary hydraulic modeling indicated that, for the channel to accommodate the 100-year flood in the Study Area's urban environment, the channel bottom needed to be widened to approximately 50 feet, with 1.5 horizontal to 1 vertical side slopes, in reaches 1, 2, and 3. Thus, all preliminary alternatives propose widening of the channel bottom, with natural materials, by approximately 15 feet in reaches 1 and 3. The existing concrete infrastructure is not proposed to be removed from those reaches. In Reach 2, the channel bottom is proposed to be widened by approximately 35 feet, using approximately half concrete and half natural materials.

MULTI-USE TRAILS ALONG THE CREEK CORRIDOR

The Grantville Community Plan Implementation Overlay Zone (CPIOZ) includes supplemental development regulations (SDR). One such SDR reguires that "...development along Alvarado Creek shall provide a 10-foot wide multi-use pedestrian and bicycle trail directly adjacent to the Alvarado Creek."5 In accordance with this requirement, all preliminary alternatives propose a Multi-Use Trail system along the Alvarado Creek channel. The exact trail alignment varies depending on the alternative. In all alternatives, the Multi-Use Trail system provides for connections to the Navajo Canyon Trail via Alvarado Canyon Road as well as to the San Diego River, potentially, at Mission Gorge/Fairmount Avenue. This proposed intervention follows the recommendations of the San Diego River Park Master Plan.⁶

BIKE/PEDESTRIAN BRIDGES OVER THE CHANNEL

All preliminary, viable alternatives specify conceptual locations for pedestrian/bicycle bridges over the Alvarado Creek channel, in order to increase connections, particularly north-south connections, within the Study Area.

GREEN STREET AT MISSION GORGE PLACE

All three preliminary alternatives also propose to enhance connectivity within the Study Area and to the creek by recommending that Mission Gorge Place, where the street is aligned roughly north-south and crosses the creek, would be improved with green infrastructure, such as tree plantings, pervious surfaces, bio-retention areas, and treatment planters.

MINIMAL INTERVENTION IN REACHES 5 AND 6

All three preliminary alternatives show minimal interventions in reaches 5 and 6, based on the results of preliminary hydraulic modeling. These reaches are wider than the others, with approximately 45-feet-wide channel bottoms, and have the existing capacity to manage a 25-year storm event. The hydraulic capacity of Reach 5 would likely be improved if the triple RCB culvert in Reach 4 were improved.

Another planning objective is to improve Alvarado Creek's water quality, in part by minimizing stormwater runoff into the creek. Currently, Alvarado Creek's water quality is negatively impacted by its location in a low-lying area and the presence of industrial uses, open storage, vehicular parking, and hardscape immediately adjacent to it.



Current conditions of Mission Gorge Place



Green Infrastructure improvements, such as those pictured above, would help implement Study objectives for Mission Gorge Place in all three preliminary viable alternatives.

⁵Navajo Community Plan on page 44.

⁶See San Diego River Park Master Plan, pages 73-74.

ALTERNATIVE 1

Alternative 1 proposes a linear green space north of the Alvarado Creek channel to both manage flooding and provide a recreational corridor, as well as to emphasize the connection to the San Diego River. A conceptual plan and cross-sections are provided (Figures 3-1 through 3-6). They show the following conceptual improvements.

REACHES 1, 2, 3

- 1. Widening the creek channel to the north in reaches 1, 2, and 3
- 2. Floodable Green Space, approximately 80 feet wide, north of the creek in reaches 1, 2, and 3
- 3. Three pedestrian/bicycle bridges over Alvarado Creek

REACH 4

- 4. The addition of a new underground culvert adjacent and to the north of the existing triple RCB culvert in Reach 4
- 5. A Recreational Corridor, approximately 80 feet wide, over the existing and proposed culvert system in Reach 4

FIGURE 3-1 ALTERNATIVE 1

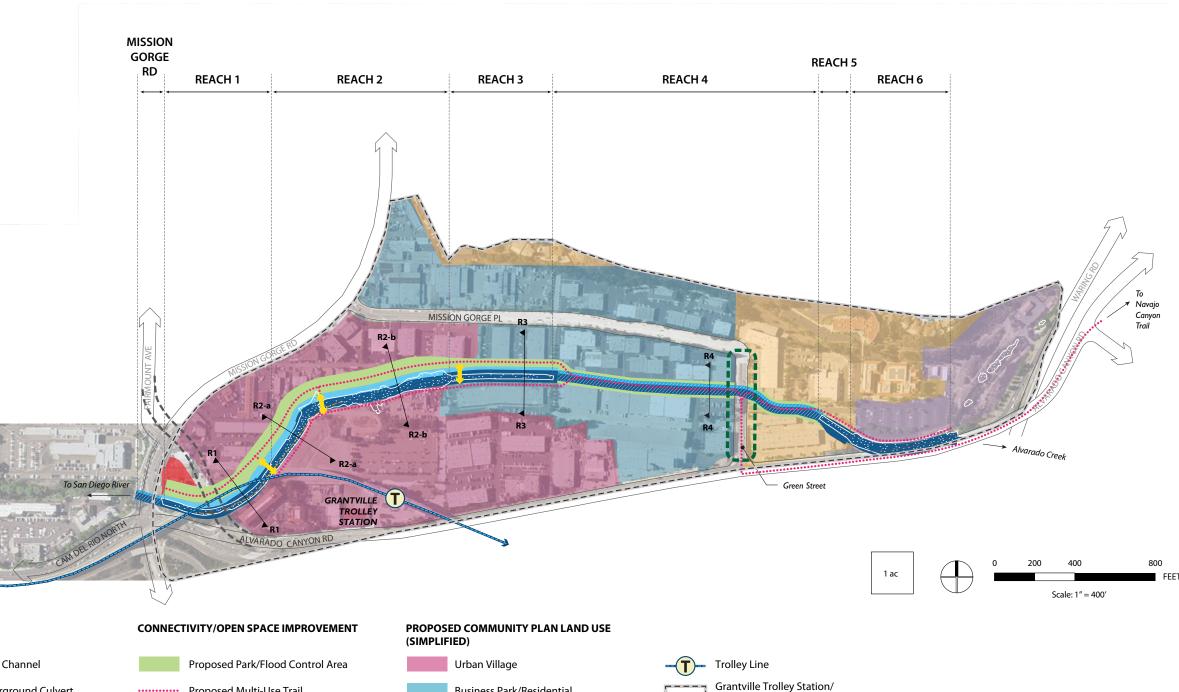




FIGURE 3-2 ALTERNATIVE 1, REACH 1 SECTION

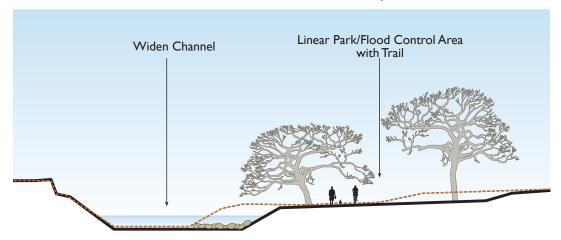


FIGURE 3-4 ALTERNATIVE 1, REACH 2-B SECTION

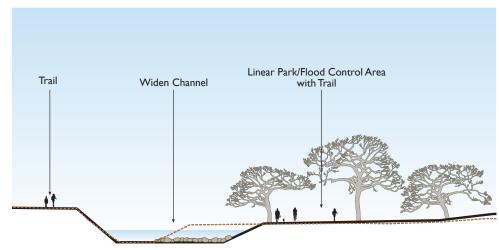


FIGURE 3-6 ALTERNATIVE 1, REACH 4 SECTION

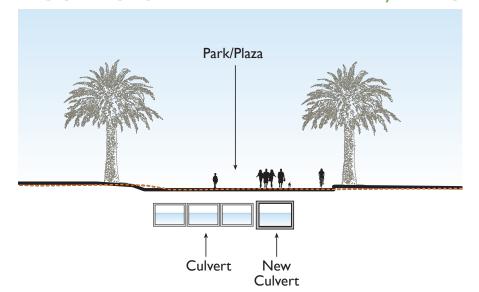


FIGURE 3-3 ALTERNATIVE 1, REACH 2-A SECTION

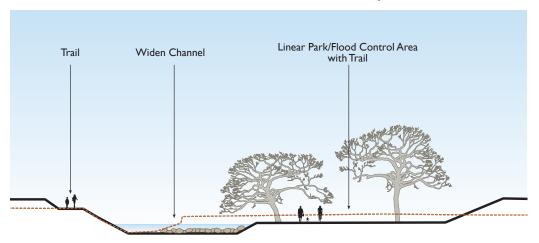
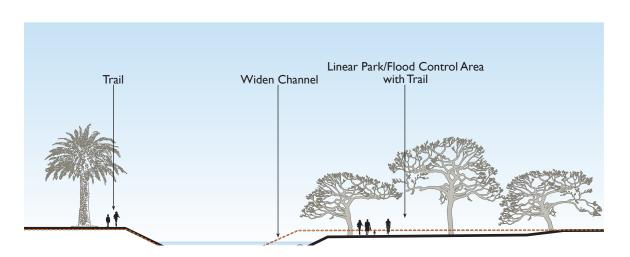


FIGURE 3-5 ALTERNATIVE 1, REACH 3 SECTION



Realignment

ALTERNATIVE 2

Alternative 2 proposes pockets of green space in strategic locations as a means to manage flooding, provide recreational opportunities, and transform the creek into an amenity. A conceptual plan and cross-sections are provided (Figures 3-7 through 3-12). They show the following conceptual improvements.

REACHES 1, 2, 3

- 1. Widening the creek channel to the north in reaches 1, 2, and 3
- 2. Floodable pockets of green space on both sides of the creek in reaches 1, 2, and 3
- 3. Four pedestrian/bicycle bridges over Alvarado Creek
- 4. The addition of fill in between the creek channel and Mission Gorge Place in the western portion of Reach 2

REACH 4

- 5. The addition of an open channel (with a bottom composed of concrete in part, and natural materials in part) north of the existing triple RCB culvert
- 6. Two pedestrian/bicycle bridges over the proposed open channel

FIGURE 3-7 ALTERNATIVE 2

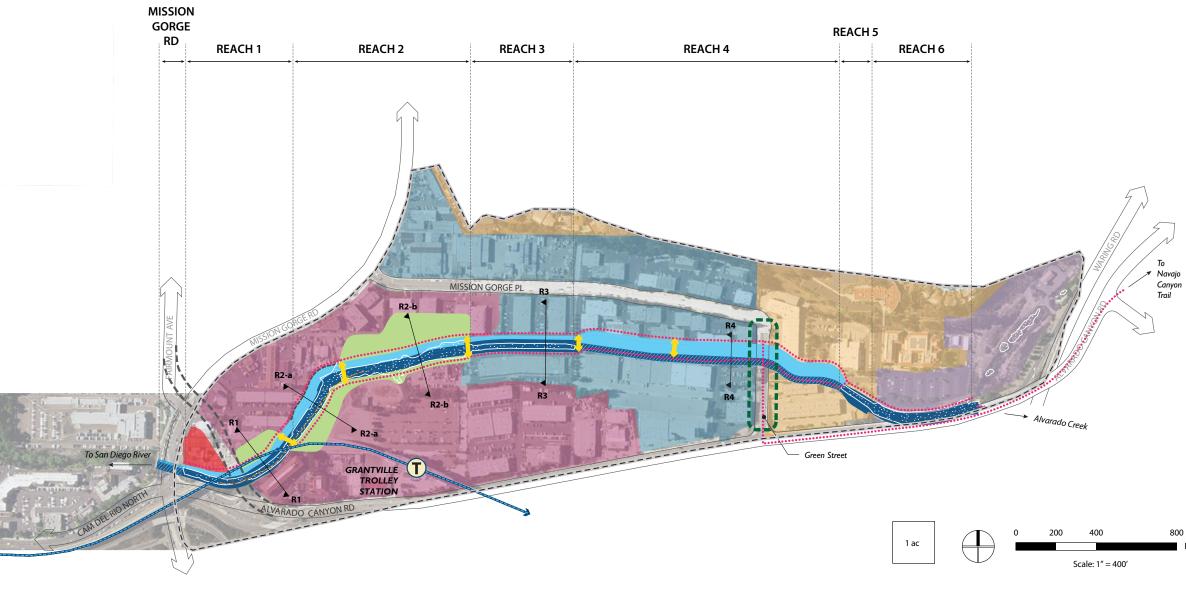




FIGURE 3-8 ALTERNATIVE 2, REACH 1 SECTION

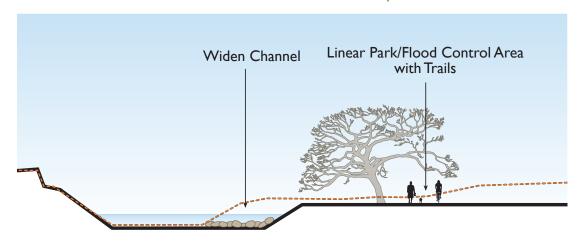


FIGURE 3-10 ALTERNATIVE 2, REACH 2-B SECTION

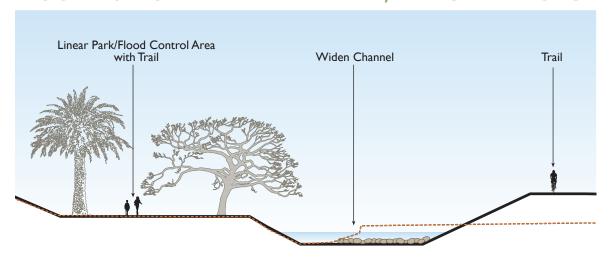


FIGURE 3-12 ALTERNATIVE 2, REACH 4 SECTION

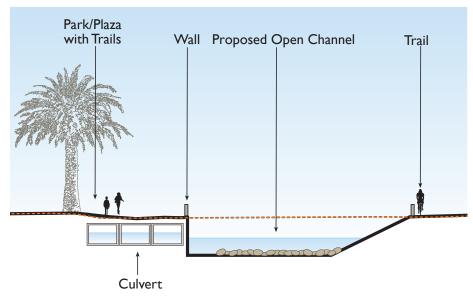


FIGURE 3-9 ALTERNATIVE 2, REACH 2-A SECTION

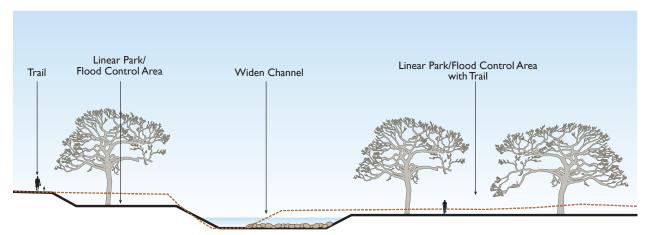
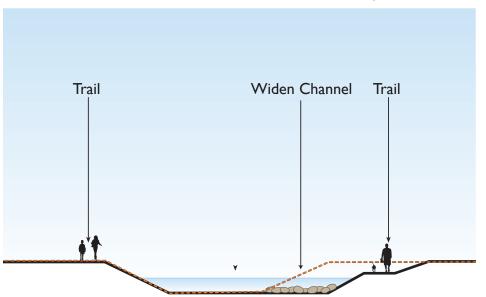


FIGURE 3-11 ALTERNATIVE 2, REACH 3 SECTION



Realignment 1

ALTERNATIVE 3

Alternative 3 proposes a larger swath of green space, mostly south of the creek and adjacent to Grantville Station, to manage flooding, provide recreational opportunities, and enhance the creek and its surroundings. A conceptual plan and cross-sections are provided (Figures 3-13 through 3-18). They show the following conceptual improvements.

REACHES 1, 2, 3

- 1. Widening the creek channel to the north in reaches 1, 2, and 3
- 2. Floodable pockets of green space to the north of the creek in Reach 1 and to the south of the creek in reaches 2 and 3
- 3. Three pedestrian/bicycle bridges over Alvarado Creek
- 4. The addition of fill in the area between the creek channel and Mission Gorge Place in Reach 2

REACH 4

- 5. The addition of a new underground culvert adjacent and to the north of the existing triple RCB culvert in Reach 4
- 6. A Recreational Corridor directly over the existing and proposed underground culvert in Reach 4

FIGURE 3-13 ALTERNATIVE 3

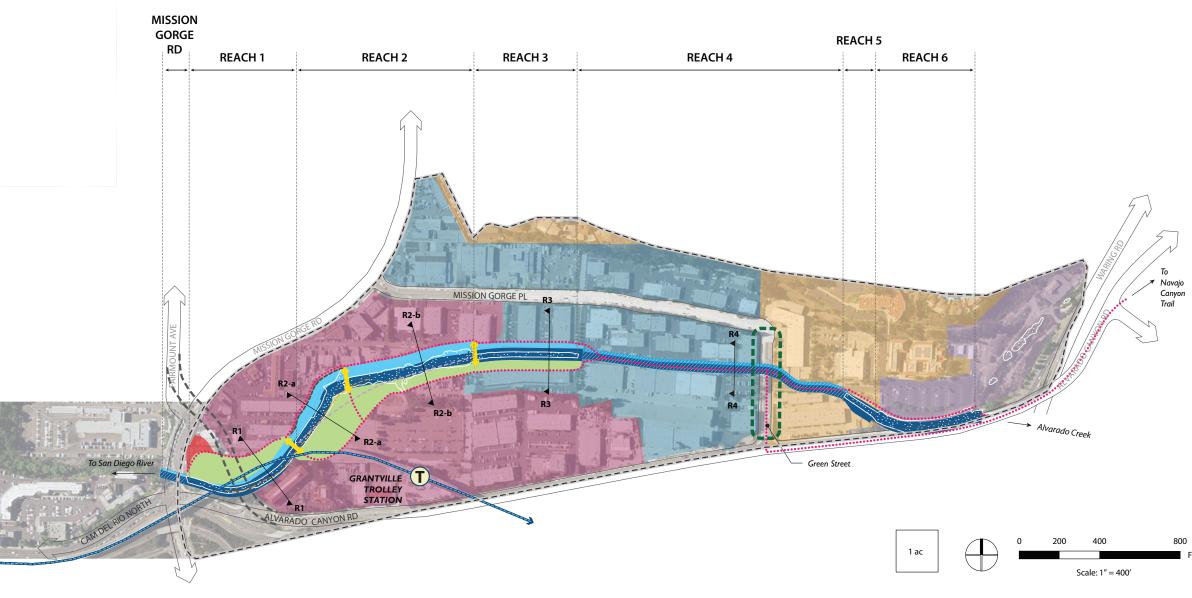




FIGURE 3-14 ALTERNATIVE 3, REACH 1 SECTION

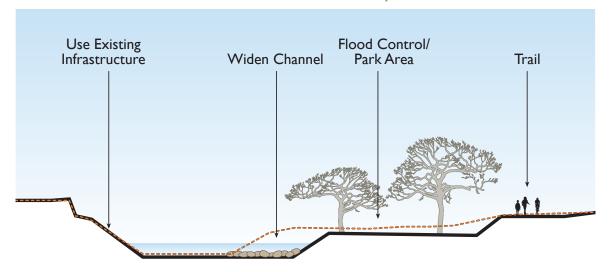


FIGURE 3-16

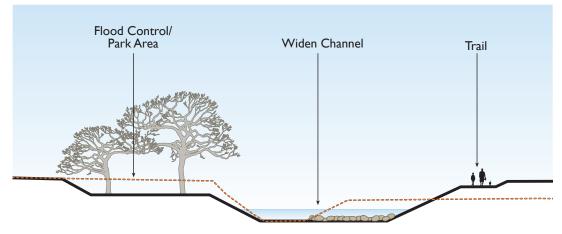


FIGURE 3-18 **ALTERNATIVE 3, REACH 4 SECTION**

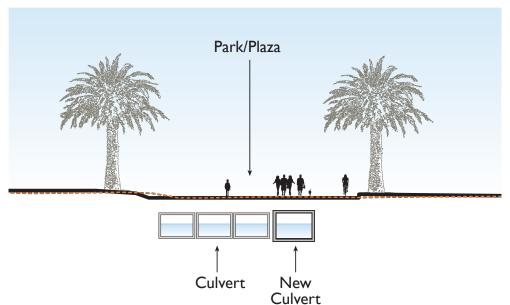
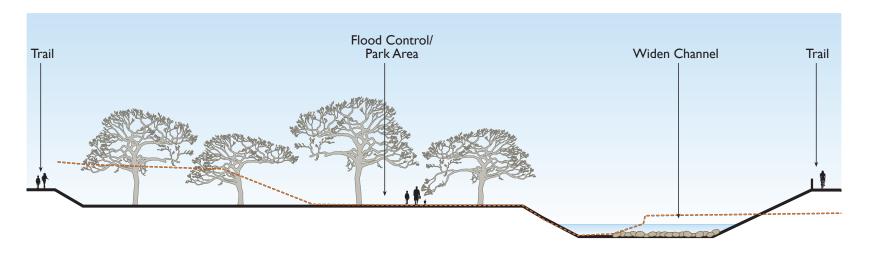
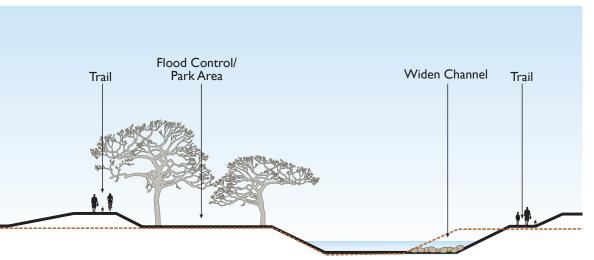
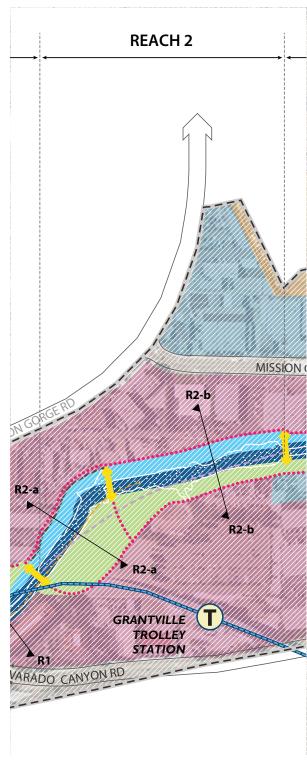


FIGURE 3-15 ALTERNATIVE 3, REACH 2-A SECTION



ALTERNATIVE 3, REACH 2-B SECTION FIGURE 3-17 ALTERNATIVE 3, REACH 3 SECTION





The dashed purple line above indicates the centerline for the potential realignment considered in Alternative 3.

3.2 ADDITIONAL CONSIDERATIONS CHANNEL RE-ALIGNMENT

As shown in Alternative 3, the realignment of the creek channel was examined as a potential way to avoid impacts to the sensitive vegetation communities in Reach 2, provide space for on-site wetland restoration, if any unavoidable, temporary losses to these communities occurred, and create room for habitat buffers.⁷ Realigning the creek in this fashion would provide the additional benefit of removing the pinch point present in Reach 2, thus improving hydraulic functioning.

ALVARADO CANYON ROAD ALIGNMENT

An additional consideration, shown on all viable preliminary alternatives, is the proposed realignment of Alvarado Canyon Road through the Study Area, which would connect Alvarado Canyon Road to Mission Gorge Road at the existing Fairmount Avenue intersection.

3.3 REJECTED ALTERNATIVES NATURALIZATION OF REACHES 1,2, AND 3

Naturalization of the channel in reaches 1, 2, and 3 was considered and determined to not be feasible. Preliminary hydraulic modeling analyzed the channel width required to accommodate the 100-year flood, removed all concrete from the channel bottom and sides in reaches 1 and 3, and maintained

a vegetated bottom and sides in Reach 2. This modeling revealed that an unmaintained channel with riparian vegetation would require approximately 200 feet of width to accommodate a 100-year storm event without flooding. This substantial widening of the existing channel would foreclose development opportunities on adjacent properties, and likely require acquisition of many properties. This intervention would also fail to meet several Study objectives.

CONSISTENT CHANNEL GEOMETRY

Early on, the question was asked about whether the channel could be equally distributed on all privately-owned areas. The preliminary hydraulic modeling indicated that some consistency in channel components would improve conditions (i.e., consistent channel bottom width). However, the creek channel currently varies in geometry through the Study Area; the channel's width is inconsistent, as are the materials used in it. A portion of the channel has already been directed into underground box culverts; however, full channelization would fail to meet the Study objectives. Additionally, due to the presence of sensitive vegetation and potential mitigation requirements, it was deemed infeasible to construct and permit a full culvert.

DIVERSION OF WATER UN-DERGROUND VIA PIPE

Diverting water from large storm events into an underground pipe was considered and determined to be infeasible. First, such a pipe would, most likely, need to be aligned underneath the right-of-way or an easement. This would likely require moving utilities and/or new easements, which could be cost prohibitive. Second, such a pipe would likely

need to be quite large; to provide a rudimentary estimate, a 20 foot by six foot pipe could be needed. A project of this size would also, very likely, be cost prohibitive.

DAYLIGHTING OF REACH 4

Removal of the triple RCB culvert and daylighting of the channel in Reach 4 was also considered and determined to not be feasible. There were several factors that contributed to this decision. First, daylighting would raise the water surface elevation in Reach 4, requiring significant channel widening there. Second, daylighting would reduce connectivity in the Study Area, as the culvert enables uninterrupted connections between the northern and southern portions of the Study Area. Third, the expense to replace a functioning culvert that has been constructed over is an avoidable one, the area would require re-parcelization, and at least one additional pedestrian/bicycle bridge would need to be constructed. As shown in Alternative 2, the addition of an open channel adjacent to the existing culvert was explored as a means to allow access to the creek channel in Reach 4 while leaving more developable land north of the channel. That too was determined to not be the preferred solution in Reach 4. Indeed, feedback from the community and stakeholders did not support this Alternative.

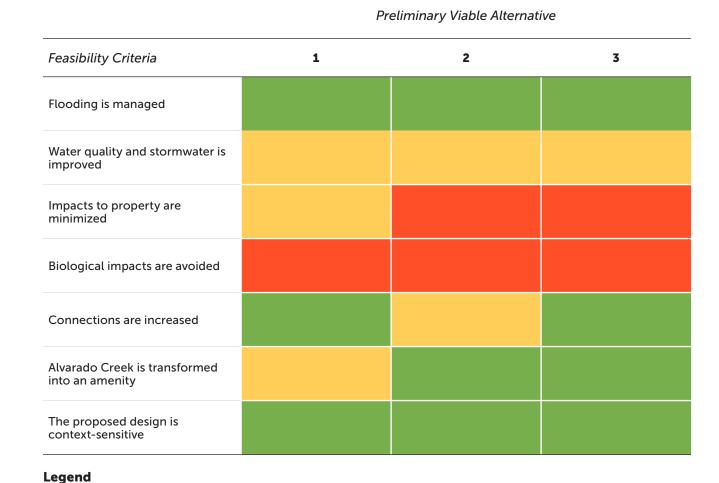
Evaluation of Preliminary Alternatives

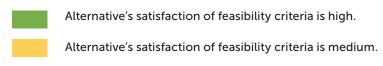
Evaluation of the viable, preliminary alternatives was based on the Study Team's assessment of how well each alternative satisfied the feasibility criteria and responded to the feedback and concerns expressed by stakeholders throughout the planning process, and in particular during a series of community workshops held in February 2017 and in discussions with City staff. Table 4-1 is a matrix that was prepared to compare the preliminary, viable alternatives and how they satisfied the feasibility criteria.

Based on the evaluation of the preliminary, viable alternatives and stakeholders' feedback, the Study Team further refined elements from Alternative 1 and 3. The key considerations were whether impacts on adjacent property owners could be further minimized, allow property owners to meaningfully develop their property, and be equitably distributed, while managing flooding and providing some passive recreation space for new residents.

In addition, the Study Team further explored realigning the channel in Reach 2, to evaluate if impacts to the sensitive vegetation and wetlands could be avoided completely, and if not, then minimized. Widening Reach 2 in its existing alignment, as contemplated in all of the viable, preliminary alternatives, would unavoidably impact the vegetation communities located within that reach. If the existing alignment of Reach 2—the narrowest reach—was maintained without alteration, there would be no feasible manner to protect adjacent public and private development from flooding conditions, as necessary for public safety. The Study Team decided to further explore realigning Reach 2 due to the intervention's potential both to improve flooding conditions with minimal impacts to the sensitive vegetation communities and to improve the wetland and riparian habitat of Reach 2 by providing space on-site to create and enhance wetlands and buffer new and existing wetlands.

TABLE 4-1: EVALUATION OF PRELIMINARY VIABLE ALTERNATIVES



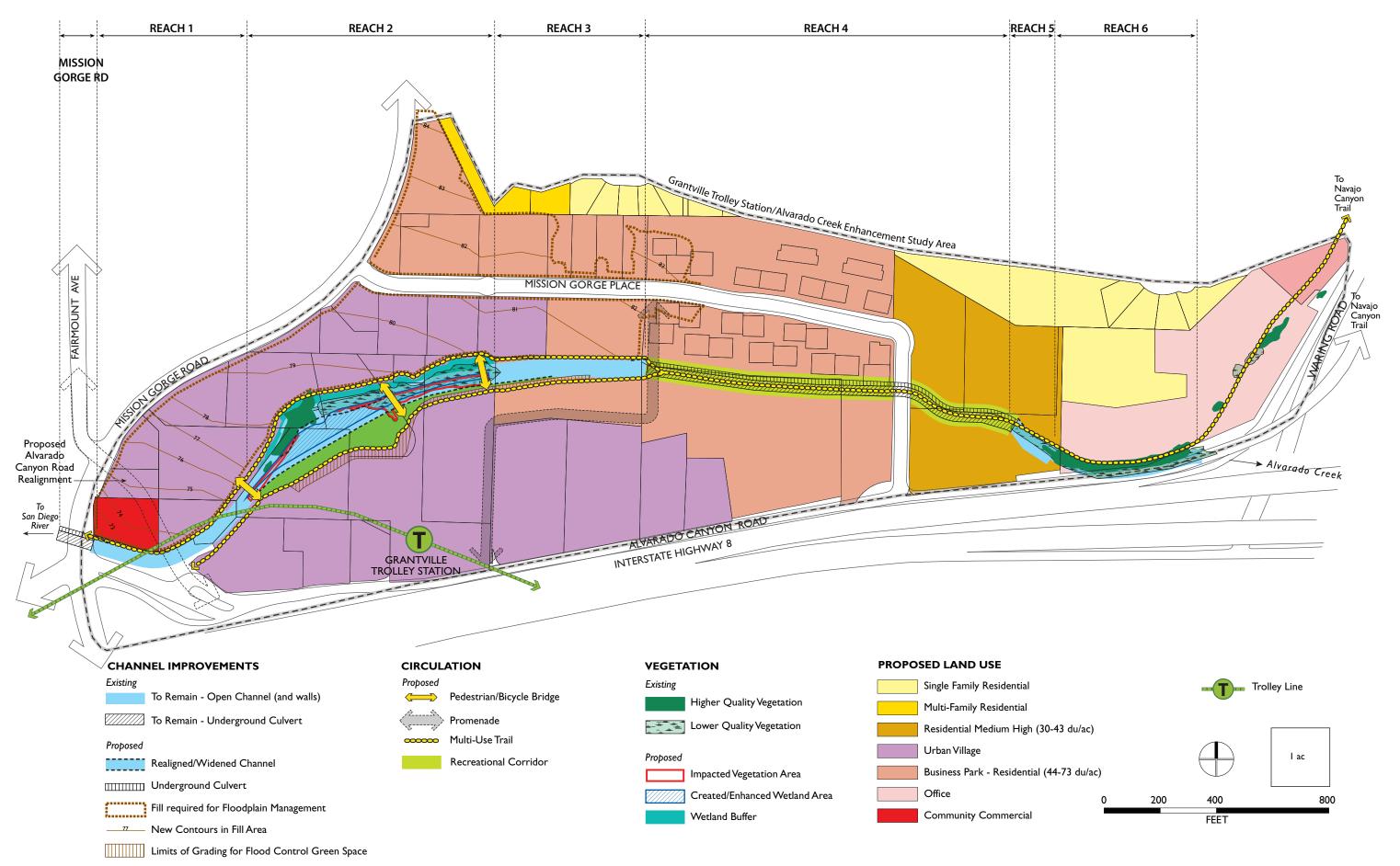


Alternative's satisfaction of feasibility criteria is low.

5 Preferred Alternative/Design

As stated previously, the Preferred Design was arrived at by refining elements of preliminary, viable Alternatives 1 and 3, and realigning the channel in Reach 2. Below is a discussion of the analysis and iteration preformed on each reach, which helped the Study Team arrive at the Preferred Design. Detailed discussion of the hydraulic analysis performed may be found in the Hydrology Background Report (Appendix D). A full discussion of the proposed Revitalization Strategy, including conceptual solutions for each reach, may be found in the Revitalization Study (Chapter 3).

FIGURE 5-1 PREFERRED ALTERNATIVE/DESIGN



5.1 PREFERRED CHANNEL IMPROVEMENTS MISSION GORGE ROAD

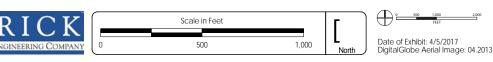
Hydraulic modeling analyzed whether an additional box, adjacent to the existing triple RCB culvert under Mission Gorge Road, was required to alleviate the flooding conditions seen along Mission Gorge Road. This modeling indicated that an additional 15-foot-wide by 8-foot-high box alleviates the flooding condition; if such a box is not installed, the flooding along Mission Gorge Road would continue during 100-year storm events (Figure 5-2). As a result, the Preferred Design proposed a new box under Mission Gorge Road, parallel to the existing culvert.

REACH 1

Hydraulic modeling also analyzed to what extent channel widening was required in Reach 1. All preliminary, viable alternatives showed the channel bottom of Reach 1 being widened by approximately 15 feet to the north. Modeling indicated that the only widening required was as needed to connect to the new box under Mission Gorge Road. As a result, the Preferred Design proposes only this amount of widening in Reach 1, lessening the impacts to adjacent properties, in comparison to the preliminary viable alternatives.

FIGURE 5-2 MISSION GORGE ROAD CULVERT EXPANSION EVALUATION



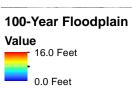


100-year floodplain with Mission Gorge Road culvert expansion





100-year floodplain without Mission Gorge Road culvert expansion



REACH 2

All preliminary, viable alternatives showed the potential to impact sensitive vegetative communities and wetlands in Reach 2. Moreover, they did not provide space on-site for restoration or for a sufficient buffer for the existing and/or created wetlands. Thus, the Study Team evaluated whether impacts could be avoided by altering the flow of the creek in Reach 2, in support of the Study objective to avoid biological impacts to the extent possible (Figure 5-3).

It was determined that given the need to widen the creek for hydraulic capacity and flood control, impacts to wetlands could not be fully avoided. Thus, the question became how impacts could be minimized. It was determined that directing some of the flow into a newly constructed, realigned channel south of the existing channel in Reach 2 is a potential solution that could achieve the greatest number of Study objectives. In this solution, the original channel would be graded only in key areas, minimizing the impacts to wetlands and sensitive vegetation that are found along the existing vegetated bottom and sides there. In comparison, widening the existing channel to manage flooding would cause much greater impacts. In addition, constructing a new channel would allow for

greater flood management. Further, the space in between the existing and new channels, as well as within the new channel, could provide space and the conditions for creating and enhancing wetlands, thus improving the habitat functioning on-site. This alignment potentially provides space to account for temporary losses to approximately 0.02 acres of higher quality sensitive vegetation communities (i.e. southern willow scrub) to be created at a ratio of approximately 2:1; temporary losses to approximately 0.55 acres of low-quality vegetation (i.e., arundo-dominated riparian, non-native riparian, and disturbed wetland) to be enhanced at a ratio of approximately 1:1.

The Study Team also iterated on the size and alignment of floodable green space south of the channel in Reach 2. As shown in Figure 5-4, several iterations were explored to satisfy two key Study objectives of creating an amenity and managing flooding.

FIGURE 5-3 REACH 2 REALIGNMENT EVALUATION

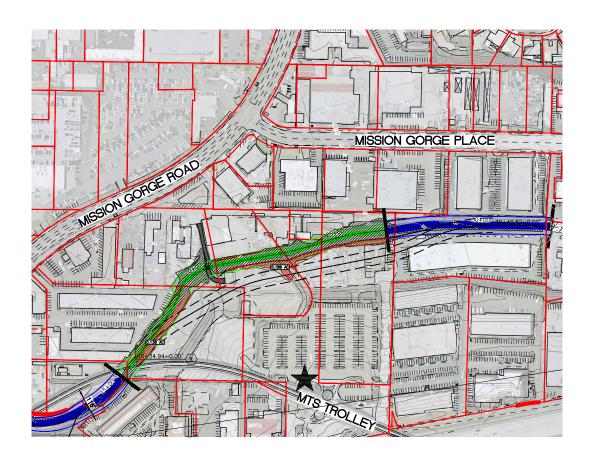
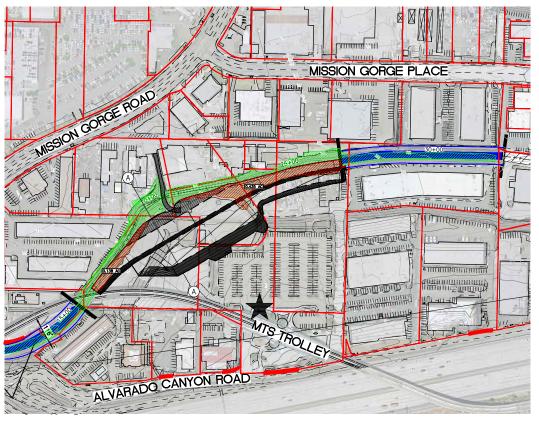
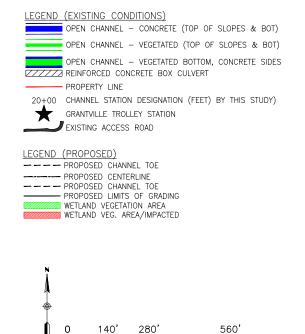


FIGURE 5-4

REACH 2 FLOODABLE GREEN SPACE SIZE AND ALIGNMENT EVALUATION







Reaches 3 and 4

REACH 3

All preliminary, viable alternatives showed the channel bottom of Reach 3 being widened by approximately 15 feet to the north, across the entire reach, as this was needed to connect reaches 2 and 4 with sufficient conveyance capacity. Hydraulic modeling analyzed to what extent channel widening was required in Reach 3. With the additional capacity upstream and downstream, Reach 3 only needed to be widened to connect to Reach 2 and to the new box in Reach 4, impacting the southwestern and northeastern portions of the reach, respectively. As a result, the Preferred Design proposes only this extent of widening in Reach 3, lessening the impacts to adjacent property in comparison to the preliminary viable alternatives.

REACH 4

Hydraulic modeling indicated that in the absence of any improvements to Reach 4, flooding on adjacent properties ensued. An open culvert would need greater right-of-way and to create new barriers to north-south movement, compared to a boxed culvert. Modeling indicated the appropriate box culvert size to be 15 feet by eight feet, which is reflected in the Preliminary Draft Preferred Alternative. A Recreational Corridor for passive recreation is proposed over the existing and proposed culvert to provide an amenity and also provide a continuous path indicated in the supplemental development regulations; the corridor extends 15 feet on either side of the culvert system, over the area where an easement would be required to preserve access to repair and maintain the culverts.

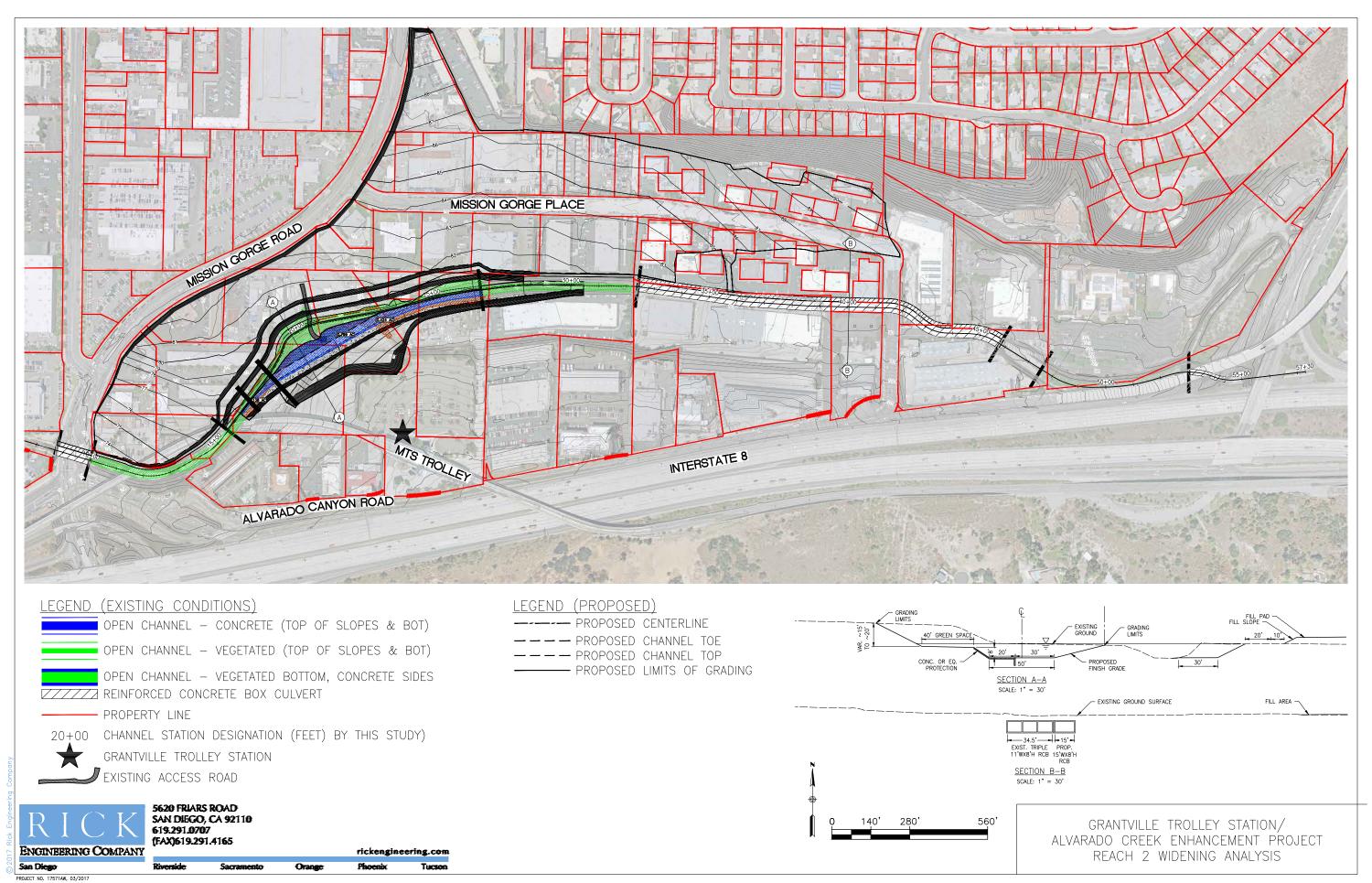
5.2 REJECTED CHANNEL IMPROVEMENTS WIDENED CHANNEL TO DECREASE FILL

In response to inquiries by the Flooding Subcommittee of the Navajo Community Planning Group, the Study Team evaluated whether widening the creek channel in reaches 2 and 3 would lessen the amount of fill required north of the channel while still managing the flooding. The Study Team found that widening the channel by 30 feet would reduce the required fill by, on average, one-half of a foot (Figure 5-5). This option was determined infeasible, as widening the channel did not meaningfully lessen the fill required north of the channel and would have negative impacts on adjacent property owners' ability to develop their properties. This analysis was discussed at the meeting of the Flooding Subcommittee on June 9, 2017.

FLOODABLE GREEN SPACE IN REACH 4

The Study Team also evaluated whether the addition of floodable green space in Reach 4 could manage the flooding in that location and achieve Study objectives of transforming Alvarado Creek into an amenity, in part by providing open space for new residents planned for the area. Hydraulic modeling indicated that the addition of such a green space would not manage the flooding that is seen in Reach 4, absent the addition of a new box adjacent to the existing RCB culvert. Thus, this option was rejected.

FIGURE 5-5 REACH 2 CHANNEL WIDENING/REQUISITE FILL ANALYSIS



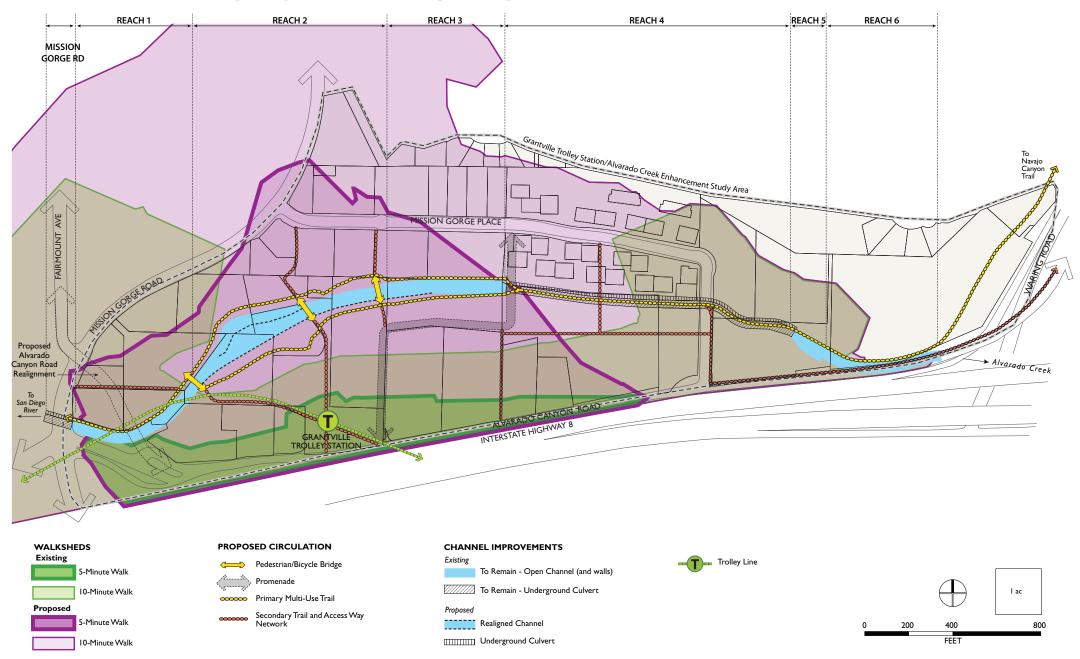
5.3 PREFERRED TRAIL ALIGNMENT

Upon refinement of the channel improvements, the trail alignment was evaluated and refined. The preferred trail and circulation system consists of three proposed, interconnected components:

- The Promenade, which extends private vehicular roadways into a public, neighborhood, multimodal connector.
- 2. **The Primary Trail System,** consisting of the two elements three pedestrian/bicycle bridges over the creek, and a Multi-Use Trail system along the creek corridor.
- 3. The Secondary Access Way and Trail Network, which provides additional north-south and east-west connections for pedestrians and bicyclists in the Study Area, increases access to Grantville Station, and provides pathways from surrounding transit stops and land uses to key points within the Study Area.

These are elaborated upon below and mapped in Figure 5-6. A visualization of walksheds from Grantville Station, taking into account the proposed trail alignment and circulation system, is also shown. The process undertaken to assess the range of alignment alternatives and arrive at the preferred routes is discussed below.

FIGURE 5-6 PROPOSED TRAIL AND CONNECTIVITY IMPROVEMENTS WITH WALKSHEDS FROM GRANTVILLE STATION



PROMENADE

The Promenade provides a public, multi-modal, neighborhood connection aligned with existing (private) vehicular pathways, currently within a single ownership. As a condition of development of the site with higher intensity uses and in accordance with the Community Plan, the Promenade could be developed as a vehicular/pedestrian/bikeway connection, potentially remaining in private ownership, but with a public easement. This connection would be vital given the extensive number

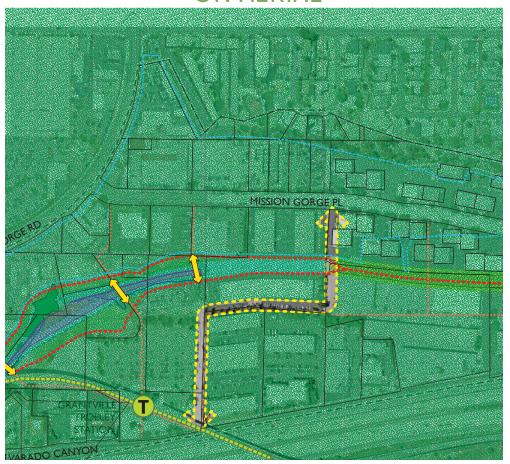
of new housing units planned at the site where the Promenade is located, as well as the Study Area in general. This would enable residents of these units greater ability to connect to existing roadways and move within the Study Area, and access Grantville Station and Alvarado Creek. The connection would also be necessary for emergency vehicles to easily access all parts of the Study Area from the north or the south.

The southern portion of the Promenade is situated on an already-existing private roadway (Figure 5-7). Where the roadway ends, the Promenade would turn eastbound along what now is an existing surface parking lot. The Promenade would then turn northward, onto an existing private road, which extends over the underground triple RCB culvert in Reach 4, and connects to Mission Gorge Place.

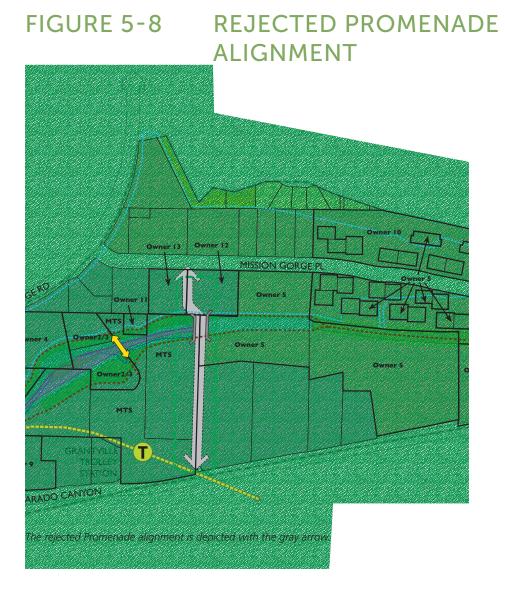
REJECTED ALIGNMENT

Initially, the Promenade was contemplated as extending in a straight north-south direction over the creek, approximately where Reach 2 meets Reach 3 (Figure 5-7). However, the construction of a vehicular bridge over the creek was determined to not be feasible for several reasons, and thus did not meet the Study's feasibility criteria. As a result, a Promenade that provided a north-south connection without requiring a creek crossing was explored and chosen.

FIGURE 5-7 PROMENADE ALIGNMENT ON AERIAL



The Promenade alignment is depicted with the yellow arrow.



PRIMARY TRAIL SYSTEM

The feasible Primary Trail System through the Study Area is described, by reach. Its purpose is to provide a multi-use path within the creek corridor, as specified in the Navajo Community Plan. This Multi-Use Trail system can transform Alvarado Creek into a linkage for the Study Area and provide the possibility for future connections to the San Diego River pathway and the Navajo Canyon Trail, as recommended in the San Diego Park Master Plan. In addition, three pedestrian/bicycle bridges would enhance north-south connectivity within the Study Area, increasing connections to Grantville Station, particularly from points north of the creek. This has the potential to be particularly important to the new residents planned for the area.

CREEK CROSSINGS

Three pedestrian/bicycle creek crossings are proposed in the preferred trail design, per the Grant-ville supplemental development regulations.⁸ These are located with the following principles in mind:

- Provide creek crossings every 800 feet or less
- Locate the crossings to enhance connections to Grantville Station and other transit stops, strengthen connections to surrounding land uses, and to generally improve north-south connectivity in the Study Area
- Minimize the required bridge length, and thus, cost
- Avoid impacting higher quality, sensitive vegetation communities and wetlands
- Locate crossings along property lines
- Avoid interfering with existing utilities and structures

The length of the pedestrian/bicycle bridges will depend on their design. To provide an approximate sense of their length, the westernmost pedestrian and bicycle bridge, located where Reach 1 intersects with Reach 2, could extend between approximately 50 and 100 feet in length, with intermediate structural supports. A range is provided with the minimum distance having the landing right at the existing top of bank on the north side, which would be inundated during larger storms. Importantly, this bridge is located where the channel narrows substantially. In addition, this crossing location provides pedestrian and cyclists from Mission Gorge Road/Fairmount Avenue and other points to the north of the Study Area a way to cross the creek and access Grantville Station.

The central pedestrian/bicycle bridge, located squarely in Reach 2, could extend between 70 and 110 feet, with intermediate structural supports provided. Like the bridge to the west, this bridge is also located where the channel narrows, and where higher quality vegetation is not present. It is also located to provide direct access to Grantville Station from points north, including Mission Gorge Place.

The easternmost pedestrian/bicycle bridge, also located within Reach 2, could extend approximately 120 feet or greater. This proposed crossing location is located along parcel lines, away from higher quality vegetation. It also provides access to Grantville Station from the northern portion of the Study Area, and provides a direct connection to the Promenade from the northwest portion of the Study Area.

MULTI-USE TRAIL SYSTEM

Mission Gorge Road

The San Diego River Park Master Plan recommends providing a pedestrian connection under or over the bridge at Mission Gorge Road, to provide access to the San Diego River Pathway from Alvarado Creek. It is not feasible to route pedestrians belowgrade, to provide access to the San Diego River. There are numerous concerns with this approach, including cost, safety, and ADA accessibility.

It also does not appear feasible to provide a pedestrian crossing on Mission Gorge Road/Fairmount Avenue where it crosses Camino Del Rio North. Currently, there is no pedestrian crossing on this heavily-trafficked, seven-lane road, which provides access to I-8. The addition of an at-grade pedestrian facility at this location has not previously been studied, and this roadway operates at the lowest level of service (F).9

The realignment of Alvarado Canyon Road presents an opportunity for a pedestrian connection to the San Diego River Pathway, at-grade, across Mission Gorge Road/Fairmount Avenue. The Preferred Design contemplates that the eventual connection to the San Diego River Pathway could be made as a part of this Capital Improvement Project.

Reach 1

The Primary Trail System in Reach 1 is composed of two elements: The Multi-Use Trail serving pedestrians and bicyclists and a pedestrian/bicycle bridge. The Multi-Use Trail is shown directly adjacent to and north of the existing channel. Although the supplemental development regulations of the Navajo Community Plan indicate that the Multi-Use Trail is to be provided along both sides of the creek

corridor, the Multi-Use is shown only adjacent to a portion of the area south of Reach 1, due to site constraints.

Reach 2

To the north of the channel in Reach 2, the Preferred Design shows the Multi-Use Trail aligned on the periphery of the Floodable Green Space and Wetland Buffer (see Revitalization Study, Figure 3-4). This serves two purposes. First, it expands the hydraulic capacity of the channel. Second, it furthers the goal of transforming the creek into an amenity by aligning the trail in a way that allows community members to interact with the natural environment in their day-to-day lives, while navigating through the city. There is also a second option for the trail alignment north of the channel; this alignment locates the Multi-Use Trail adjacent to the channel, in the middle of the fill pad. In this option, the Multi-Use Trail would remain usable during large storm-events.

To the south of the channel, the Multi-Use Trail is aligned at outer edges of the Floodable Green Space, in the western portions of the reach. In the eastern portion of Reach 2, where there is no Floodable Green Space, the Multi-Use Trail is shown directly adjacent to the channel (see Revitalization Study, Figure 3-5). The Multi-Use Trail will need to climb many feet within this reach; the precise trail alignment would need to be further refined to meet ADA requirements.

⁸The Navajo Community Plan states, "Provide the opportunity for a pedestrian/bicycle bridge over Alvarado Creek, which will allow any development projects adjacent to Alvarado the creek a critical linkage to the Grantville Trolley Station." (UD-10)

⁹Grantville Focused Plan Amendment Final Environmental Impact Report, April 2015.

Reach 3

In Reach 3, the Multi-Use Trail to the north of the creek is located within the channel; the trail would experience flooding during larger storm events (see Revitalization Study, Figure 3-7). To the south of the channel, the Multi-Use Trail is located outside of and adjacent to the channel. This is a result of the site topography.

Reach 4

In Reach 4, the northern and southern branches of Reach 3's Multi-Use Trail converge into one within the proposed Recreational Corridor, sited on top of the culvert system (see Revitalization Study, Figure 3-9).

Reaches 5 and 6

In reaches 5 and 6, the Multi-Use Trail extends immediately north of the creek. Due to the freeway, there is insufficient space south of the channel to provide a 10-foot-wide trail there.

The Study Team determined that the Multi-Use Trail could extend, in Reach 6 and beyond, along a San Diego Gas & Electric easement, in order to create a potential future connection to Navajo Canyon Trail. In the alternative, a connection can be made to Navajo Canyon Trail via Mission Gorge Place to Alvarado Canyon Road (Figure 5-9). In order for the Multi-Use Trail to connect from the easement to Navajo Canyon Trail, further study would be needed regarding how to make the connection from where the trail terminates in the Study Area to where the protected crossing is located at Waring Road and Adobe Falls Road (Figure 5-10).

SECONDARY ACCESS WAY AND TRAIL NETWORK

The secondary trail system provides an additional north-south and east-west pedestrian and bicycle network in the Study Area. It proposes connections from the primary trail network to Grantville Station, existing roadways, and land uses within and outside of the Study Area. The mapped alignment of these pathways is conceptual, and would likely need to be reassessed as development is proposed. It is envisioned that as development occurs, a more robust network of vehicular and pedestrian and bicycle access ways would be constructed, in conformance with the Navajo Community Plan, which provides as follows:

SDR 4. All new development shall provide a minimum of one vehicular access way through the project site. The layout of a private street or private drive should be in a grid pattern or modified grid pattern, emphasizing interconnected streets and the ability to reach local destinations through multiple routes. It is desirable to have streets with block faces of 400 feet in length or less. Private streets or private drives shall be coordinated and connected to the public street system. Fencing, walls, or gates that limit access are prohibited. Where possible, streets shall frame vistas of the mixed-use core, Grantville Trolley Station, San Diego River, and Alvarado Creek.

SDR 5. All new development shall provide a minimum of one pedestrian and bicycle access-way through the project site. Pedestrian and bicycle access-ways shall be coordinated and connected to public streets. Fencing, walls, and gates that limit access are prohibited. Sidewalks shall be separated from the street by landscaped parkways and shall be provided as follows:

a. Provide a minimum 5-foot landscaped parkway and minimum 5-foot non-contiguous sidewalk on at least one side of any private drive.

b. Provide a minimum 5-foot landscaped parkway and minimum 10-foot non-contiguous sidewalk along any public or private street.

FIGURE 5-9 MULTI-USE TRAIL, NAVAJO CANYON TRAIL CONNECTION VIA ALVARADO CANYON RD.



FIGURE 5-10 MULTI-USE TRAIL, EASEMENT CONNECTION TO NAVAJO CANYON



GRANTVILLE TROLLEY STATION/ALVARADO CREEK REVITALIZATION STUDY

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APPENDIX D HYDROLOGY BACKGROUND REPORT



Prepared By:

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1 Introduction

1.1 HYDROLOGY BACKGROUND REPORT SCOPE

The City of San Diego (City) is preparing a planning study to develop and evaluate scenarios for the portion of Alvarado Creek channel in the Grantville area, which is located north of the Grantville Trolley Station. This Hydrology Background Report was developed as part of the overall planning study and included hydrology and hydraulic analyses that were specifically performed in support of the key planning objective of formulating and presenting a design option that will increase the hydraulic capacity of Alvarado Creek to improve flooding.

The general approach to prepare this Hydrology Background Report was to first gain an understanding of the existing drainage systems and potential issues. Next, the acquired understanding of the drainage systems was used to formulate channel geometries that may improve hydraulic conveyance. These alternative geometries were then analyzed to quantify the increase in the hydraulic capacity that each option may provide. The final step of this approach included the preparation of this report that describes the methodology and results of the data collected, hydrologic, and hydraulic analyses. The Hydrology Background Report does not address detention, hydromodification management, or water quality.

The preparation of this report included the following scope:

- 1. Review City provided topographic data, historical aerial photographs, stream flow data, flood history, habitat data, water quality data, groundwater reports, geology reports, and relevant monitoring reports.
- 2. Conduct a LIDAR topographic survey and identify creek centerlines and culvert locations and prepare a topographic survey map including 1 foot contour data and an orthorectified aerial photograph.
- 3. Review survey data and identify different channel reaches based on geometry and material.
- 4. Utilize existing data to identify design storm flow rates.
- 5. Perform a hydraulic analysis to identify existing channel deficiencies.
- 6. Perform hydraulic analyses to determine the performance of proposed design alternatives.
- 7. Preparation of this Hydrology Background Report, which includes hydrologic and hydraulic methodologies, parameters, results, a description of existing channel deficiencies, an assessment of the performance of the proposed design alternatives, and a description of grading or other improvements required for each channel reach.

1.2 STUDY AREA DESCRIPTION

The Study Area consists of approximately 102 acres that encompasses the Grantville Trolley Station and several nearby properties. The Study Area is bound by Mission Gorge Road to the west, Waring Road to the east, Alvarado Creek Road to the south, and single-family residences to the north. Alvarado Creek transects the southern portion of the Study Area. A vicinity map that includes the planning Study Area boundary is shown on Figure 1-1. Enhancements to Alvarado Creek have the potential to reduce the risk of flooding, improve natural habitat, and provide various multiple benefits including additional green space and improvement to pedestrian connectivity (i.e., transform Alvarado Creek into an amenity). Thus, enhancements to Alvarado Creek may facilitate redevelopment of the properties along the creek and are consistent with the Navajo Community Plan, which was most recently amended in 2015 and seeks to promote housing and mixed-use development in a transitoriented setting. The Navajo Community Plan also encourages the implementation of new bicycle and pedestrian trails along open spaces. A revitalization of the Alvarado Creek is consistent with the San Diego River Park Master Plan, adopted May 20, 2013.

1.3 ALVARADO CREEK EXISTING CONDITIONS

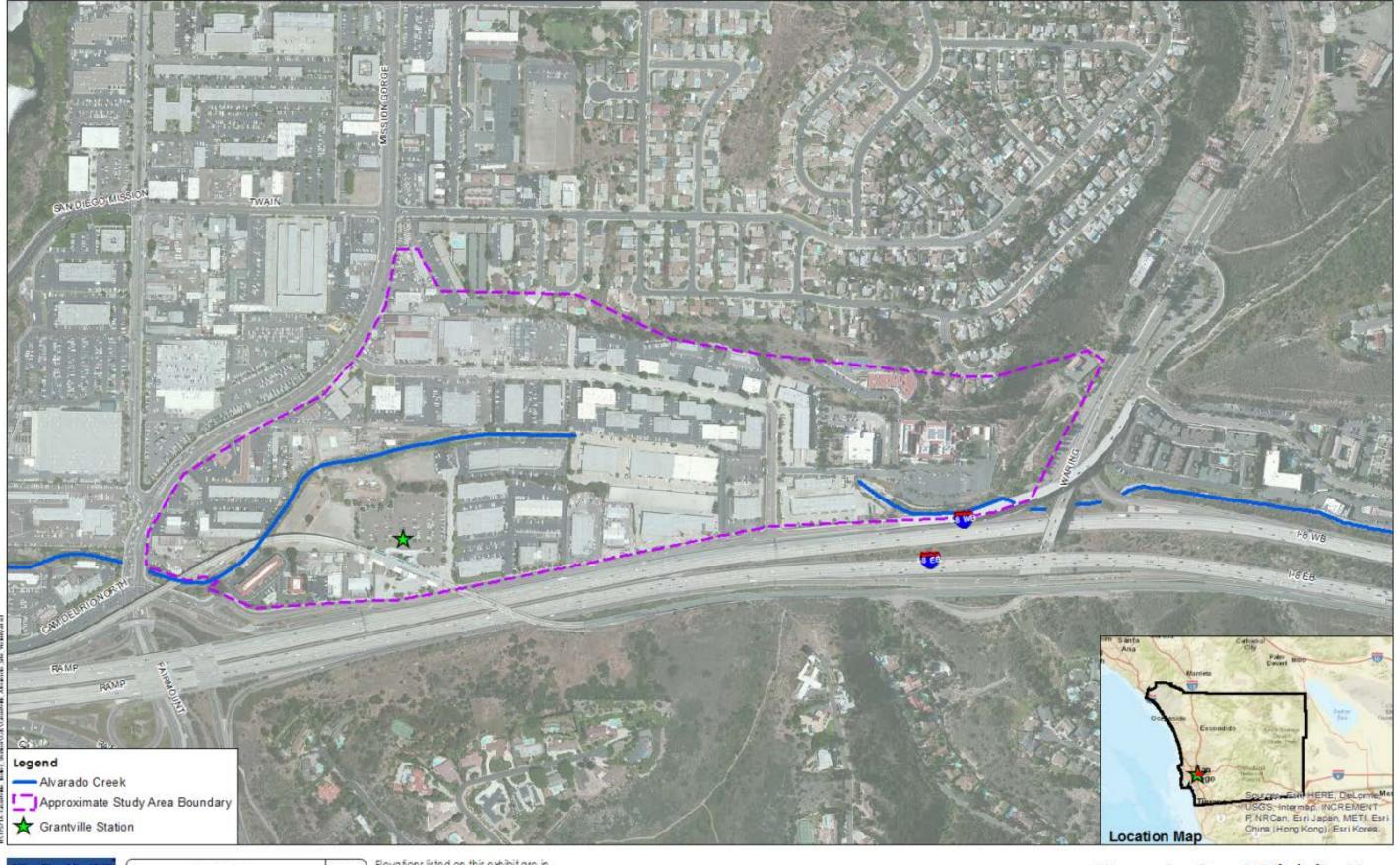
Alvarado Creek flows east to west through the Study Area. The portion of Alvarado Creek within the Study Area is located between Waring Road and Mission Gorge Road. At the upstream end of the Study Area, Alvarado Creek is conveyed underneath the southbound Waring Road onramp to Westbound I-8 in a triple 11-foot wide by 8-foot high Reinforced Concrete Box (RCB) culvert. The downstream end of this culvert is the upstream end of Alvarado Creek within the Study Area. From this point, Alvarado Creek flows west for approximately 4,400 feet before passing underneath Mission Gorge Road. Here, flows are conveyed underneath Mission Gorge Road through a triple RCB culvert, with each cell measuring 11-foot wide by 8-foot high. The upstream end of this culvert is the downstream end of Alvarado Creek, within the Study Area. The Alvarado Creek confluence with the San Diego River is approximately 1,200 feet downstream of Mission Gorge Road.

Within the Study Area, Alvarado Creek has three different types of channel lining. Near the downstream limits of the Study Area, Alvarado Creek alternates between a concrete lined, trapezoidal channel and an earthen, trapezoidal channel. In the central portion of the Study Area, Alvarado Creek is conveyed underground through a triple 11-foot wide by 8-foot high RCB culvert that is approximately 1,340 feet long. A concrete lined, trapezoidal channel extends for approximately 175 feet east of the long RCB culvert. Alvarado Creek, near the upstream limits of the Study Area, consists of an earthen bottom with concrete lined sides, trapezoidal channel.

The unlined (earthen) sections of Alvarado Creek have established native riparian vegetation as well as non-native wetland vegetation (e.g., Arundo). Seven sensitive vegetation communities have been identified within Alvarado Creek as part of the overall Study. These areas are protected by the California Department of Fish and Wildlife (CDFW), the San Diego Regional Water Quality Control Board (SDRWQCB), and the United States Army Corps of Engineers (USACE).

Densely developed, low-lying areas are located along either side of Alvarado Creek. Along the north bank, the existing grade is approximately 7 to 8 feet above the flow line of the channel. Throughout the western half of the Study Area, low lying areas extend approximately 400 feet north of the channel. Land use within this low lying area consists of commercial and industrial buildings, parking lots, and a portion of Mission Gorge Place. A large portion of the western half of the Study Area is located within a Flood Emergency Management Administration (FEMA) mapped Special Flood Hazard Area (SFHA). That is, north of the open channel portion of Alvarado Creek and west of the long RCB culvert located near the center of the Study Area, the mapped SFHA, or 100year floodplain, generally extends north of the channel approximately 400 feet. In the central portion of the Study Area, Alvarado Creek is conveyed underground, many properties between Alvarado Creek and Mission Gorge Place are located within the 100-year SFHA. Along the south bank, the existing grade is approximately 11 to 15 feet above the flow line of the channel. Due to the higher elevation, the majority of properties along the southern bank of Alvarado Creek are located outside of the FEMA mapped floodplain. South of the channel, only a low-lying area near the Grantville Trolley Station is included in the SFHA. Applicable Flood Insurance Rate Maps (FIRMs) showing the Study Area are included in Attachment 1.

FIGURE 1-1 VICINITY MAP







Elevations listed on this exhibit are in reference to the NAVD 88 datum

Date of Exhibit: 6/27/2017 Digital Globe Aeria I Image: 04/2013

Alvarado Creek Vicinity Man

2 Data Collection

The data collection for this Study included performing a topographic survey of the Study Area, reviewing as-built drawings, and conducting a site visit. In the context of the Hydrology Background Report, the purpose of the data collection was to acquire, filter, and organize information related to the Study Area in support of preparing hydraulic modeling of the existing condition. The results of these modeling efforts were evaluated to determine areas where hydraulic capacity is restricted and where improvements will provide increased capacity. The collected data was also used to create base geometry in which proposed channel geometries were overlaid.

2.1 TOPOGRAPHIC SURVEY

Topographic aerial imagery and survey data were collected by Rick Engineering Company (RICK) on June 29, 2016 and July 14, 2016, respectively. The horizontal basis of coordinates for this survey is referenced to the California Coordinate System of 1983 (CCS83) Zone 6, North American Datum of 1983 (NAD83), and elevations are referenced to the National Geodetic Vertical Datum of 1929 (NGVD 29). Data from the surveys were compiled into computer aided drafting and design (CADD) files that included ground elevation data as well as key features, such as storm water conveyances (e.g., channels flowlines, culverts, etc.).

2.2 CITY OF SAN DIEGO REVIEW OF AS-BUILT PLANS

As-built plans for the Study were obtained from the City of San Diego Development Services Department (DSD). These plans were used to obtain data related to the horizontal and vertical alignment of Alvarado Creek. As-built plans were also used to determine constraints impacting proposed alternative designs. A summary of the as-built plans reviewed is provided in Table 2-1.

TABLE 2-1: SUMMARY OF CITY OF SAN DIEGO AS-BUILT DRAWINGS REVIEWED

As-Built Drawing	Construction Completion Date	Relevant Data Obtained	Datum¹
18412-D	6/1/1980	Downstream portion of triple RCB culvert in Reach 4	MSL, City of San Diego
19862-D	11/10/1988	Upstream portion of triple RCB culvert in Reach 4, Reach 5, Reach 6	MSL, City of San Diego
10116-D	4/28/61	Right Bank, Reach 2	MSL, City of San Diego
21647-D	7/2/1984	Reach 1	MSL, City of San Diego
16540-D	3/17/1976	Reach 3	MSL, City of San Diego
10569-AD	9/1/1973	Sewer Line	MSL, City of San Diego

Notes:

¹MSL is mean sea level, and this datum is synonymous with the NGVD 29 datum.

TABLE 2-2: FEMA SUMMARY OF BASE FLOOD ELEVATIONS FOR ALVARADO CREEK WITHIN THE GRANTVILLE TROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY AREA

Cross Section	Return Period Storm Event	Base Flood Elevations (Feet NGVD 29) ¹	Base Flood Elevations (Feet NAVD 88) ¹
	10-Year	70.5	72.6
В	50-Year	75.4	77.5
	100-Year	75.6	77.7
	10-Year	74.1	76.2
С	50-Year	75.8	77.9
	100-Year	76.0	78.1
	10-Year	77.9	80.0
D	50-Year	78.4	80.5
	100-Year	78.5	80.6
	10-Year	89.6	91.7
E	50-Year	93.8	95.9
	100-Year	94.6	96.7
	10-Year	93.6	95.7
F	50-Year	95.7	97.8
	100-Year	96.4	98.5

Notes:

1. Add 2.1 feet to NGVD 29 elevations to obtain NAVD 88 elevations.

2.3 FLOOD INSURANCE RATE MAP REVIEW

The Flood Emergency Management Administration (FEMA) Flood Insurance Rate Maps (FIRMs) for Alvarado Creek were reviewed (2016 FIS, Panels 06073C1636H, 06073C1637H, 06073C1638H and 06073C1639H; Cross Sections B-F). These documents describe the Base Flood Elevations (BFEs) for Alvarado Creek in the Study Area. Table 2-2 provides a summary of the BFEs for the return periods listed in the FIRM. The cross section locations are shown on the applicable pages from the FIRM for Alvarado Creek, which are included in Attachment 1.

Topographic survey data was reviewed in order to determine the relevance of the FIRM data to the Study Area. It should be noted that the topographic survey obtained as part of this Study is reference to the NGVD 29, whereas the FIRM is referenced to the North American Vertical Datum of 1988 (NAVD 88). Per Table 12 of the Flood Insurance Study, San Diego County, California and Incorporated Areas, Volume 2 of 11, dated April 5, 2016, the conversion between the two datums is +2.1 feet. The elevations discussion throughout this report are referenced to the NGVD 29 datum, unless specific to FEMA BFE data (e.g., Column 4 in Table 2-2).

Based on the topographic survey data obtained as part of this Study, facilities adjacent to the north side of Alvarado Creek range in elevations between approximately 70.5 feet NGVD 29 at the downstream end (near FEMA Cross Section B), approximately 81 feet NGVD 29 near the middle of

the Study Area (near FEMA Cross Section D), and 105.0 feet NGVD 29 the upstream end (near FEMA Cross Section F). Facilities adjacent to the south side of Alvarado Creek range in elevations between 76.0 feet, 81.0 feet, and 98.0 feet NGVD 29 for the similar downstream, central, and upstream locations, respectively. This is consistent with the FEMA FIRMs, which show the 100-year floodplain extending 250 to 400 feet north of Alvarado Creek for the majority of the Study Area.

2.4 SITE VISIT

A field site visit was performed on May 20, 2016 to determine the Manning's Roughness Coefficients, channel conditions, and channel properties. Alvarado Creek photos from the May 20, 2016 site visit are included in Attachment 2. Manning's Roughness Coefficients for the creek were determined based on field observations and to be consistent with Table 1-104.14A of the City of San Diego Drainage Design Manual, dated April 1984. Manning's Roughness Coefficients for urban areas were determined based on typical values using the FEMA model for the region. A Manning's Roughness Coefficient of 0.018 was used to represent concrete channel bottoms and side slopes. A Manning's Roughness Coefficient of 0.1 was used to represent dense vegetation along earthen bottomed sections of the channel. Parking lots and other areas of asphalt pavement were represented using a Manning's Roughness Coefficient of 0.045.

3 Hydrology

3.1 HYDROLOGIC ANALYSIS METHODOLOGY

Hydrologic data relevant to Alvarado Creek and the Study Area was obtained from the *Flood Insurance Study, San Diego County, California and Incorporated Areas, Volume 1 of 11* (FIS), prepared by FEMA, dated April 5, 2016. The referenced FIS provided 10-, 50- and 100-year frequency peak flow rates, which were then plotted on log-probability paper in order to determine the flow rate distribution. From this distribution, peak flow rates were determined for the 2-, 5- and 25-year design storm events.

In order to gain a better understanding of the watershed associated with the Study Area, the watershed was delineated. The overall watershed tributary to Alvarado Creek at Mission Gorge Road was delineated using the U.S. Geological Survey's (USGS) StreamStats 3.0 Program. This program delineated watersheds to a user specified location

using USGS survey data. A simplifying and conservative assumption was made that the total peak flow enters the Study Area at the upstream end of the Study Area. Therefore, contributing flows from local watersheds within the Study Area were not quantified separately and the local watersheds were not delineated.

3.2 HYDROLOGIC ANALYSIS RESULTS

According to the FEMA FIS, the Alvarado Creek watershed at the confluence with the San Diego River is 14 square miles (8,960 acres). The peak design storm flow rates obtained from the FEMA FIS are shown in Table 3-1.

According to the USGS StreamStats 3.0 program, the Alvarado Creek watershed at Mission Gorge Road is approximately 13.7 square miles (8,737 acres). This location is approximately 1,200 feet upstream from the confluence of the San Diego River. The StreamStats generated watershed is consistent with the watershed listed in the FEMA

FIS referenced in Section 2.0. The Alvarado Creek watershed extends from the Grantville Area approximately seven miles east to the Grossmont Area and generally consists of developed urban land uses. The majority of the uses within the watershed are residential, including multi-family and single-family housing. Residential uses combined with road rights-of-way account for more than half of the uses in the watershed. The watershed also includes a variety of other land uses including commercial, industrial, and open space. Lake Murray is located near the center of the watershed.

3.3 LOCAL DRAINAGE

Runoff from the Study Area is conveyed directly into Alvarado Creek. Typically, sheet flows from parking lots and rooftops is conveyed via trench drains into existing catch basins located at the corners of parking lots. These catch basins discharge directly to Alvarado Creek via existing underground storm drains. Based on a review of the available as-built drawings, underground storm drain pipes that discharge directly to Alvarado Creek within the Study Area range in size from 18 to 36 inches in diameter. This Hydrology Background Report is focused on identifying and correcting deficiencies with the existing design of Alvarado Creek, not the local storm drain network. Therefore, an in-depth analysis of the storm drain network within the Study Area was not performed as part of this Study.

TABLE 3-1: SUMMARY OF PEAK FLOW RATES

Storm Event	Flow Rate (cfs)	Source
2-Year	1,180	Interpolated
5-Year	2,050	Interpolated
10-Year	2,700	2016 San Diego FIS
25-Year	3,800	Interpolated
50-Year	4,500	2016 San Diego FIS
100-Year	5,100	2016 San Diego FIS

4 Hydraulic Analysis Existing Conditions

TABLE 4-1: SUMMARY OF REACH DESIGNATIONS

Reach	Channel Type	Bottom Width (feet)	Approximate Length (feet)
0	Triple RCB	3 X 11 = 33	128
1	Concrete Trapezoidal	29.5	550
2	Vegetated	13 to 16	1,100
3	Concrete Trapezoidal	29.5	550
4	Triple RCB	3 X 11 = 33	1,340
5	Concrete Trapezoidal	45	175
6	Concrete & Vegetated Trapezoidal	44.5	550

4.1 METHODOLOGY

Hydraulic analyses were performed for existing conditions to determine the capacity of the channel and culverts. The U.S. Army Corps of Engineers' Hydrologic Engineering Center's River Analysis System (HEC-RAS) v 4.10 software was used to perform the analyses. The HEC-RAS software is designed to perform one-dimensional hydraulic calculations for steady or gradually-varied flow in natural and constructed channels. The HEC-RAS software was used to calculate the water surface elevation (WSE). Hydraulic modeling using HEC-RAS was performed to quantify WSEs and the limits of flooding for the exiting conditions for Alvarado Creek. The HEC-RAS modeling was performed using a subcritical flow regime for the existing conditions using the 100-year frequency design storm (5,100 cubic feet per second). In addition to the 100-year frequency, the peak flow rates for various other storm event frequencies were analyzed in order to estimate the maximum hydraulic capacity of culverts and reaches (i.e., performed to determine the storm event frequency that results in overtopping of channel banks).

Channel Reaches

For the purposes of this Report, the Alvarado Creek was divided into seven reaches (referred to as six reaches and Mission Gorge Road in the Revitalization Study). The culvert at Mission Gorge Road is identified as Reach 0, and the portion of Alvarado Creek within the Study Area is identified as Reaches 1 through 6 moving from downstream to upstream. Reaches were determined by sections of the creek having consistent geometries and compositions. A summary of the reaches and associated channel types is shown in Table 4-1, and the extent of each channel reach is shown on Figure 4-1. The HEC-RAS model extended approximately 600 feet downstream from the triple RCB culvert underneath Mission Gorge Road. The downstream boundary condition for the HEC-RAS model was based on normal depth calculations.

Cross sections data were obtained using a HEC-GeoRAS, which is a utility for processing geospatial data. The location of each cross section was selected to appropriately capture changes in the channel geometry. Essentially a cross section was selected

near upstream and downstream limits of each reach. The HEC-GeoRAS tool was then used to extract topographic data for each cross section for use in the HEC-RAS hydraulic modeling software. A hydraulic map showing the locations of the cross sections that were incorporated into the hydraulic modeling is provided in Attachment 4.

Below is a summary of the existing conditions at each reach:

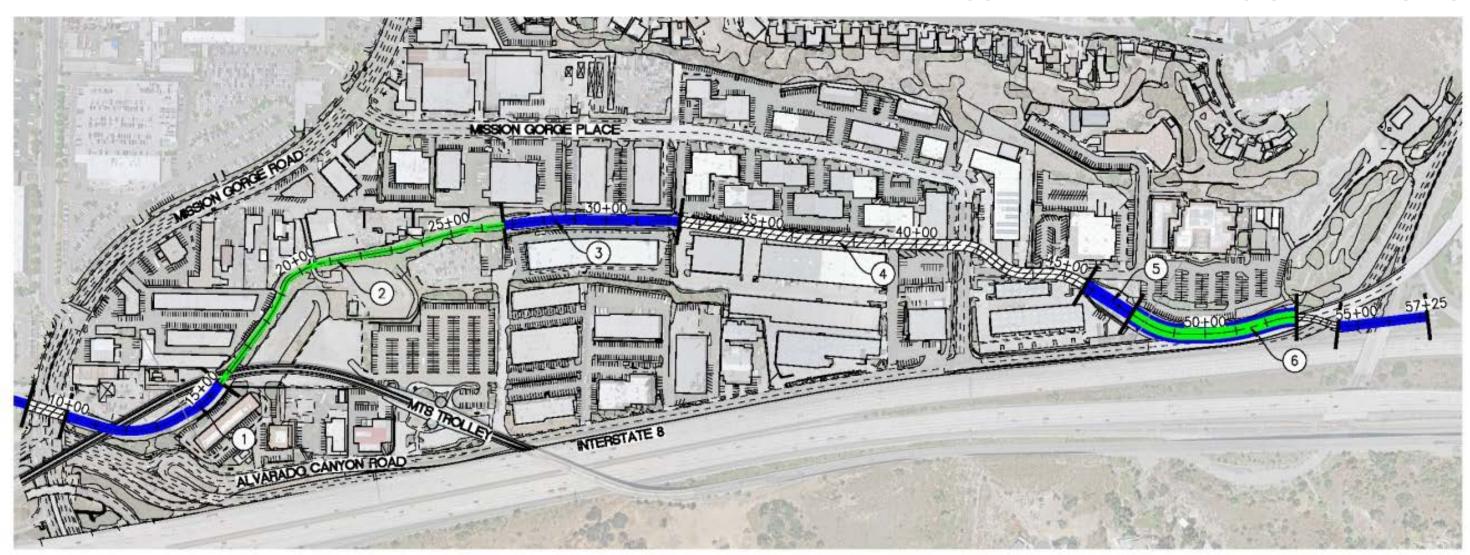
REACH 0 (MISSION GORGE ROAD) – TRIPLE RCB CULVERT:

This reach consists of 11 feet wide by 8 feet high RCB culvert and a concrete lined trapezoidal channel. This reach extends approximately 600 feet downstream of the triple 11 feet wide by 8 feet high RCB underneath Mission Gorge Road. The channel has a bottom width of 29.5 feet and has a depth of 15 feet with 1.5 to 1 (horizontal to vertical) side slopes.

REACH 1 – CONCRETE LINED CHANNEL:

A concrete lined, trapezoidal channel extends approximately 550 feet from the upstream end of the triple 11-foot wide by 8-foot tall RCB underneath Mission Gorge Road to a point underneath the MTS Trolley Line where Alvarado Creek transitions to an earthen bottomed channel. Per As-Built Sheet 21647-D prepared by Simpson Engineering, dated July 2, 1984, and as verified with topography survey data the channel has a bottom width of approximately 29.5 feet. The banks are concrete with a slope of approximately 1.5 to 1 (horizontal to vertical). The north bank is approximately 8 feet in height. The south bank height varies but is generally approximately 12 feet.

FIGURE 4-1 ALVARADO CREEK REACHES





OPEN CHANNEL - CONCRETE

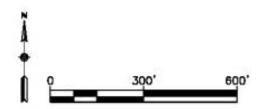
OPEN CHANNEL - VEGETATED

OPEN CHANNEL - VEGETATED BOTTOM, CONCRETE SIDES

REINFORCED CONCRETE BOX CULVERT

1) CHANNEL REACH DESIGNATION BY THIS STUDY

20+00 CHANNEL STATION DESIGNATION (FEET) BY THIS STUDY)



REACH 2 – EARTHEN CHANNEL:

This reach is an earthen, trapezoidal channel with a length of approximately 1,100 feet. The bottom width varies between approximately 13 and 17 feet. Similar to Reach 1, the north bank is approximately 8 feet in height, and the south bank height varies but is generally approximately 12 feet. The bank slopes are generally approximately 2 to 1 (horizontal to vertical). Near the middle of this reach, a small paved access road crosses the channel, functioning as a grade control structure. Downstream of the access road, there is a rapid drop in elevation of approximately 0.5 feet over approximately 20 feet. The vegetation downstream of the access road is mature and consists of palm trees intermixed with riparian woody vegetation. The vegetation upstream of the access road consists primarily of tulles vegetation with some riparian vegetation. At the upstream limits of this vegetated reach, the channel becomes wider as it transitions to a concrete channel.

REACH 3 – CONCRETE LINED CHANNEL:

This reach is a concrete lined trapezoidal channel. This reach extends approximately 550 feet from the upstream end of Reach 2 to the start of the triple 11-foot wide by 8-foot tall RCB culvert underneath the central part of the Study Area. Pursuant to As-Built Sheet 16540-D, prepared by Rick Engineering Company, dated March 17, 1976, and as verified with topographic survey data the channel has a bottom width of approximately 29.5 feet. The side slopes have a height of approximately 11 feet and a slope of approximately 1.5 to 1 (horizontal to vertical).

REACH 4 - TRIPLE RCB CULVERT:

This reach consists of a triple 11-foot wide by 8-foot high RCB culvert. This culvert extends approximately 1,340 feet underneath the central part of the Study Area. Based on a review of the asbuilt drawings, this culvert was constructed as part of two separate projects. As Built Plan 18412-D, prepared by the City of San Diego and dated April 19, 1979, shows that the downstream or westernmost 900 feet of the culvert was constructed first. This portion of the culvert is nearly straight with only minor angles. As Built Plan 19862-D, prepared by Deacon Associates and prepared March 9, 1981, shows that the upstream 440 feet of culvert was constructed later. This section of the culvert includes three approximately 45-degree angles in order to align the culvert underneath an existing access road.

REACH 5 – CONCRETE LINED CHANNEL:

A concrete lined trapezoidal channel extends approximately 175 feet upstream from the triple 11-foot wide by 8-foot tall RCB culvert to a point where Alvarado Creek changes to an earthen bottomed channel. Per As Built Sheet 19862-D, prepared by Deacon Associates, dated March 9, 1981, and as verified by topographic data, this reach has bottom width of approximately 44.5 feet and approximately 1.5 to 1 (horizontal to vertical) side slopes with heights of approximately 7 feet. A 60-foot long warped wingwall with an approximately 5 degree taper transitions the open channel into the triple RCB culvert at the downstream end of the reach.

REACH 6 – EARTHEN BOTTOM, CONCRETE SIDE SLOPE CHANNEL:

This reach consists of an earthen bottomed and concrete lined side slopes, trapezoidal channel. This reach extends approximately 480 feet upstream to a triple 11-foot wide by 8-foot tall RCB culvert located underneath the southbound Waring Road onramp to Westbound I-8. Per As Built Plan 19862-D, prepared by Deacon Associates, dated March 9, 1981, and as verified by topographic data, this reach has a base width of approximately 44 feet and 1.5 to 1 (horizontal to vertical) concrete side slopes with heights of approximately 9 feet.

4.2 EXISTING CONDITION RESULTS

The results of the hydraulic modeling indicate that the Alvarado Creek, in the existing condition, does not have adequate capacity to convey the 100-year frequency design storm event through the Study Area without significant flooding. Reach 2 due to having a narrow cross section and dense vegetation is the most restrictive reach within the Study Area and does not have adequate capacity to convey the 2-year frequency design storm. In general, the concrete lined reaches, without the effects of downstream restrictions, have the capacity to convey approximately the 10-year frequency design storm event, and the culverts have the capacity to convey approximately the 25-year frequency design storm event. For ease of discussion, the existing hydraulic capacity conditions of Reaches 0 through 6, as identified by this Study, are discussed individually below:

REACH 0 (MISSION GORGE ROAD) – TRIPLE RCB CULVERT:

Events in excess of the 25-year frequency storm event will exceed the capacity of the Mission Gorge Road culvert and cause a backwater condition immediately upstream (potential flooding). The backwater condition caused by the Mission Gorge Road triple RCB geometry during the 50-year frequency storm event will result in a water surface elevation above the northbound Mission Gorge Road sidewalk and pavement surface, but not the southbound pavement surface. The 100-year frequency storm event will result in an additional approximately 1.5 feet of water depth in comparison to the 50-year storm event and thus will result in the complete inundation of Mission Gorge Road, adjacent Alvarado Canyon Road, as well as extensive flooding of the area north of Alvarado Creek and east of Mission Gorge Road.

REACH 1 – CONCRETE LINED CHANNEL:

The backwater effects from the Mission Gorge Road culvert result in water depths significantly in excess of the north channel bank along Reach 1 during the 25-, 50-, and 100-year frequency design storm events. The hydraulic modeling shows that the 10-year frequency design storm event results in a channel depth of approximately 8.5 feet, which would overtop the north bank in some areas (bank is as low as 7.5 feet above the channel bottom) but flooding from the channel would be limited to the upland areas along bank (i.e., only extend approximately 10 to 20 feet outside of the channel bank). Without the backwater effect caused by the Mission Gorge Culvert (i.e., if the culvert was improved to have adequate capacity), the adjacent concrete trapezoidal channel (Reach 1) would flow at a depth of about 10 feet for the 100-year event. As previously stated, some of the top of bank elevations along the north side of this reach have a height of approximately 7.5 feet, and thus there would still be flooding along this reach for the 100-year event if the downstream culvert was improved without also implementing improvements to Reach 1, albeit significantly less flooding compared to the same event without improving the capacity of the Mission Gorge Road culvert.

REACH 2 - EARTHEN CHANNEL:

The 1,100 feet of vegetated channel (Reach 2) has by far the least hydraulic capacity of the entire channel system within the Study Area. The 2-year return period storm event flow will cause some flooding through this area. The degree of flooding during smaller storms, such as the 2-year event, is related to the resistance to flow imposed by

the vegetation. Or in other words, heavy vegetation will result in slightly more flooding compared to the maintained condition. Assuming regularly maintained vegetation, the reach will have minor flooding along the north side of the channel for small events. The 5 year and greater flows will cause flooding along the north side of the channel regardless of the maintenance condition of this reach. Flows in excess of about the 25-year event will result in flooding along the south side of the channel along the reach in areas where elevations are relatively low. The main factors contributing the low hydraulic capacity of this reach include the narrow bottom width, low bank height (north side), and fairly flat topography along the channel (north side).

REACH 3 – CONCRETE LINED CHANNEL:

Reach 3 has a similar channel bottom width and composition as Reach 1. If the downstream channel reaches were to be improved to eliminate the backwater condition during the 100-year flow, Reach 3 would flow at a depth of approximately 10 feet, which means the flow would be contained in the channel in the upper approximately 350 feet and over top the north bank of the lower 200 feet of the reach. Under the current conditions, the elevated flow depths caused by the Reach 2 backwater condition will result in elevated water depths along Reach 3 and flooding north of the channel during 25-, 50-, and 100-year flows.

REACH 4 - TRIPLE RCB

The approximately 1,340 feet long triple RCB culvert located near the middle of the Study Area has adequate capacity to convey up to the 25-year return period storm event flow. The 50- and 100year flows will result in water overtopping the upstream culvert inlet. During these larger storm events, flows not conveyed within the culvert will travel west down the access road, onto Mission Gorge Place, and continue down Mission Gorge Place before finding the downstream open channel before Mission Gorge Road. There is a sump along Mission Gorge Place, located approximately 600 feet east of Mission Gorge Road, and water will remain within the sump area of Mission Gorge Place until the water in the channel subsides allowing the sump to drain. Hence, the 50- and 100year return period event flows that over top the culvert inlet will cause much flooding of the immediate access road and a significant portion of Mission Gorge Place; essentially Mission Gorge Place between the alignment of the culvert and abovedescripted sump.

REACH 5 – CONCRETE LINED CHANNEL:

The concrete channel directly upstream of the Reach 3 triple RCB culvert would have adequate capacity to convey 100-year flow if the triple RCB culvert did not create a backflow condition during the larger events (i.e., if the triple RCP culvert was improved). In the existing configuration (with backwater) the 50- and 100-year flows will cause flooding along this channel reach upstream of the long triple RCB culvert.

REACH 6 – EARTHEN BOTTOM, CONCRETE SIDE SLOPE CHANNEL:

Assuming no backwater condition (downstream reaches improvement), the vegetated bottom, concrete side slope Reach 6 channel near the east Study Area boundary would have a hydraulic capacity adequate to convey between the 5-year and 25-year event depending on the condition of the vegetation (maintained verses unmaintained, respectively). For the no backwater condition, the 50- and 100-year flow will result in some flooding along each side of Reach 6 with the severity of flooding dependent on the vegetation condition at the time of the storm event. For the existing geometry scenario, the elevated 50- and 100-year flow depths at the inlet of the Reach 4 triple RCB culvert will result in flooding along both side of Reach 6; however, the steep topography (parking lot) on north side of the channel limits the extents of the flooding to approximately 80 feet outside the channel banks. On the south side of the channel, the slopes and walls associated with Highway 8 limit the extents of the flooding.

A hydraulic map showing the locations of the cross sections that were incorporated into the hydraulic modeling along with table of model output data are provided in Attachment 4.

5 Hydraulic Analyses Proposed Conditions

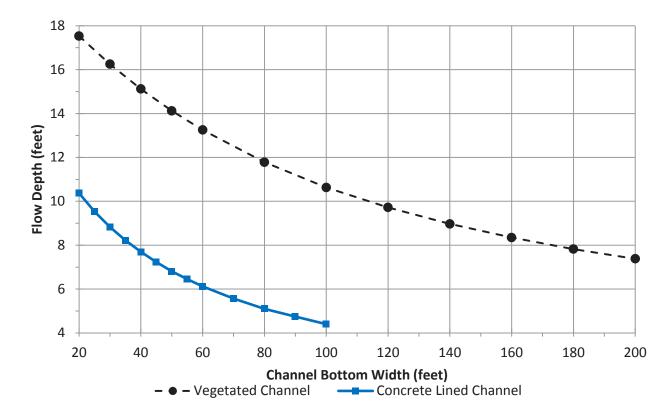
5.1 GENERAL APPROACH AND METHODOLOGY

The hydraulic analyses of the proposed condition included first formulating alternative designs, evaluating the alternative designs, and then preparing and analyzing a preferred alternative design. The existing condition hydraulic modeling results identified several locations with deficient hydraulic capacity. Alternative designs were formulated by proposing improvements that may enhance the hydraulic capacity at the locations where existing conditions deficiencies were identified. In order to estimate the open channel geometry that will be required to achieve the desired water surface elevations, normal depth calculations were performed. As feasible, the results of the normal depth calculations were incorporated into the proposed alternatives. Hydraulic modeling of each alternative was performed using the same approach as described for the existing conditions (see Section 4.1).

5.2 PRELIMINARY GEOMETRY DESIGN CALCULATIONS

Preliminary design calculations were performed to estimate the minimum channel widths associated with various channels. The results of these calculations were used, in combination with other data, such as site area constraints, to formulate proposed geometry for the alternative designs. Manning's Equation calculations, also referred to as normal depth calculations, were performed to determine the water depth necessary to convey the 100-year frequency design storm event flow rate of 5,100 cubic feet per second for various channel widths. Two channel linings were considered that included a fully concrete-lined channel and a fully vegetated channel. Manning Roughness Coefficient values of 0.018 and 0.1 were assumed for the concrete lined and vegetated channels, respectively. A side of 1.5 to 1 (horizontal to vertical) was assumed for the concrete and 3 to 1 (horizontal to vertical) for the vegetated. A longitudinal slope of 0.35 percent was assumed based on the approximate average longitudinal slope for Reaches

FIGURE 5-1 DEPTH VERSUS WIDTH, TRAPEZOIDAL CHANNEL



1, 2, and 3. Figure 5-1 shows the results of these preliminary calculations for the concrete lined and vegetated channels.

The results of the preliminary design calculations were used, in combination with other factors (explained below), to determine the appropriate channel width for the alternative designs. The concreted lined Reaches 1 and 3 have bottom width of approximately 30 feet each in the existing. Based on the stated assumptions, a concrete lined channel with a bottom width of 30 feet results in a channel depth of approximately 9 feet. In contrast, in order to achieve a depth of approximately 9 feet, a vegetated channel requires a bottom width of approximately 140 feet. This assumed vegetated channel geometry, 140-foot bottom width and

a 30-foot side slope on each side, would result in total channel width, bank top of slope to bank top of slope, of approximately 200 feet. Due to limited area available within the Study Area, a 200-foot wide vegetated channel would not be feasible, because there would not be adequate remaining area for other improvements, such as infrastructure, green space areas, and buildings. Therefore, based on the preliminary results, the proposed channel should consist of either fully concrete lined channel or a reasonable combination of concrete and vegetation (i.e., concrete on one side and vegetation on the other). The existing conditions along Reach 2 includes sensitive wetland vegetation communities, and thus proposing a fully lined concrete channel through Reach 2 would pose great challenges related to environmental permitting. When considering these other factors of site area constraints and environmental permitting in combination with the preliminary design calculations, channel improvements, particularly for Reach 2, must include a concrete lined portion to provide much of needed hydraulic conveyance and a vegetated portion to protect and enhance the existing wetland vegetation.

5.3 FORMULATION OF ALTERNATIVES DESIGNS

Specific alternatives were formulated based on the channel deficiencies, identified as part of the existing condition hydraulic modeling, and the results of the preliminary design calculation. Initially, three alternatives were formulated that included enhancements to Reaches 1, 2, and 3 as well as improvements to the RCB culverts at Mission Gorge Road and near the center of the Study Area. Hydraulic modeling of these alternatives were performed to verify that, in comparison to the existing conditions, the alternative designs will provide improved hydraulic capacity and reduced water surface elevations. The three alternative designs were presented to the public through a number of outreach workshops. The three alternative designs were also presented to various City departments. Based on a combination of feedback from the public outreach and coordination with City departments, a preferred alternative design was formulated. Hydraulic modeling was performed for the preferred alternative design and is presented in this Study.

Initial Alternative Designs

The Alternative Designs 1, 2, and 3 included proposing channel widening, a tiered floodable control green space within the creek corridor (approximately four feet above the channel bottom), and a multi-use trail that includes pedestrian and bicycle bridges along the creek corridor. A description of each alternative is detailed below.

ALTERNATIVE DESIGN 1

This alternative design includes the widening of channel bottoms in Reaches 1 through 3 and the addition of an underground culvert adjacent to the existing culvert in Reach 4. A floodable linear park with multi-use trails is proposed adjacent to the creek and pedestrian/bicycle bridges over the creek are proposed in Reaches 1 through 3. A recreational corridor is incorporated into the alternative design over the existing and proposed culverts along Reach 4. No changes are proposed to Reaches 5 and 6 in this alternative design.

ALTERNATIVE DESIGN 2

This alternative design consists of the widening of channel bottoms in Reaches 1 through 3 and the addition of an underground culvert adjacent to the existing culvert in Reach 4. Floodable pocket parks are included to the north and south of the Creek in Reaches 1 and 2. Multi-use trails are incorporated adjacent the creek, and pedestrian/bicycle bridges over the creek are proposed in Reaches 1, 2, and 3.

ALTERNATIVE DESIGN 3

This alternative design includes the widening of channel bottoms in Reaches 1 through 3 and the addition of a new open channel adjacent to the existing underground culvert in Reach 4. A large floodable park is included north of Reach 1, and a

large floodable park is incorporated south of Reach 2 and 3. Multi-use trails are proposed adjacent to the creek, and pedestrian/bicycle bridges over the creek are proposed along Reaches 1 through 4.

Initial Alternative Designs Results

Alternative Designs 1, 2, and 3 propose widening to the channel bottoms in Reaches 1, 2, and 3 as well as enhancements to Reach 4, and thus hydraulic modeling indicates that each of these alternatives provides increased hydraulic capacity. However, the hydraulic modeling also indicates very little decrease to the water surface elevations along Reach 1 and approximately mid-span along Reach 2. The elevated water surface along these reaches is caused by the lack of capacity of the Mission Gorge Road culvert. As such, a significant result of the preparing and analyzing the initial three initial alternative designs is identifying that, without improvement to Mission Gorge Road, little benefit in terms of reduced water surface elevations will be achieved in Reach 1 and east to about mid-span of Reach 2.

Preferred Alternative Design

The Preferred Alternative Design for Alvarado Creek was formulated based on the combination of the results of the initial alternative design modeling, public outreach feedback, and coordination with various City departments. Based on the coordination efforts, assumptions were incorporated into the Preferred Alternative. One assumption is that a 10-foot wide pedestrian trail shall be aligned above the top of slope of the north channel bank. A second assumption is that in areas of existing or proposed wetland vegetation, a 20-foot buffer must be incorporated along the wetland habitat. This assumption is primarily relevant to the north side of Reach 2. The Preferred Alternative Design is shown in Attachment 3. Below is a summary of the proposed Alvarado Creek improvements for each reach through the Study Area:

REACH 0 (MISSION GORGE ROAD) – EXISTING TRIPLE RCB CULVERT

The Preferred Alternative Design includes adding a 15 feet wide by 8 feet high RCB culvert adjacent to and north of the existing triple 11 feet wide by 8 feet high RCB culverts through Mission Gorge Road in order to improve the hydraulic capacity of the existing structure. The geometry of this additional culvert was determined through hydraulic modeling. The assumed hydraulic modeling did not consider a geometry reduction as result of potential debris loading, and doing so would result in a larger geometry being required. The area immediately downstream and upstream of the proposed culvert will require modifications, such as a warped wingwall in order to transition to the proposed wider geometry.

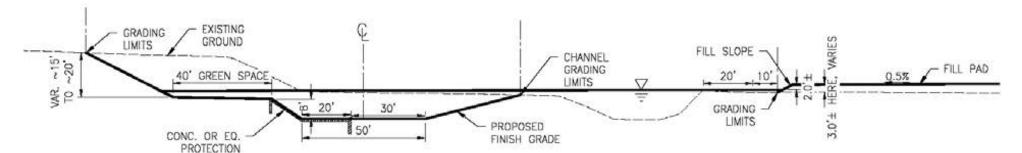
REACH 1 – CONCRETE LINED CHANNEL

The existing Reach 1 geometry, approximately 29.5 feet bottom wide concrete channel, has a fairly good hydraulic capacity, and thus only slight modifications were incorporated into Reach 1 as part of the Preferred Alternative Design. Development within the properties on the north side of Alvarado Creek will require fill in order to meet the City's reguirement that the first floor elevation of proposed buildings be at least 2 feet above Base Flood Elevations. As such, the Preferred Alternative Design assumed a 10-foot wide pedestrian trail adjacent to the north bank top of slope followed by 2 to 1 (horizontal to vertical) slope up to a fill pad. The elevations of the fill pad were determined based on water surface elevations, which were determined through hydraulic modeling of the Preferred Alternative Design. The fill pad was assumed to be two feet above the 100-year frequency storm event water surface elevation and projected upward at a slope of 0.5 percent. When the depth of flow is above the north channel bank top of slope, the pedestrian path and fill slope and pad geometries become part of the overall channel (i.e., essentially act like an extension of the north channel bank). As such, these geometries were incorporated into the hydraulic modeling. Modifications to the channel width are required at the downstream and upstream limits of the Reach 1 in order to transition to the wider geometries proposed within Reach 0 and Reach 2, respectively.

REACH 2 - EARTHEN CHANNEL

The Preferred Alternative Design included the widening of Reach 2. The hydraulic modeling of the existing conditions indicates that Reach 2 has limited hydraulic capacity relevant to the other reaches and thus significantly restricts flow and results in elevated water surface elevations. Reach 2 also contains sensitive wetland vegetation that should be protected wherever possible (i.e., not impacted by proposed improvements). In order to accomplish the objectives of adding hydraulic capacity while minimizing impacts to the exiting sensitive wetland vegetation, The Preferred Alternative Design for Reach 2 consists of extending the channel geometry south of the existing channel to improve flood control. That is, a channel with a bottom width of 50 feet is proposed in an alignment slightly south of the existing Reach 2 alignment. The longitudinal slope along this shifted alignment is approximately 0.35 percent. The northern 30 feet of this channel is proposed as an earthen with riparian vegetation and 4 to 1 (horizontal to vertical) side slopes that daylight in the vicinity of the existing channel southern limits. The southern 20 feet of the channel is proposed as concrete lined with 1.5 to 1 (horizontal to vertical) side slopes. South of the concrete lined channel, at an elevation of approximately 8 feet above the channel bottom, a floodable green space is proposed. North of the existing channel, a 20-foot buffer zone and 10-foot pedestrian path was assumed. North of pedestrian path a fill slope and fill pad was incorporated into the hydraulic modeling as described in Reach 1. Figure 5-2 shows a typical cross section for Reach 2 of the Preferred Alternative Design.

FIGURE 5-2 REACH 2 PREFERRED ALTERNATIVE DESIGN TYPICAL CROSS SECTION



REACH 3 – CONCRETE LINED CHANNEL

Similar to Reach 1, no changes were proposed to the typical cross section of Reach 3 as part of the Preferred Alternative Design. A 10-foot pedestrian path, fill slope, and fill pad were incorporated into hydraulic modeling north of the existing north channel bank top of slope (same assumptions as described in Reach 1). Modifications, including a wider bottom width, at the upstream and downstream limits of Reach 3 were included to transition to the wider Reach 2 and Reach 4 geometries, respectively.

REACH 4 - TRIPLE RCB CULVERT

The Preferred Alternative Design for Reach 4 including adding a 15 feet wide by 8 feet high RCB culvert adjacent to and north of the existing triple 11 feet wide by 8 feet high RCB culvert in order to increase culvert capacity. The geometry of this additional culvert was determined through hydraulic modeling. The assumed hydraulic modeling did not consider a geometry reduction as result of potential debris loading, and doing so would result in a larger geometry being required.

REACH 5 – CONCRETE LINED CHANNEL

With the exception of the widening the downstream connection with Reach 4 to create a transition, no improvements are proposed to Reach 5. This reach is fairly steep, and hydraulic modeling indicates that Reach 5 will have adequate hydraulic capacity to convey the 100-year frequency storm event if the hydraulic capacity of Reach 4 is adequately improved.

REACH 6 – EARTHEN BOT-TOM, CONCRETE SIDE SLOPE CHANNEL

Hydraulic modeling indicated that Reach 6 does not have adequate capacity to convey the 100-year frequency design storm event within the channel banks; however, this modeling also shows that the resulting flooding does not inundate existing structures (i.e., within the Study Area, the flooding is limited to a parking lot). As such no improvements were proposed to Reach 6 as part of the Preferred Alternative Design.

Preferred Alternative Design Results

The hydraulic modeling of the Preferred Alternative Design indicates reduced water surface elevations, at varying magnitudes in comparison to the existing conditions, for Reaches 0 through 5. The most significant improvements are observed for Reach 1 where, on average, water surface elevations are reduced by approximately five feet for the 100year frequency design storm event. Along Reach 2 water surface elevations are reduced between approximately one to two feet. Water surface elevations along Reach 3 are also reduced by, on average, approximately one foot. Along Reach 4, the hydraulic modeling indicates that the 100-year flow will be entirely conveyed within underground culverts. The Reach 4 improvements result in a significant decrease in the water surface elevations along Reach 5. No improvements are proposed in Reach 6; however, the proposed improvements to Reach 4 result in reduction of the water surface elevations at the downstream limits of Reach 6. The results of the hydraulic modeling of the Preferred Alternative Design are provided in Attachment 4.

The hydraulic modeling results were used to prepare inundation maps. Geographic Information Software (GIS) was used to prepare the inundation maps. The software compared the HEC-RAS generated water surface elevations to the existing and assumed topography for the existing condition and Preferred Alternative Design, respectively, in order to generate geospatial depths and limits of flooding. The inundations maps are provided in Attachment 5.

6 Summary

The City is preparing a Study to develop and evaluate scenarios for the portion of the Alvarado Creek channel in the Grantville area, which is located near the Grantville Trolley Station. Enhancements to Alvarado Creek have the potential to reduce the risk of flooding, improve natural habitat, and provide various multiple benefits including additional green space and pedestrian connectivity (i.e., transform Alvarado Creek into an amenity). Thus, enhancements to Alvarado Creek may facilitate redevelopment of the properties along the creek and are consistent with the Navajo Community Plan. This Hydrology Background Report was prepared as part of the overall planning in order to evaluate specific development scenarios and provide general recommendations to improve the hydraulic conveyance and reduce water surface elevations of Alvarado Creek through the Study Area.

Data collection was performed in order to get a better understanding of the Study Area and to facilitate the preparation for the hydraulic models. The data collection included performing a topographic survey of the Study Area, reviewing asbuilt drawings, review FEMA data, and conducting

a site visit. In the context of the Hydrology Background Report, the purpose of the data collection was to acquire, filter, and organize information related to the Study Area in support of preparing hydraulic modeling of the existing condition.

The determination of peak flow rates for this Study was based on data obtained from FEMA FIS. Additionally, the USGS StreamStats 3.0 Program was utilized to gain watershed related data. The FEMA FIS data is consistent with the data obtained from the StreamStats Program. The Alvarado Creek watershed at Mission Gorge Road is approximately 13.7 square miles (8,737 acres). The 100-year frequency peak design storm flow obtained from the FEMA FIS and utilized in the Study is 5,100 cubic feet per second (see Table 3-1 for addition design storm events).

For the purposes of this Study, the Alvarado Creek was divided into seven reaches. The culvert at Mission Gorge Road is identified as Reach 0, and the portion of Alvarado Creek within the Study Area is identified as Reaches 1 through 6 moving from downstream to upstream. Reaches were deter-

mined by sections of the creek having consistent geometries and compositions (see Table 4-1).

Hydraulic modeling of the existing condition indicated that in general the system does not have adequate capacity to convey the 100-year freguency storm. In particular, Reach 2 has the least hydraulic capacity. The area of Reach 2 includes sensitive wetland habitat, which results in potential limitations on what may be proposed to improve hydraulic conveyance through this reach. In general, impacts to sensitive wetland habitat should be avoided as much as possible. The hydraulic modeling of the existing condition also indicates that the Mission Gorge Road culvert and the culvert located within the Study do not have the capacity to convey the 100-year frequency storm event (each has the capacity to convey approximately the 25-year frequency storm event). Both of these culverts will result in backwater during large storm events (e.g., 50- and 100-year frequency storm events) with the Mission Gorge Road culvert contributing to the inundation of Mission Gorge Road as well as several properties located along Reaches 1 and 2 north of Alvarado Creek.

The identified existing condition deficiencies, preliminary hydraulic design calculations results, the constraint of having limited property (area), and the potential location multi-use features were considered as part of the process to formulate three initial alternative designs. The three alternative designs were presented to the public through a number of outreach workshops and also to various City departments in order to solicit feedback. Based on a combination of feedback from the public outreach and coordination with City departments, a preferred alternative design was formulated that included improvements to the Mission Gorge Road and on-site culverts and widening Reach 2 (see Figure 5-2). Hydraulic modeling was performed for the Preferred Alternative Design, and the results indicate that the water surface elevations will be reduced, at various magnitudes, across the Study Area. The most significant reductions in water surface elevations will be along Reach 1 and Reach 2 with reductions, on average, of approximately 5 and 1.5 feet, respectively. The hydraulic modeling workmap and results are provided in Attachment 4, and inundation mapping of the existing and proposed conditions is provided in Attachment 5.

GRANTVILLE TROLLEY STATION/ALVARADO CREEK REVITALIZATION STUDY

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Attachments

Attachments are provided on the following pages in the order listed below

Attachment 1: Applicable Flood Insurance Rate Maps for Alvarado Creek

Attachment 2: Site Photos

Attachment 3: Existing Conditions and Preferred Alternative Design Exhibits

Attachment 4: Model Workmaps and Output Tables

Attachment 5: Inundation Maps

Attachment 6: 30% Design Drawings

Attachment 7: Fill Depth Mar

ATTACHMENT 1 - APPLICABLE FLOOD INSURANCE RATE MAPS FOR ALVARADO CREEK

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program, it does not necessarily identify all areas subject to flooding, perticularly from local drainage sources of small size. The community map repository should be consulted for possible undested or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Earwations (IPEE) and for floodways have been determined, users are encuraged to consist the GRO Profiles and Floodway Data and/or Summany of Sithwater Eventions tables on Floodway Data and/or Summany of Sithwater Eventions tables and Floodway Data and/or Summany of Sithwater Eventions the Floodway Data and Subject (Floodway Data and Sithwater Eventions the Floodway of Sithwater Eventions the Floodway of Sithwater Sithwa

Coassal Base Flood Elevations (BFEs) shown on this map apply only landward of 0.7 North American Vertical Datum of 1988 (MAVO SIS). Users of the FIRM should be aware that coastal flood elevations are also provided in the Summary of Silvariand Florations table in the Flood elevations are also provided in the Summary of Silvariand Florations table in the Flood elevations of Silvariand Silvariand (Floration Silvariand Floration Silvariand Floration Silvariand Silvaria

between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Flazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study appear for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universit Transverse Mercator (URIV Zone 11. The horizontal datum was NADBS, GRESTBOS operand Differences in datum, spheroid, projection or UTM zones used in the production of PIRMs for earliest projection fundations may resurt in sight positional differences in may leasures across jurisdiction boundaries. These differences do not affect the occuracy of this PIRMs.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to shrutcher and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1920 and the North American Vertical Datum of 1989, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following

NGS Information Services. NOAA, NNGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20010-3282

To obtain current elevation, description, and/or location information for bench mark shown on this map, please contact the Information Services Branch of the Nation Conducts Survey at 7813 1713-1324 or visit its whichts at bitto Nation Income.

Base map information shown on this FIRM was provided in digital format by the USDA National Agriculture Imagery Program (NAP), this information was photogrammetrically compiled at a scale of 1.24.000 from serial photography dated

This map reflects more detailed and up-to-date stream channel configuration has those shown on the previous FRIM for this jurisdiction. The floodigates are floodways that were transferred from the previous FRIM may have been adjusted for conform to these them the terms configurations. As a result, the FRIO all authoritative hydraulic dates may reflect stream channel distances that differ from what is shown on this map.

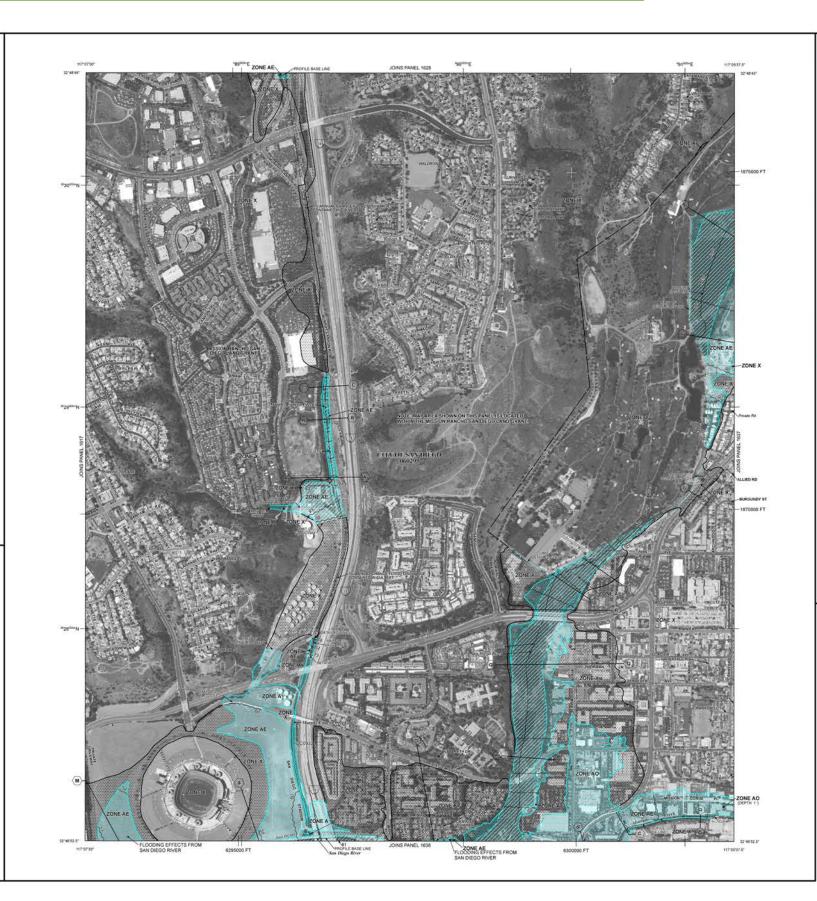
Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

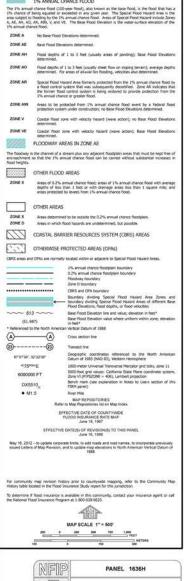
showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

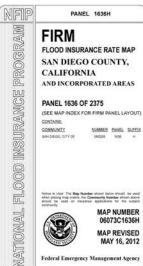
Contact the FEMA Map Service Center at 1-877-FEMA MAP (1-977-339-927) information on available products associated with the FERA. Available product minclude previously issued Letters of Map Change, a Flood Insurance Study repolandor digital versions of this map. The FEMA Map Service Center may also treached by Fax at 1-900-358-9620 and its versions for map. The FEMA Map Service Center may also treached by Fax at 1-900-358-9620 and its versions at http://msc.fema.gov/.

If you have questions about this map or questions concerning the National F1 insurance Program in general, please call 1-877-FEMA MAP (1-677-336-2627 visit the FEMA website at http://www.fema.gov/business/nfg/.

The "profile base lines" depicted on this map represent the hydraulic modeline baselines that match the flood profiles in the FIS report. As a result of importe topographic data, the "profile base line", in some cases, may deviate significant







NOTES TO USERS

This map is for use in administrating the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, perticularly from local desirage sources of small size. The community map repository should be consulted for

To obtain more detailed information in areas where Base Flood Elevations (IPEA) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway, Data and/or Summany of Stillwater Elevations tableton contained within the Flood insurance Study (FISI) upon that accompanies that ML Users should be aware that BFEs shown on the FIRM represent rounded whole-food elevations. These EFEs are intended to flood insurance rating purposes and should not be used as the sole source of flood elevation entire purpose contained to the state of the Contraction with the Contraction of the Contrac

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Boundaries of the **floodways** were computed at cross sections and interpoliate between cross sections. The floodways were based on hydraulic considerations wit regard to requirements of the National Flood Insurance Program. Floodway width and other pertinent floodway data are provided in the Flood Insurance Study report for this instinction.

Certain areas not in Special Flood Hazard Areas may be protected by flood contro structures. Refer to Section 2.4 "Flood Protection Measures" of the Floor

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) Zone 11. The harbonatal datum was NAUDS, GRS1900 spheroid Differences in datum, spheroid, projection or UTM zones used in the production or FIFAMs for adjacent jurisdictions may result in stight postcoral differences in may features across princiction boundaries. These differences do not defect the accuracy.

Flood elevations on this map are referenced to the North American Vertical Datum of 1968. These flood elevations must be compared to shutcure and ground elevations referenced to the same vertical datum. For information reparting convention between the North American Section 1962 and the Vorth American Datum of 1968, visit the Notional Geodetic Survey website at https://www.np.acaa.gov/ or contract the Notional Geodetic Survey website at the Politonia Section 1962 and the Vorth American Vertical Datum of 1968, visit the Notional Geodetic Survey website at the Notional Geodetic Survey and the Following Datum of 1964 to 1962 and 1962 and

NGS Information Services NOAA, NNGS12 National Geodetic Survey SSMC-3, 89202 1315 East-West Highway Silver Spring, Maryland 20910-328

To obtain current elevation, description, and/or location information for bench may shown on this map, please contact the information Services Branch of the Natio Geodetic Survey (301) 713-3242 or visit its website at http://www.ngs.noae.gov/

Base map information shown on this FIRM was provided in digital format by the USDA National Agriculture Imagery Program (NAIP). this information we photogrammetrically compiled at a scale of 1.24,000 from serial photography date 2009.

This map reflects more detailed and up-to-date stream channel configuration than those shown on the previous FIRM for this particulation. The floodpalans a floodways that were transferred from the previous FIRM may have been adjusted configuration from the residence of the production to the flood revious FIRM may have been adjusted configuration. As a result, the Production to the production of the produc

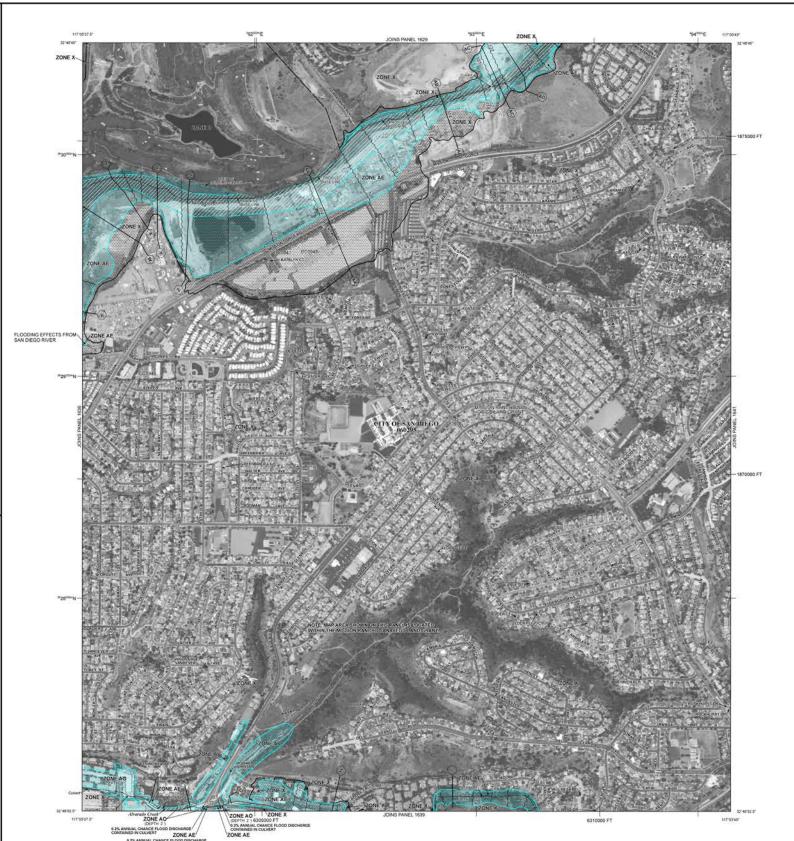
Corporate limits shown on this map are based on the best data available at the first of publication. Because changes due to amexations or de-amexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

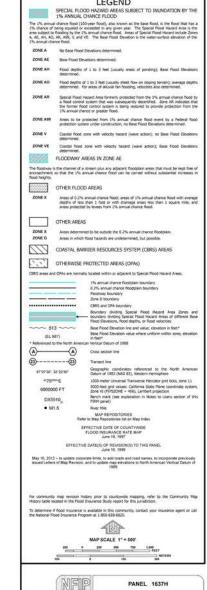
Prease refer to the separately printed Map Index for an overview map of the count showing the layout of map panels, community map repository addresses; and Listing of Communities table containing National Flood Insurance Program dates heach community as well as a sisting of the panels on which each community located.

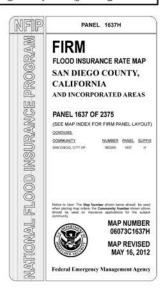
and/or digital versions of this map. The FEMA Map Service Center may also treached by Fax at 1-800-358-9620 and its website at http://msc.fema.gov/.

If you have questions about this map or questions concerning the National Flo

The "profile base lines" depicted on this map represent the hydraulic model baselines that match the flood profiles in the FIS report. As a result of improv topographic data, the "profile base line", in some cases, may deviate significar from the channel conferince or appear outside the SFHA.







NOTES TO USERS

This map is for use in administering the National Flood Insurance Program not necessarily identify all areas subject to flooding, particularly from local sources of small size. The community map repository should be con





LEGEND

SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD



NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for noiselike undered or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (IEFs) and/or floodways have been determined, where Base Flood Elevations (IEFs) and/or floodways have been determined, where and Flood Florides and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Stably (IEFs) great that accompanies this FIRM. Uses should be aware that BFEs at bown on the FIRM represent rounded wholeloof exertises. These IEFs are strended for flood insurance ranging purposes only and should not be used as the soles source of sold elevation information. Accordingly, and should not be used as the soles source of sold elevation information. Accordingly, the FIRM for purposes of construction and/or floodball meanscenered.

Coastal Base Flood Elevations (BFEs) shown on this map apply only landward of 07 North American Vertical Datino of 1980 (NAVI) 503. Users of this Fifth should be ware that coastal flood elevations are also provided in the Summary of Silbhatte Elevations table in the Flood insurance Study report for this prinction. Elevations there in the Summary of Silbhatter Elevations table should be useful for continuous shown on this Fifth.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraufic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this individual.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood

The projection used in the preparation of this map was Universal Transverse Mercator (UTIN) Zone 11. The horizontal datum was NADIS, SRS1960 spheroical Differences in distum, spheroid, projection or UTIM zones used in the production or FIRMs for adjacent jurisdictions may result in sight positional differences in major features across jurisdiction boundaries. These differences do not affect the accuracy or this FIRMs.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information reparding conversion observed the National Geodetic Vertical Datum of 1984, votal the National Geodetic Survey website at http://www.nps.com.agov/or contract the National Geodetic Survey website at http://www.nps.com.agov/or contract the National Geodetic Survey at the following

NGS information Services NOAA, NNGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Saver Spring, Maryland 20910-3282 (301) 713-3242

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Conductor (Conductor Conductor Conductor Conductor Conductor Conductor Conductor Conductor Conductor Conductor Cond

Base map information shown on this FIRM was provided in digital format by the USDA National Agriculture Imagery Program (NAIP). this information was photogrammetrically compiled at a scale of 1.24,000 from aerial photography dates

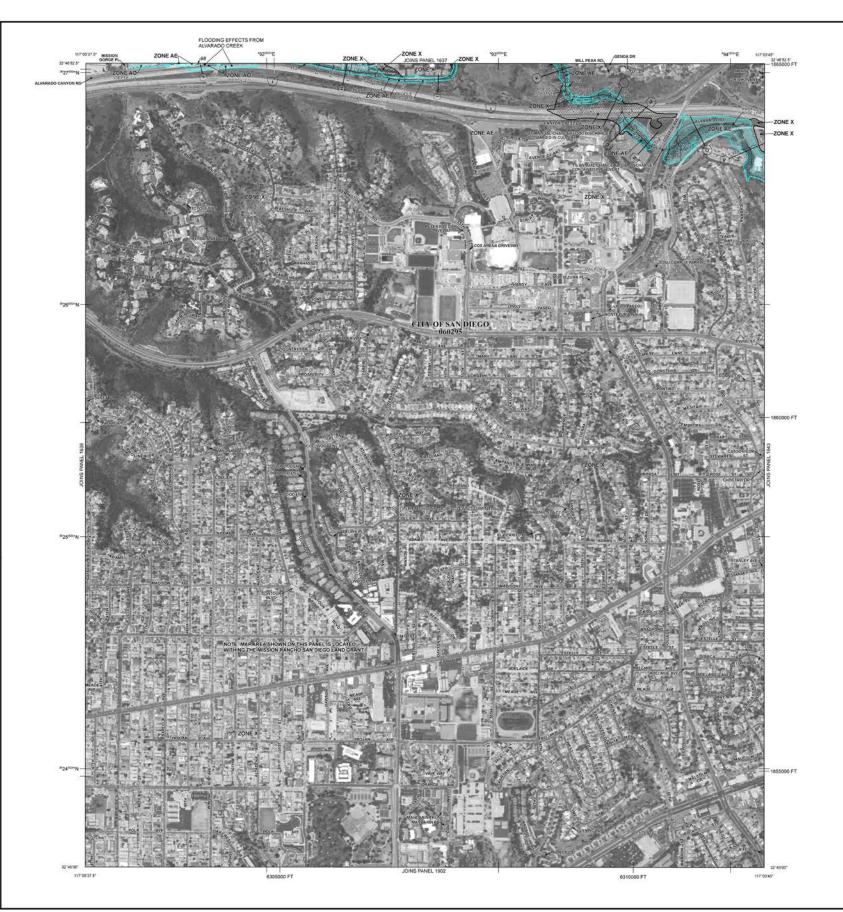
This map reflects more detailed and up-to-date stream channel configuration than those shown on the previous FIRMA or this purisdation. The floodpales and floodways that were transferred from the previous FIRM may have been adjusted in Goodways that were transferred from the previous FIRM may have been adjusted in Goodway to the property of the pro

or publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriat community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the cours showing the layout of map panels; community map repository addresses; and Listing of Communities table containing National Flood Insurance Program dates to each community as well as a listing of the panels on which each community located.

include previously issued Letters of Map Change, a Flood insurance Study report and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and its website at http://inscs.ferma.gov/.

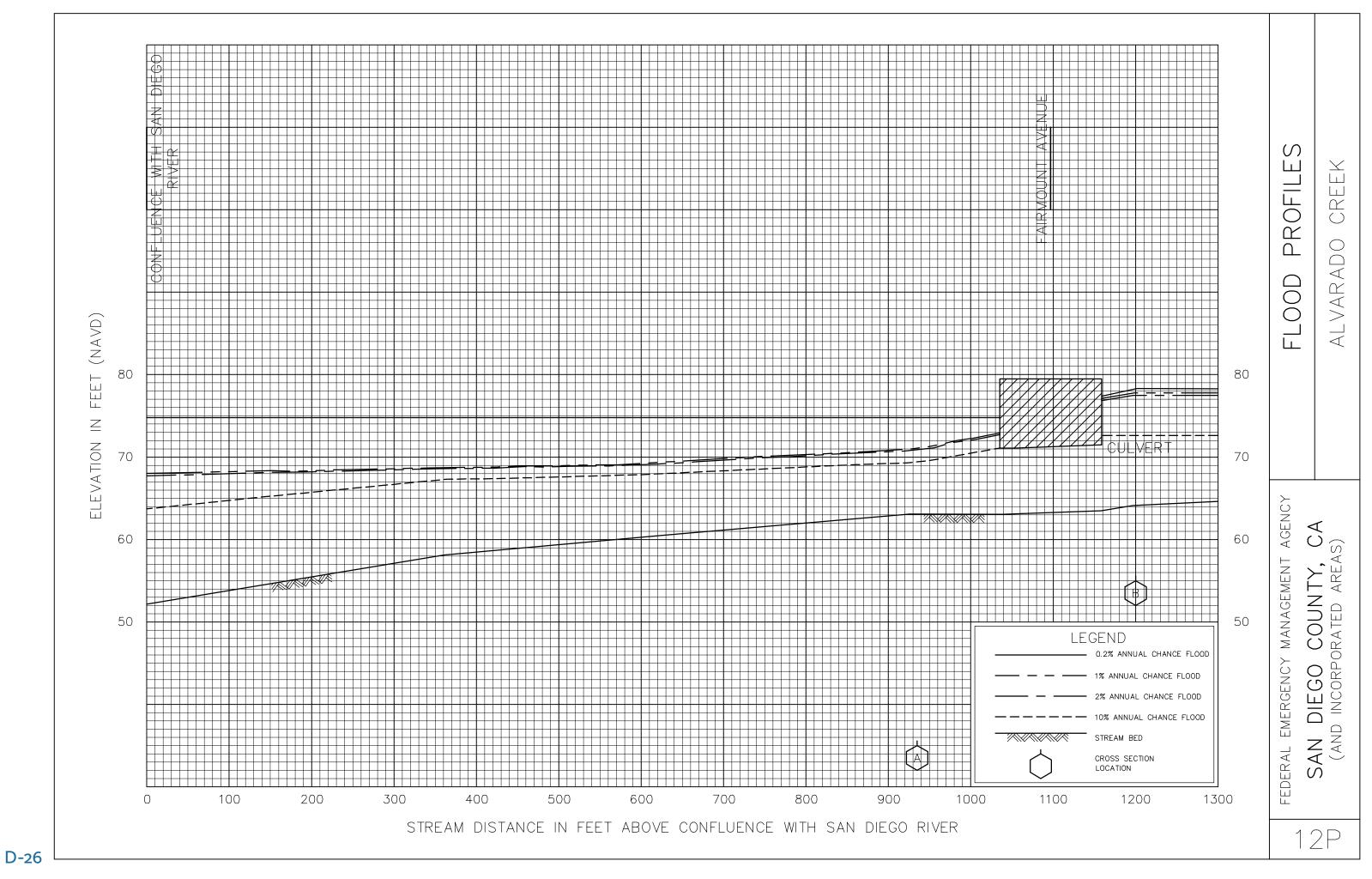
The "profile base lines" depicted on this map represent the hydrautic models baselines that match the flood profiles in the FIS report. As a result of improve topographic data, the "profile base line", in some cases, may deviate significant

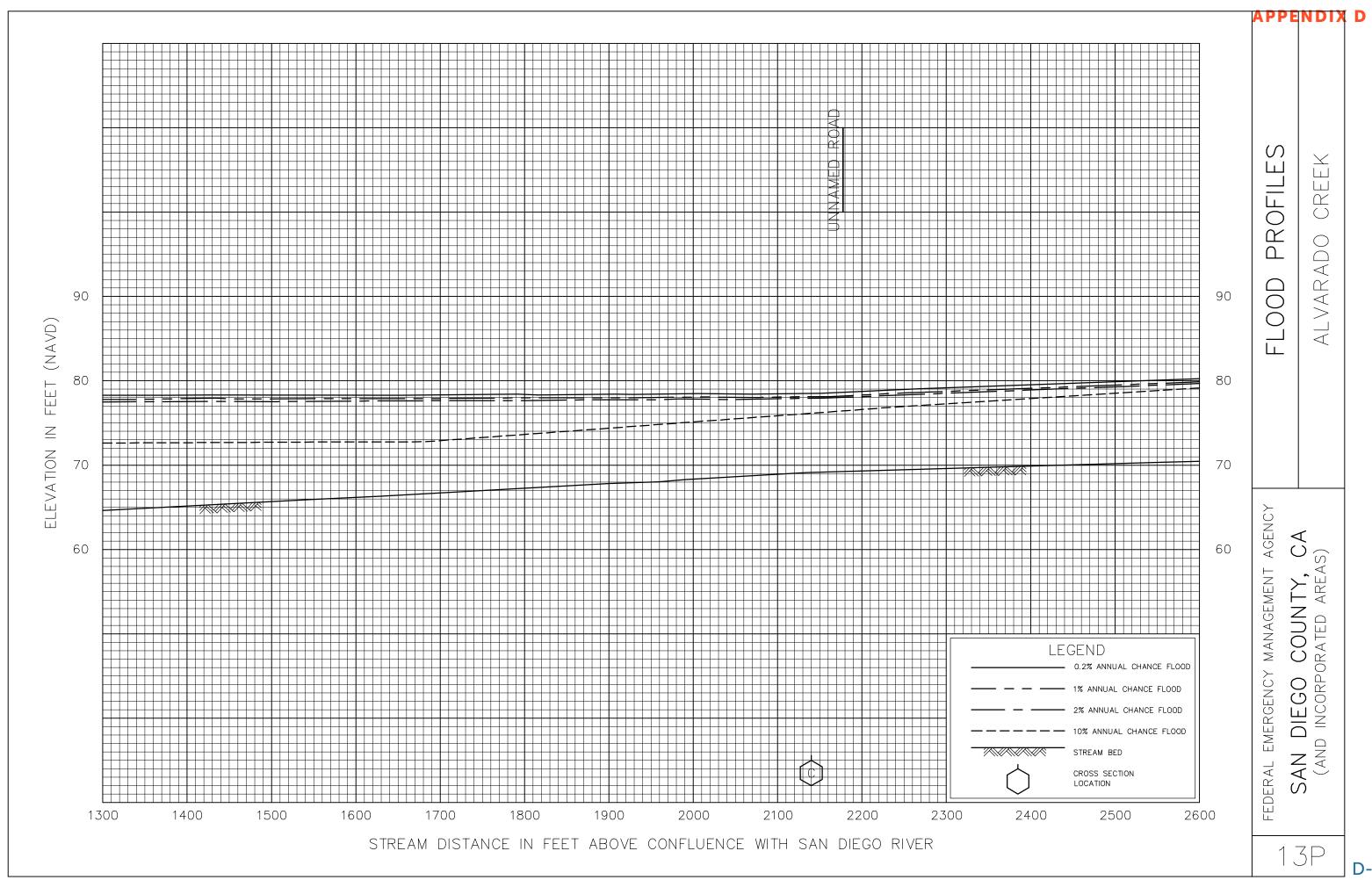


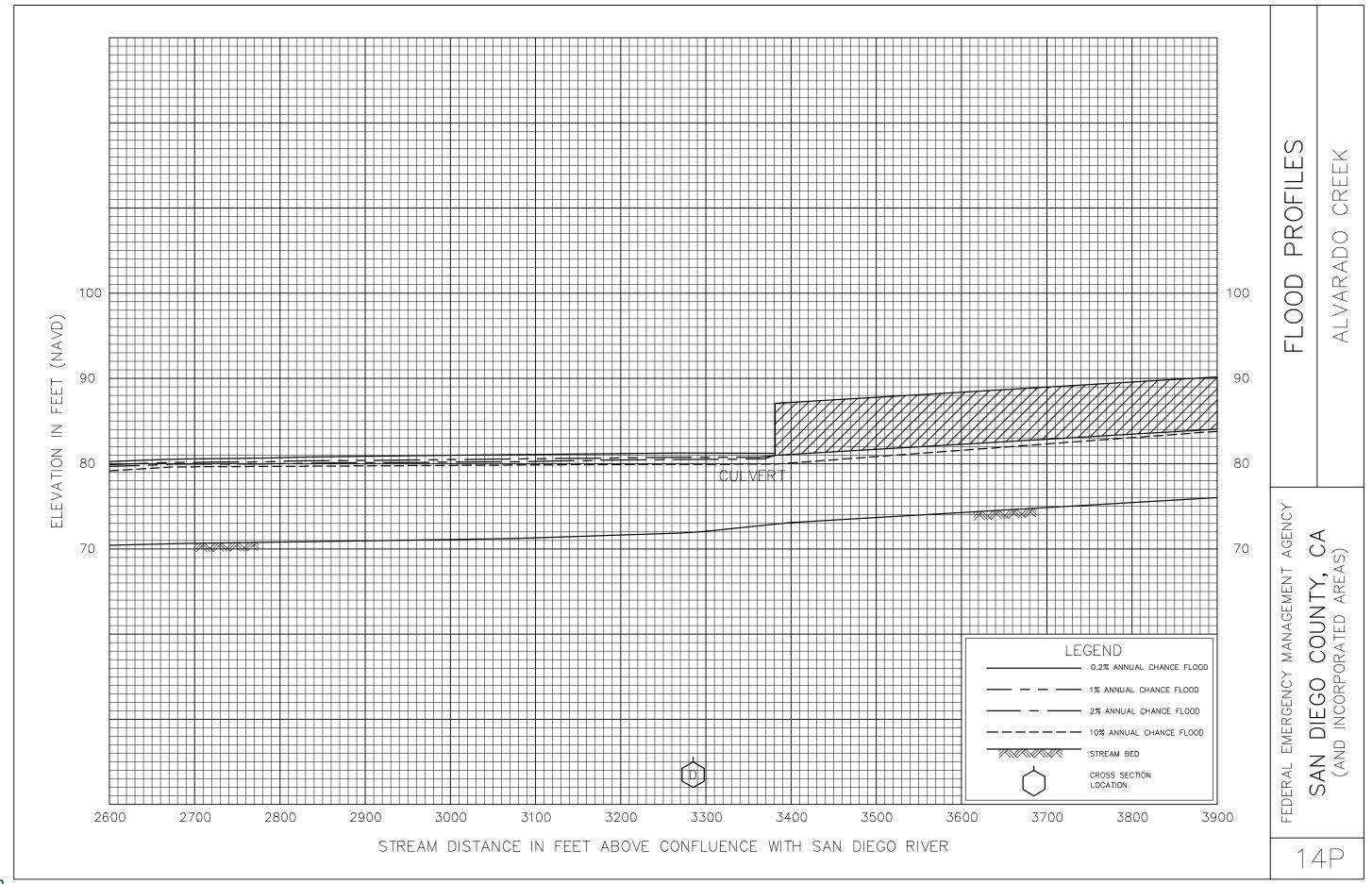


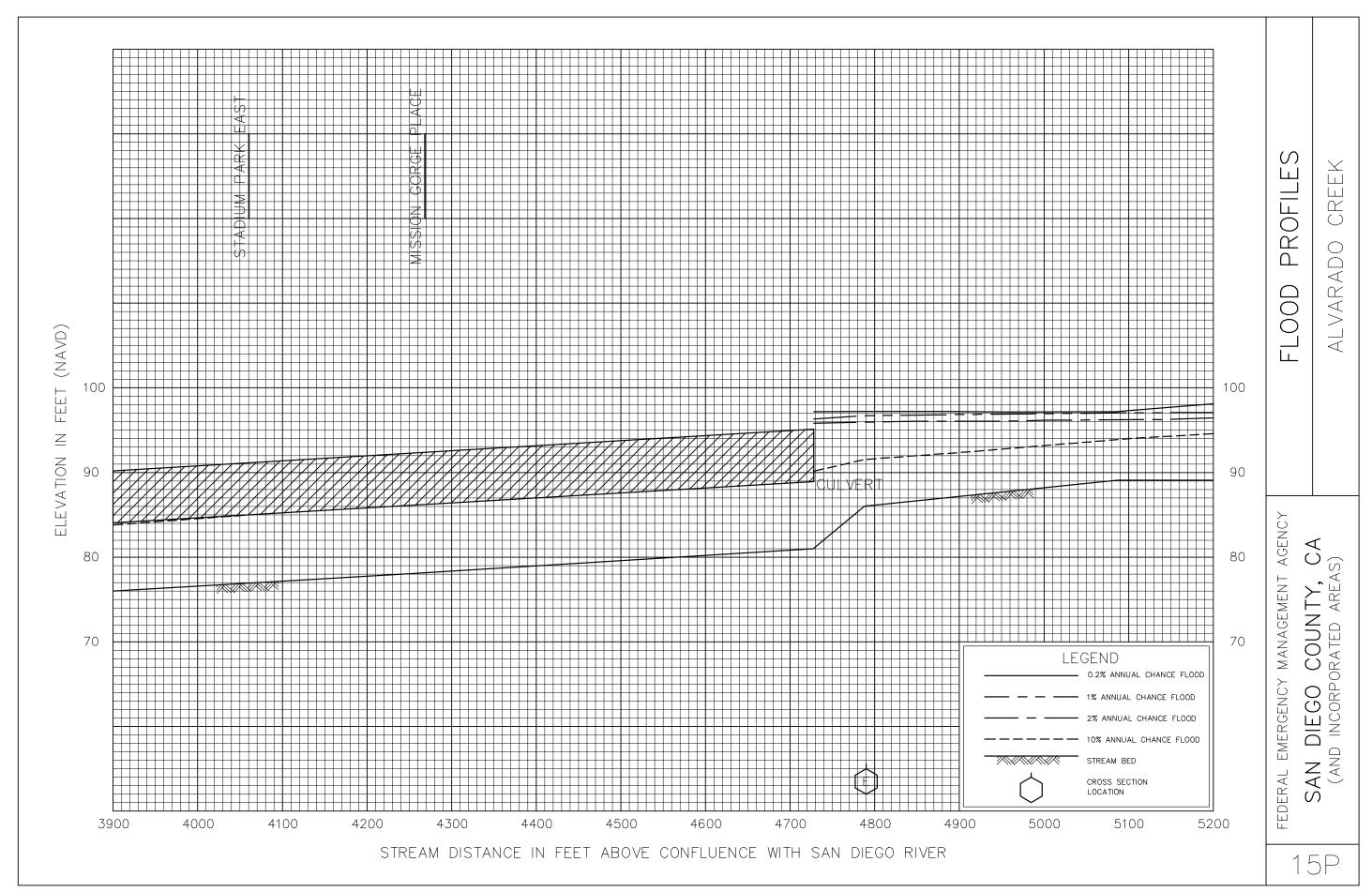
LEGEND

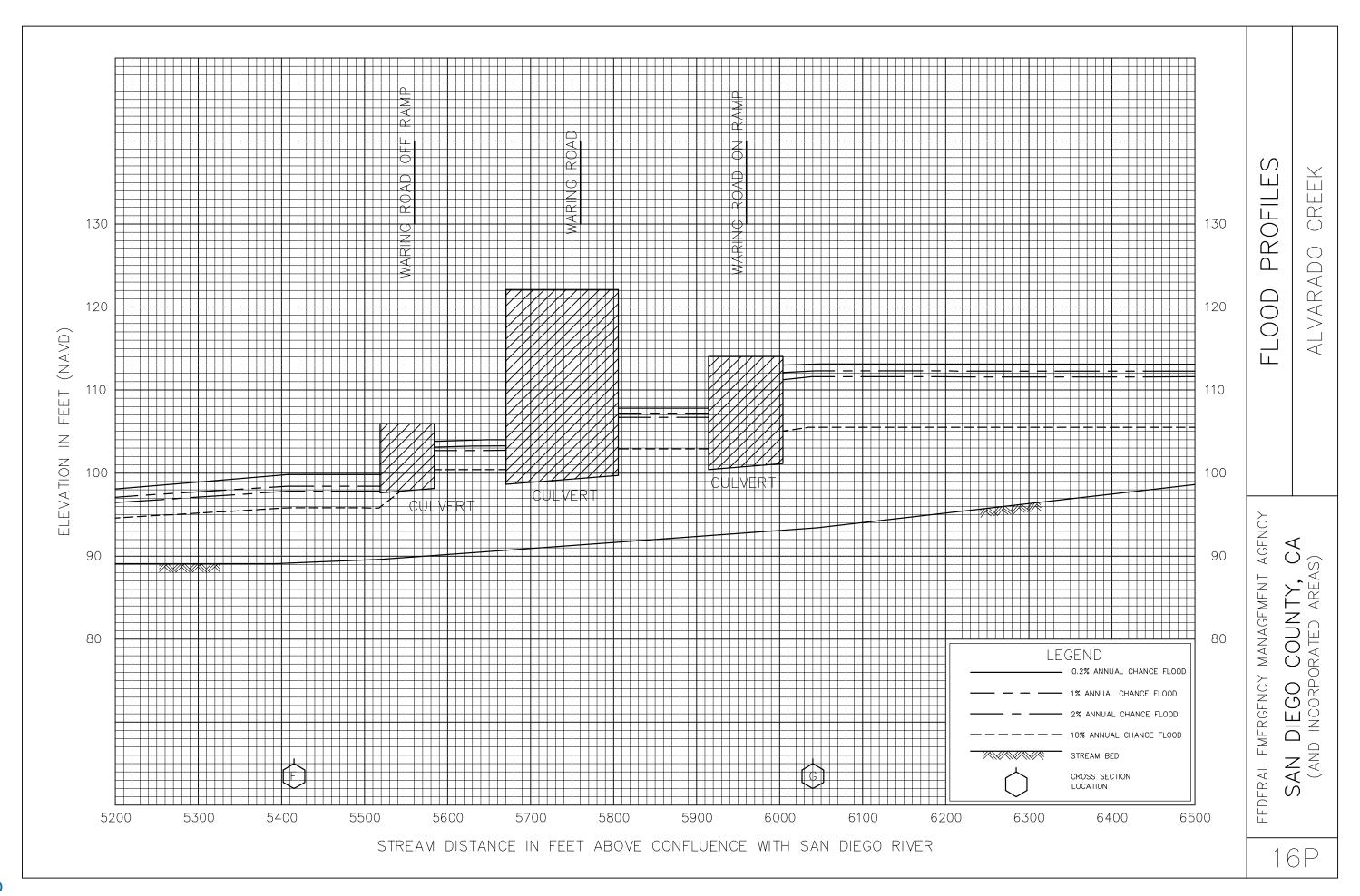
MAP NUMBER 06073C1639H MAP REVISED











3.3 Vertical Datum

All FIS reports and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum in use for newly created or revised FIS reports and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD). With the finalization of the North American Vertical Datum of 1988 (NAVD), many FIS reports and FIRMs are being prepared using NAVD as the referenced vertical datum.

All flood elevations shown in this FIS report and on the FIRM are referenced to NAVD, with exception of two panels: 06073C2151F and 06073C2152F. These panels were not updated with this revision and are referenced to NGVD. Flooding sources on the non-updated FIRMs include Nestor Creek, Otay River, San Diego Bay, Telegraph Canyon Creek, and Tijuana River. The profile panels and floodway data tables that contain information corresponding with the non-updated panels have been included in NGVD, in addition to all of the data being presented in NAVD. Structure and ground elevations in the community must, therefore, be referenced to NAVD. It is important to note that adjacent communities may be referenced to NGVD. This may result in differences in Base (1-percent-annual-chance) Flood Elevations (BFEs) across the corporate limits between the communities. The conversion factor for each flooding source studied by detailed methods is shown below in Table 12 "Flooding Source Conversion Factor."

TABLE 12: FLOODING SOURCE DATUM SHIFT VALUES

Stream Name	Elevation (feet NAVD above NGVD)		
Adobe Creek	+2.2		
Agua Hedionda Creek	+2.2		
Agua Hedionda Creek (At City of Carlsbad)	+2.2		
Agua Hedionda Creek (At City of Vista)	+2.3		
Alvarado Creek	+2.1		
Beaver Hollow Creek	+2.2		
Beeler Creek	+2.1		
Broadway Creek	+2.1		
Buena Creek	+2.3		
Buena Vista Creek	+2.3		
Buena Vista Creek Tributary 1	+2.3		
Buena Vista Creek Tributary 3	+2.3		
Calavera Creek	+2.2		
Carmel Valley Creek	+2.1		
Carroll Canyon Creek	+2.1		
Coleman Creek	+2.5		
County Ditch Creek	+2.1		

APPENDIX D

ATTACHMENT 2 - SITE PHOTOS



Photo 1: Looking downstream (W) from 11'x8' triple box culvert underneath Mission Gorge Road in Reach 1



Photo 2: Looking downstream (W) towards 11'x8' triple box culvert underneath Mission Gorge Road in Reach 1



Photo 3: Looking downstream (SW) near MTS trolley line upstream of Mission Gorge Road in Reach 1



Photo 4: Looking downstream towards Reach 1 (SW) at transition between vegetated and concrete lined channel underneath MTS trolley line



Photo 5: Looking upstream towards Reach 2 (NE) at transition between vegetated and concrete lined channel underneath MTS trolley line





Photo 7: Reach 3 looking upstream (E) towards 11'x8' triple box culvert underneath Grantville Business District



Photo 8: Close up of downstream end of 11'x8' trple box culvert underneath Grantville Business District (Reach 4)

APPENDIX D



Photo 9: Looking west towards upstream end of triple box culvert from the downstream end of Reach 5



Photo 10: Bottom of Reach 5 looking upstream (W) from upstream end of triple box culvert



Photo 11: Looking upstream from towards Reach 6 (E) at transition earthen bottomed channel



Photo 12: Reach 6 looking upstream (E) under trolley line near Waring Road

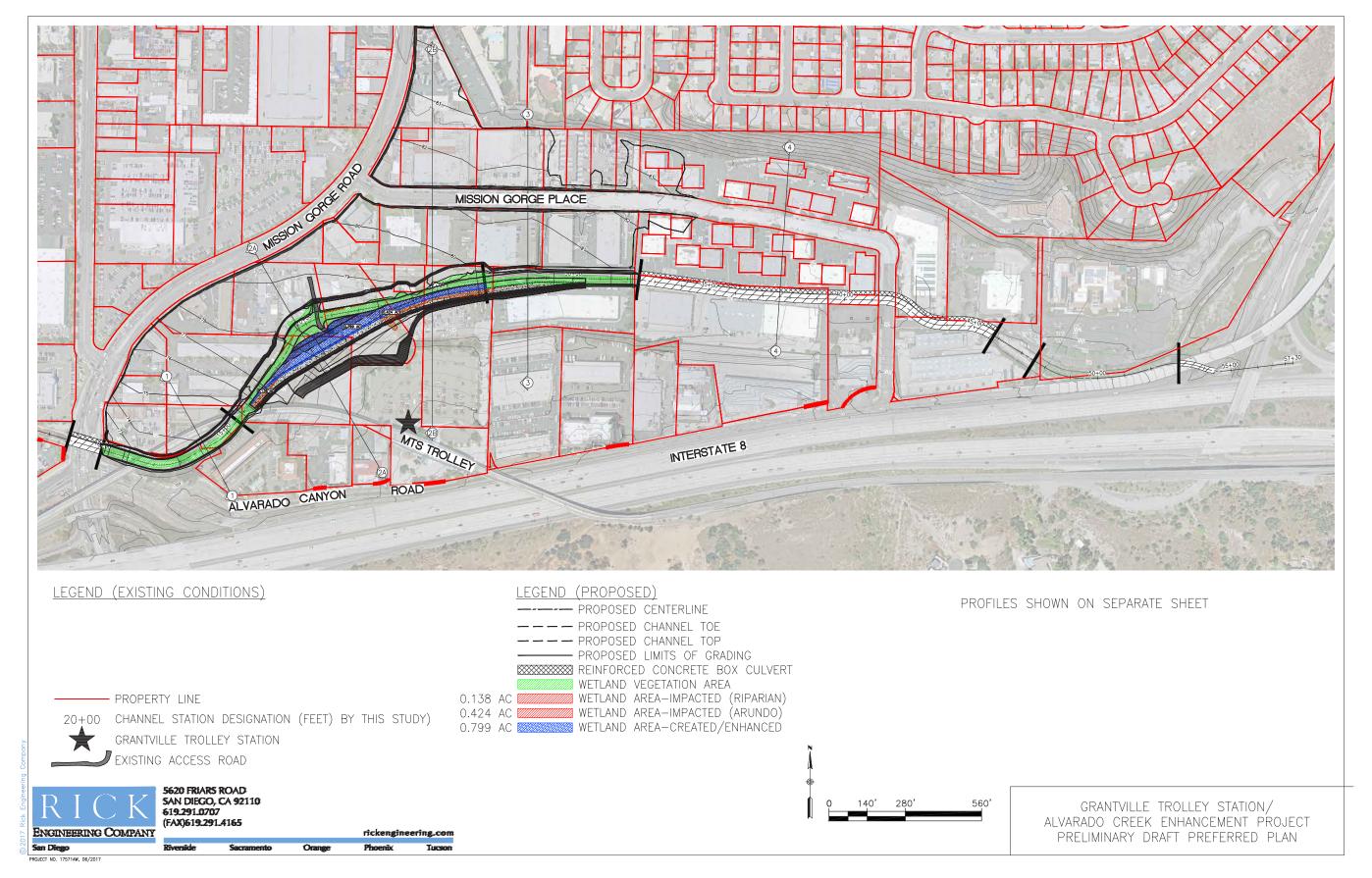


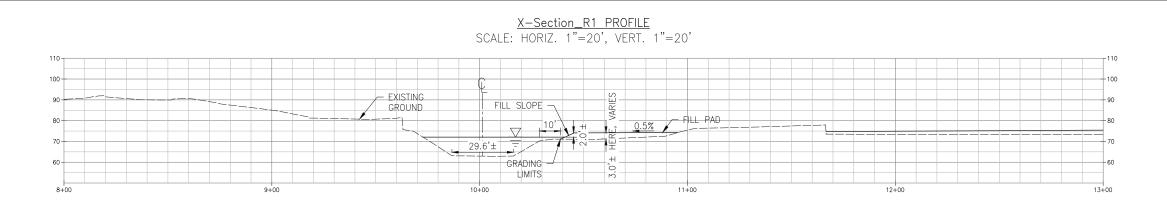
Photo 13: Downstream end of 11'x8' triple box culvert underneath Interstate 8 onramp from Waring Road

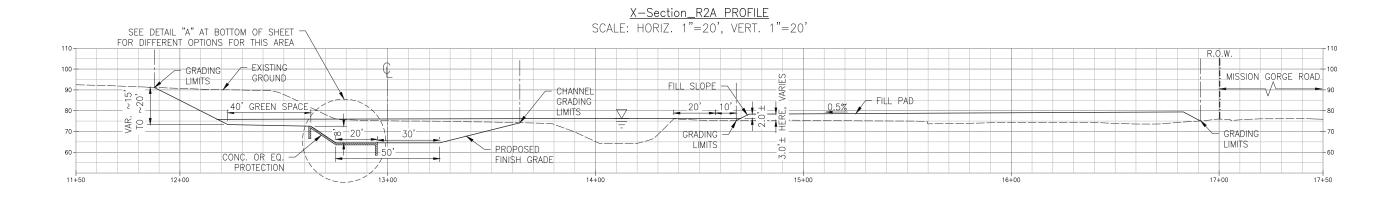


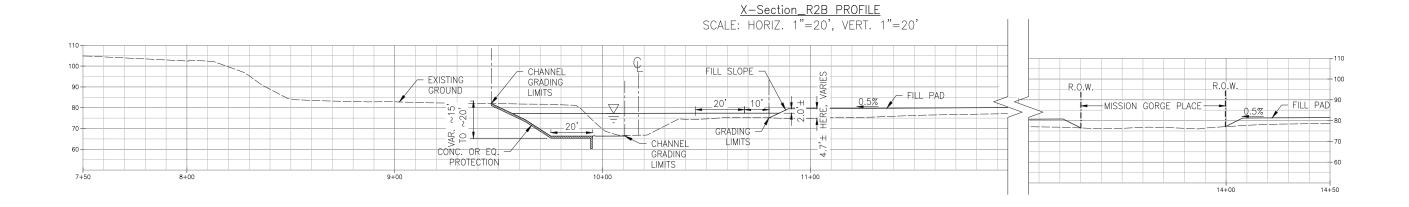
Photo 14: Upstream end of triple box culvert underneath Interstate 8 onramp from Waring Road

ATTACHMENT 3 - EXISTING CONDITIONS AND PREFERRED ALTERNATIVE DESIGN EXHIBITS





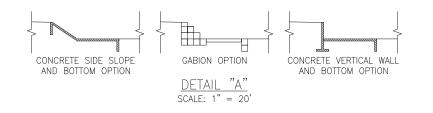




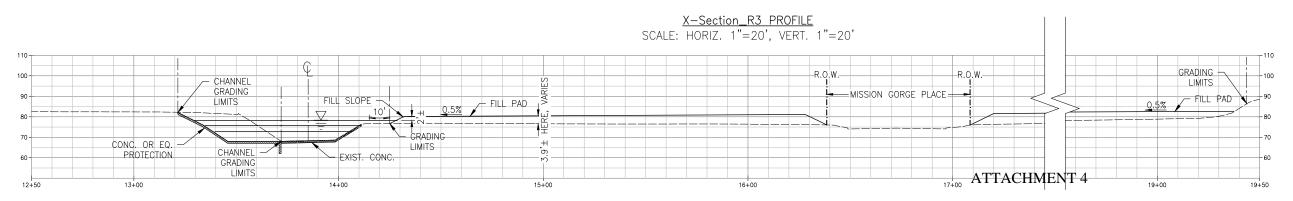


5620 FRIARS ROAD SAN DIEGO, CA 92110 619.291.0707 (FAX)619.291.4165

rickengineering.com iverside Sacramento Orange Phoenix Tucson

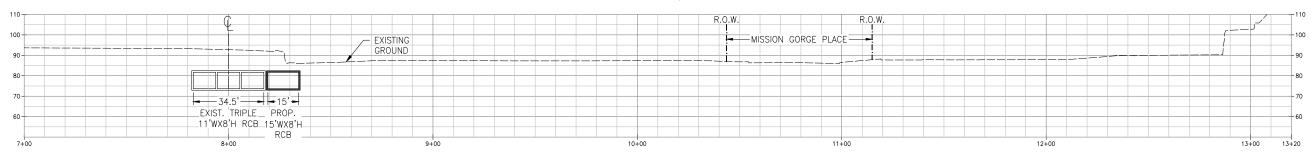


GRANTVILLE TROLLEY STATION/ ALVARADO CREEK ENHANCEMENT PROJECT PRELIMINARY DRAFT PREFERRED PLAN PROFILES SHEET 1 OF 2



MODEL WORKMAPS AND OUTPUT TABLES

X-Section_R4 PROFILE SCALE: HORIZ. 1"=20', VERT. 1"=20'





5620 FRIARS ROAD SAN DIEGO, CA 92110 619.291.0707 (FAX)619.291.4165

rickengineering.com exide Sacramento Orange Phoenix Tucson GRANTVILLE TROLLEY STATION/ ALVARADO CREEK ENHANCEMENT PROJECT PRELIMINARY DRAFT PREFERRED PLAN PROFILES SHEET 2 OF 2

GRANTVILLE TROLLEY STATION/ALVARADO CREEK REVITALIZATION STUDY



1,000

Scale in Feet

Date of Exhibit: 7/6/2017

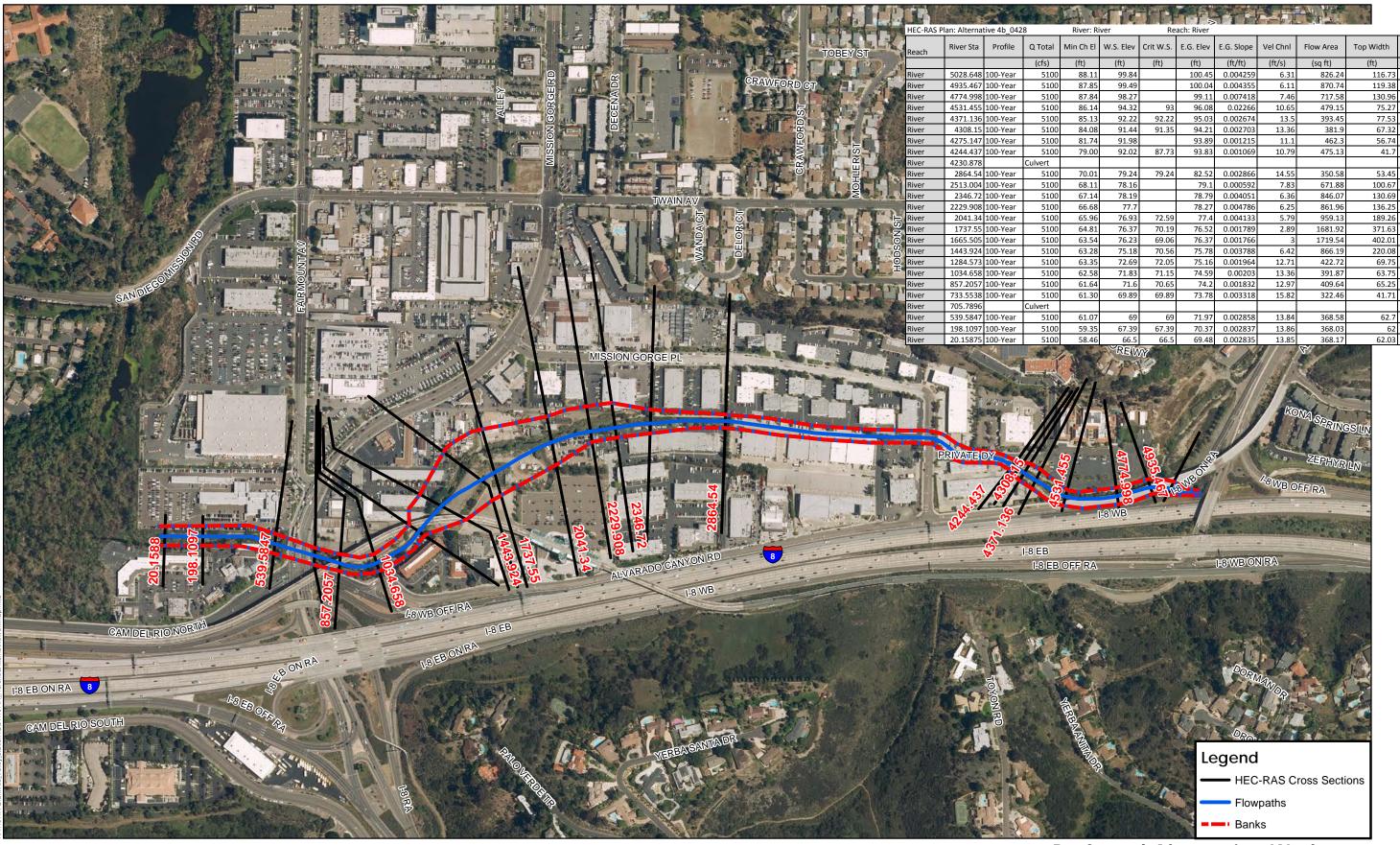
DigitalGlobe Aerial Image: 04.2013

Existing Conditions Workmap

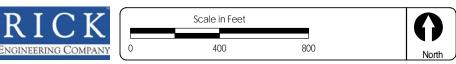
Grantville Trolley Station/Alvarado Creek Enhancement Project

APPENDIX D

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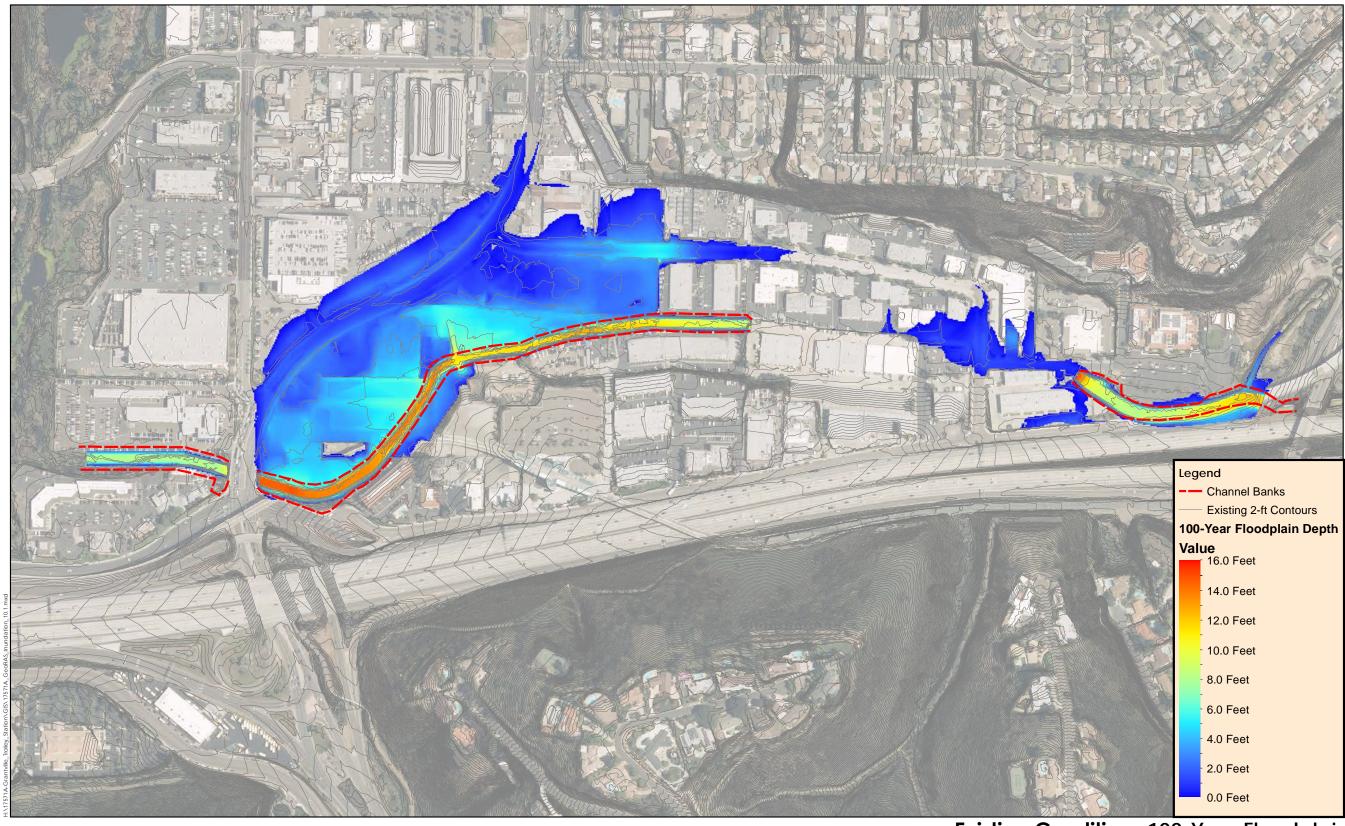


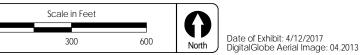
Date of Exhibit: 6/27/2017 DigitalGlobe Aerial Image: 04.2013

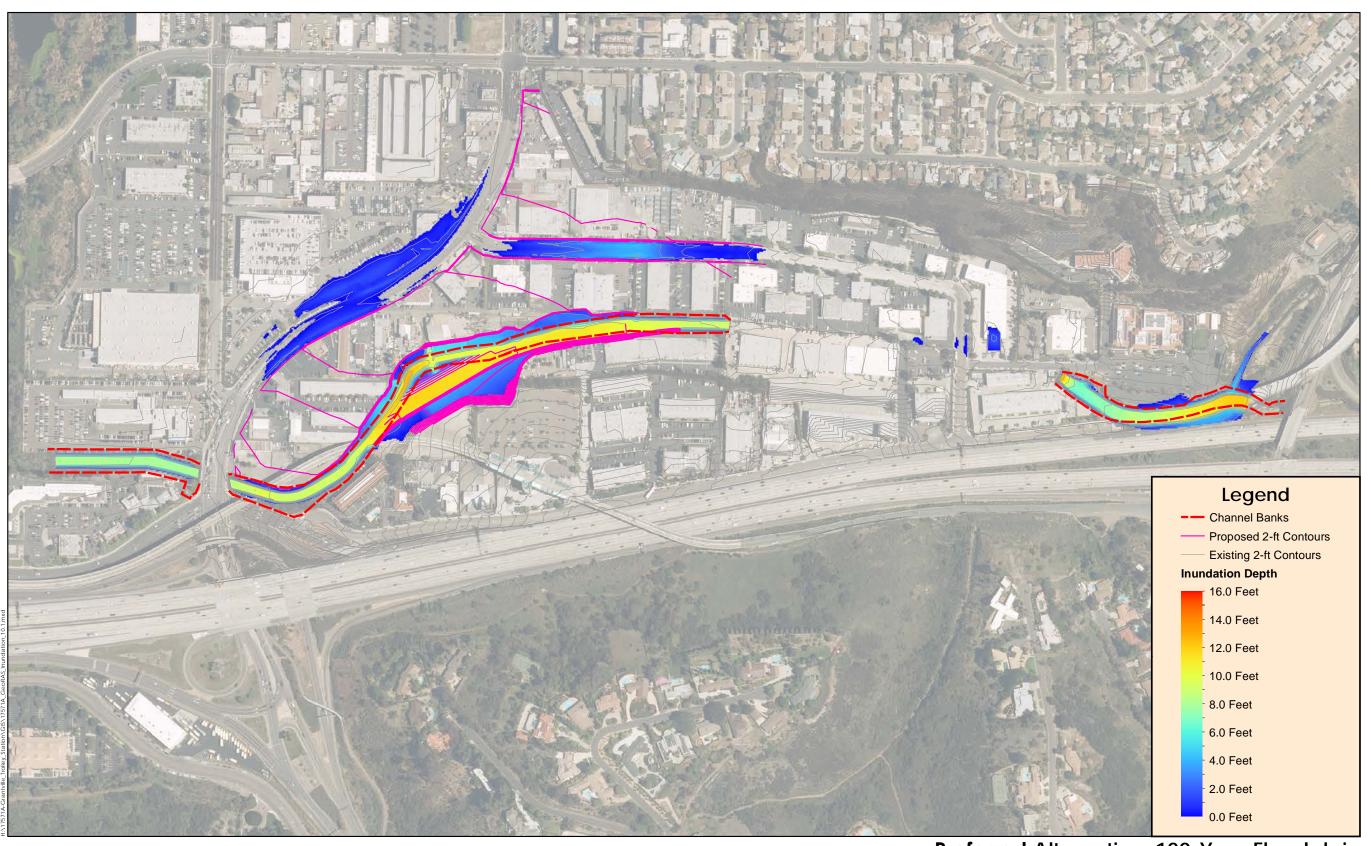


Preferred Alternative Workmap

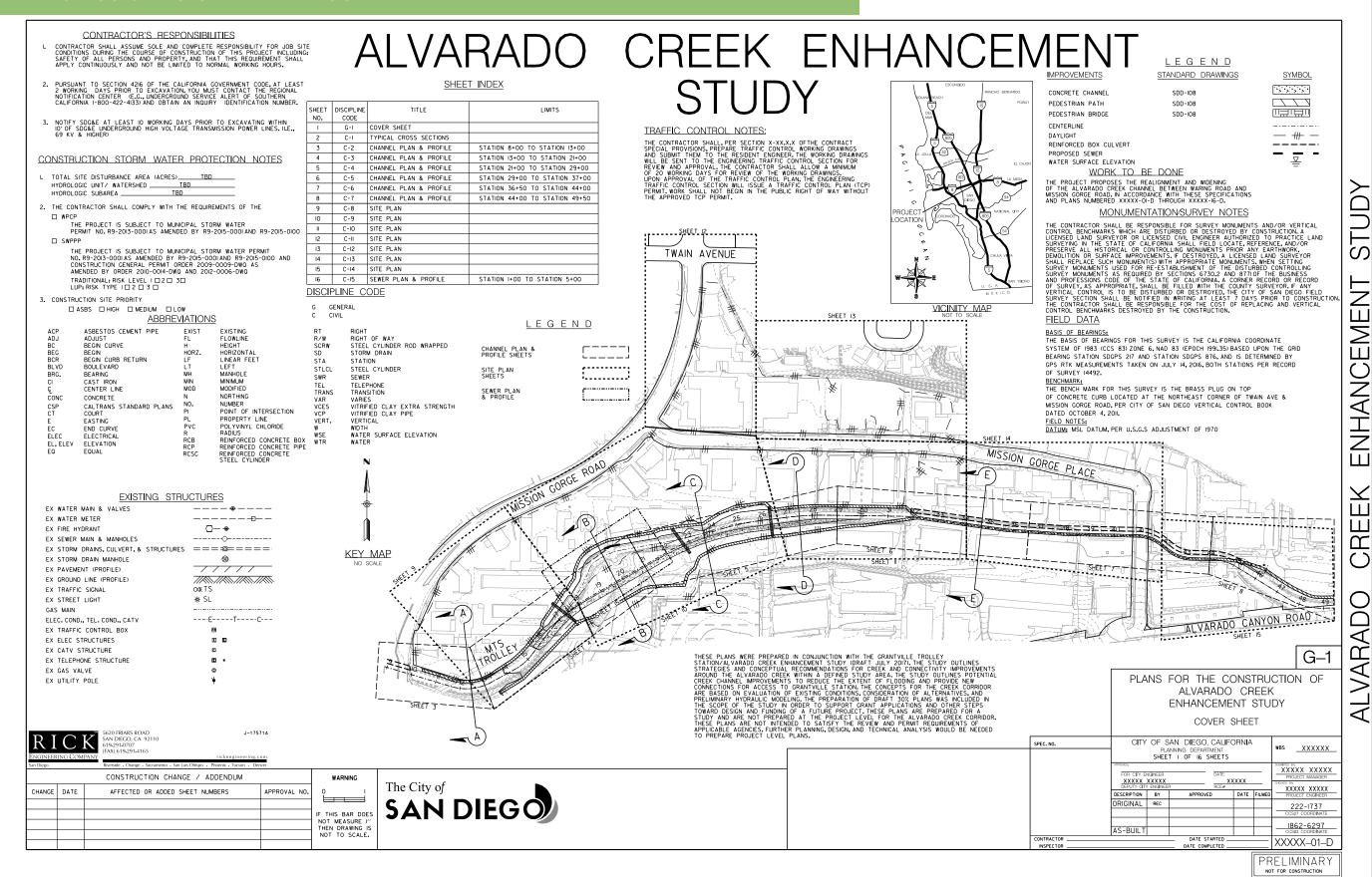
ATTACHMENT 5 - INUNDATION MAPS

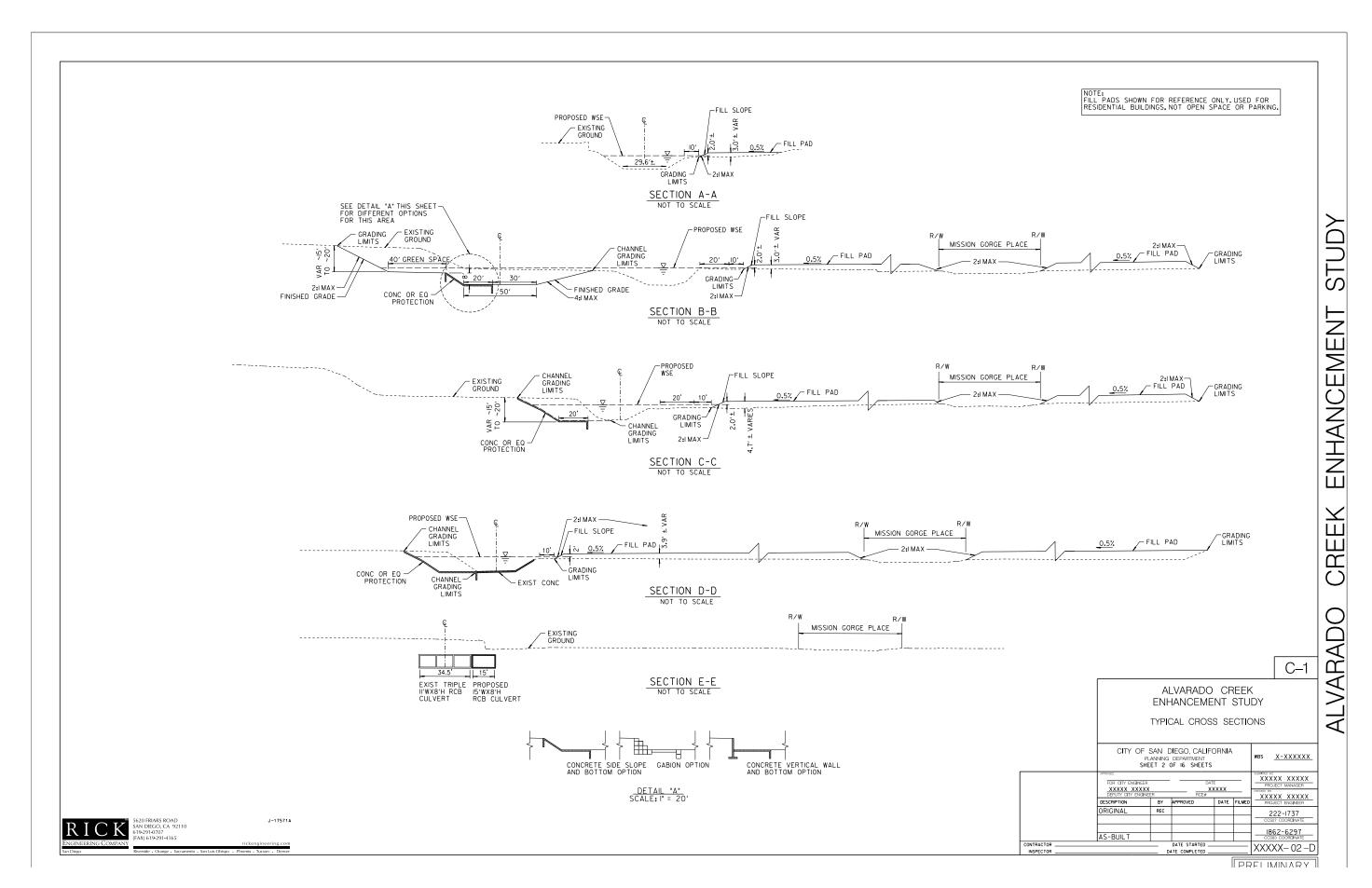


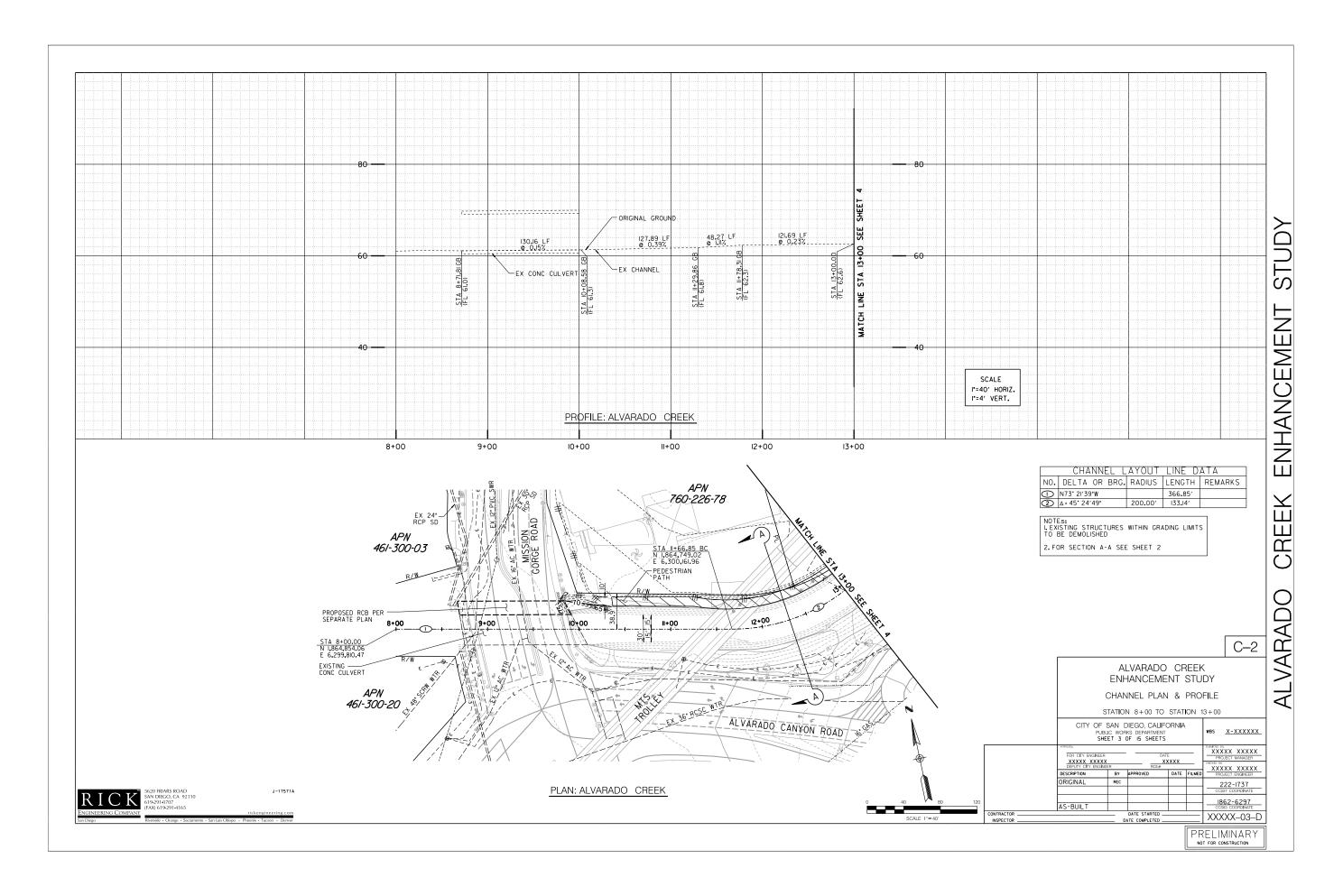


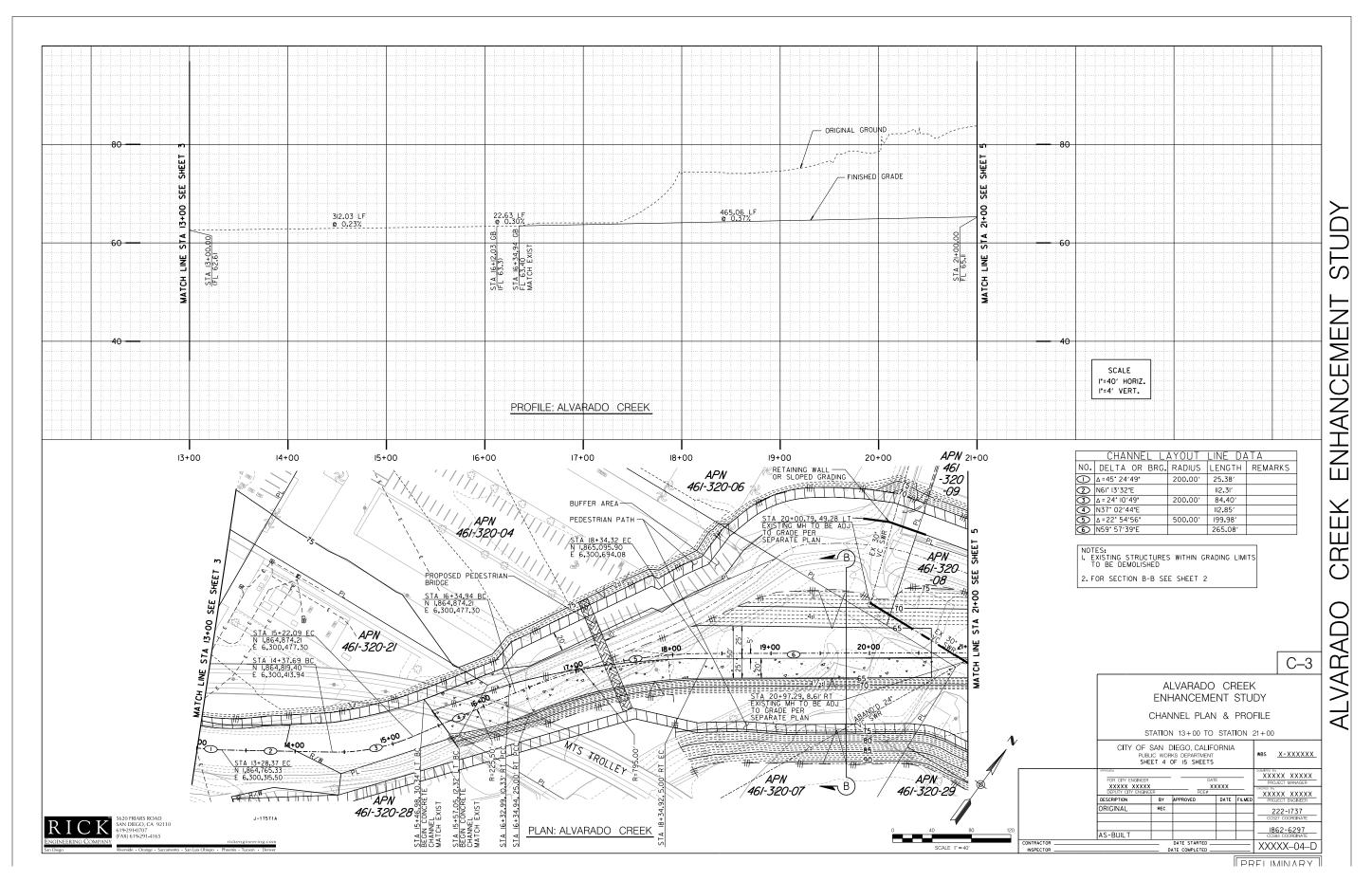


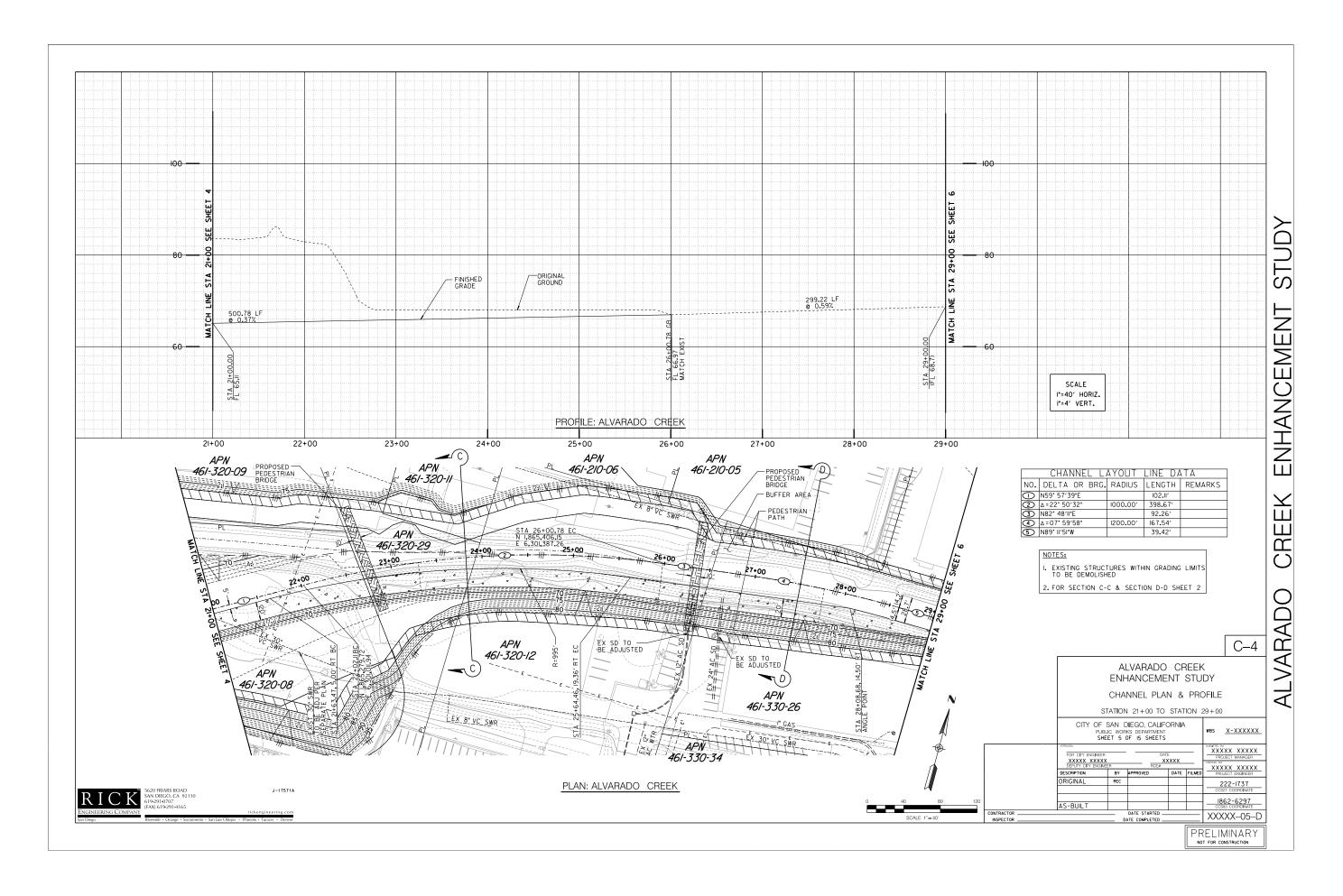
ATTACHMENT 6 - 30% DESIGN DRAWINGS

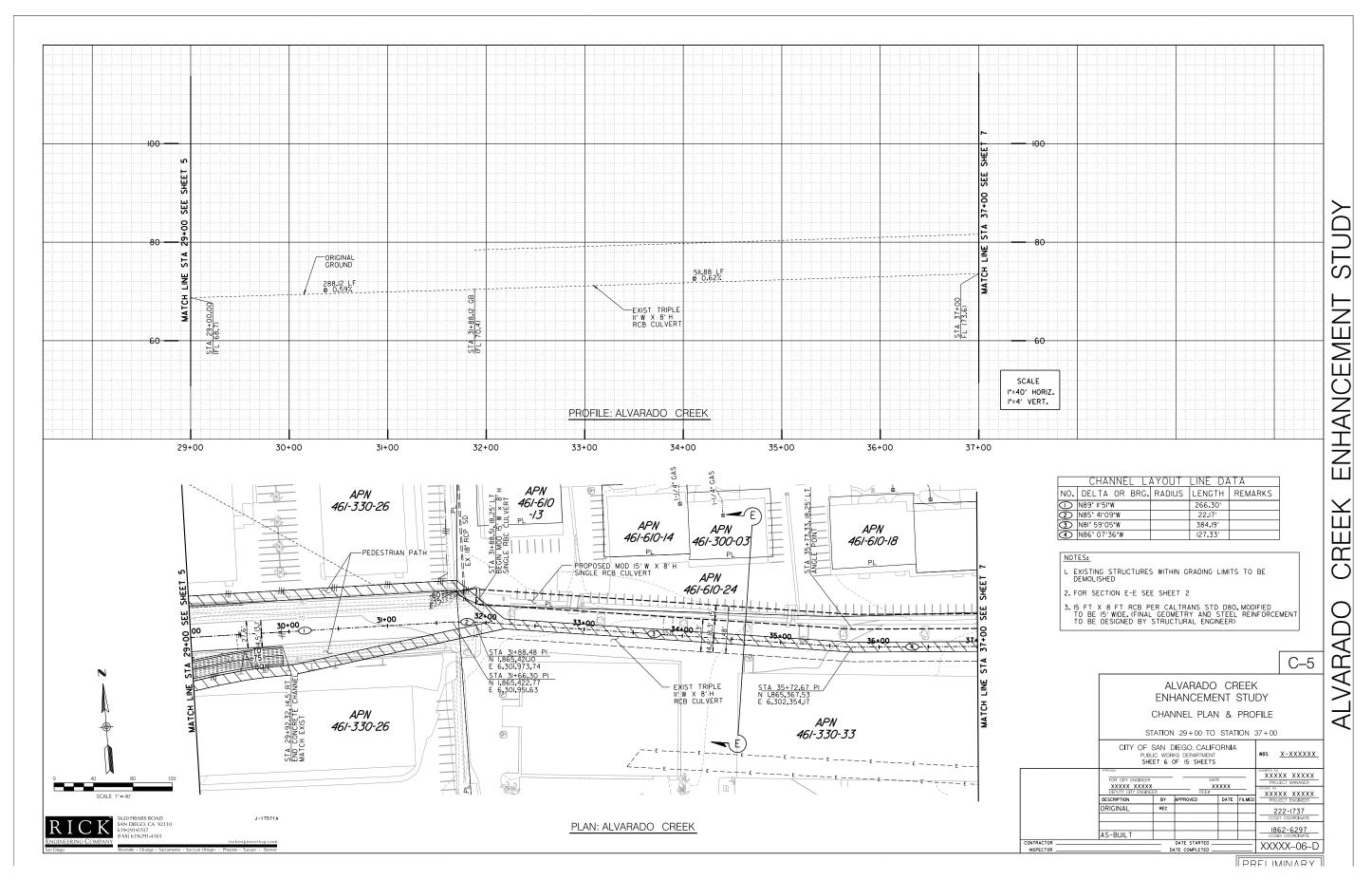


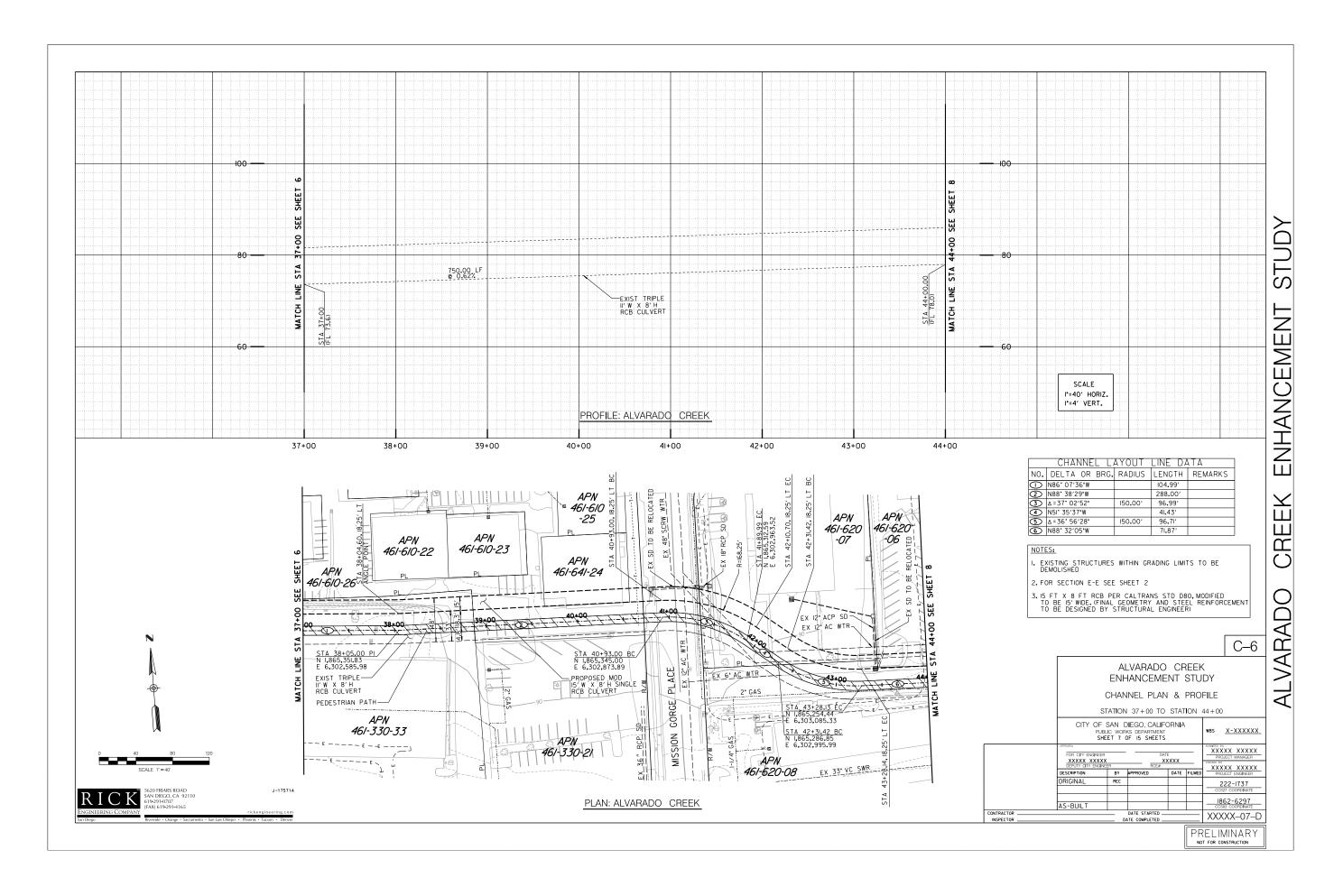


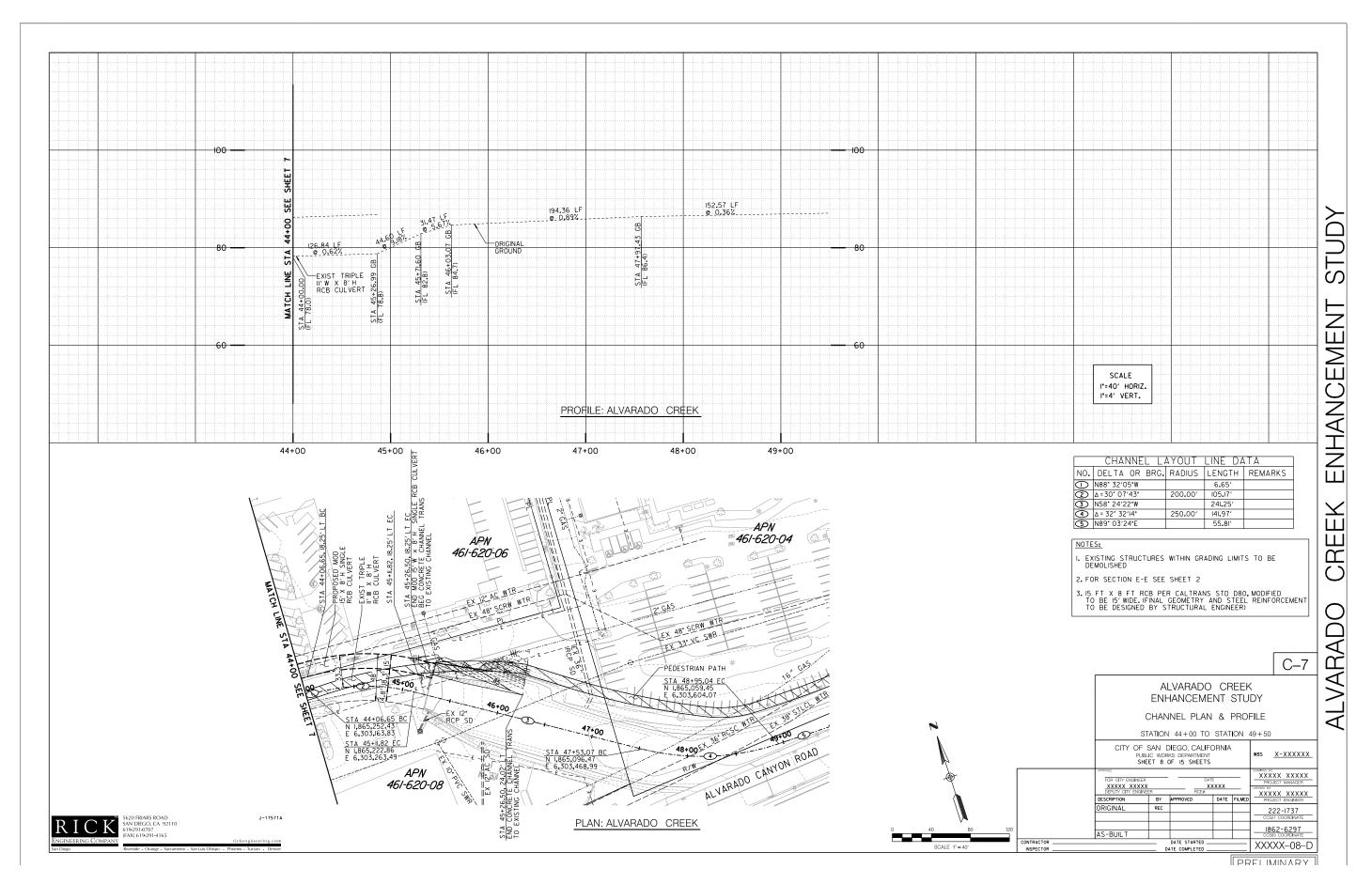


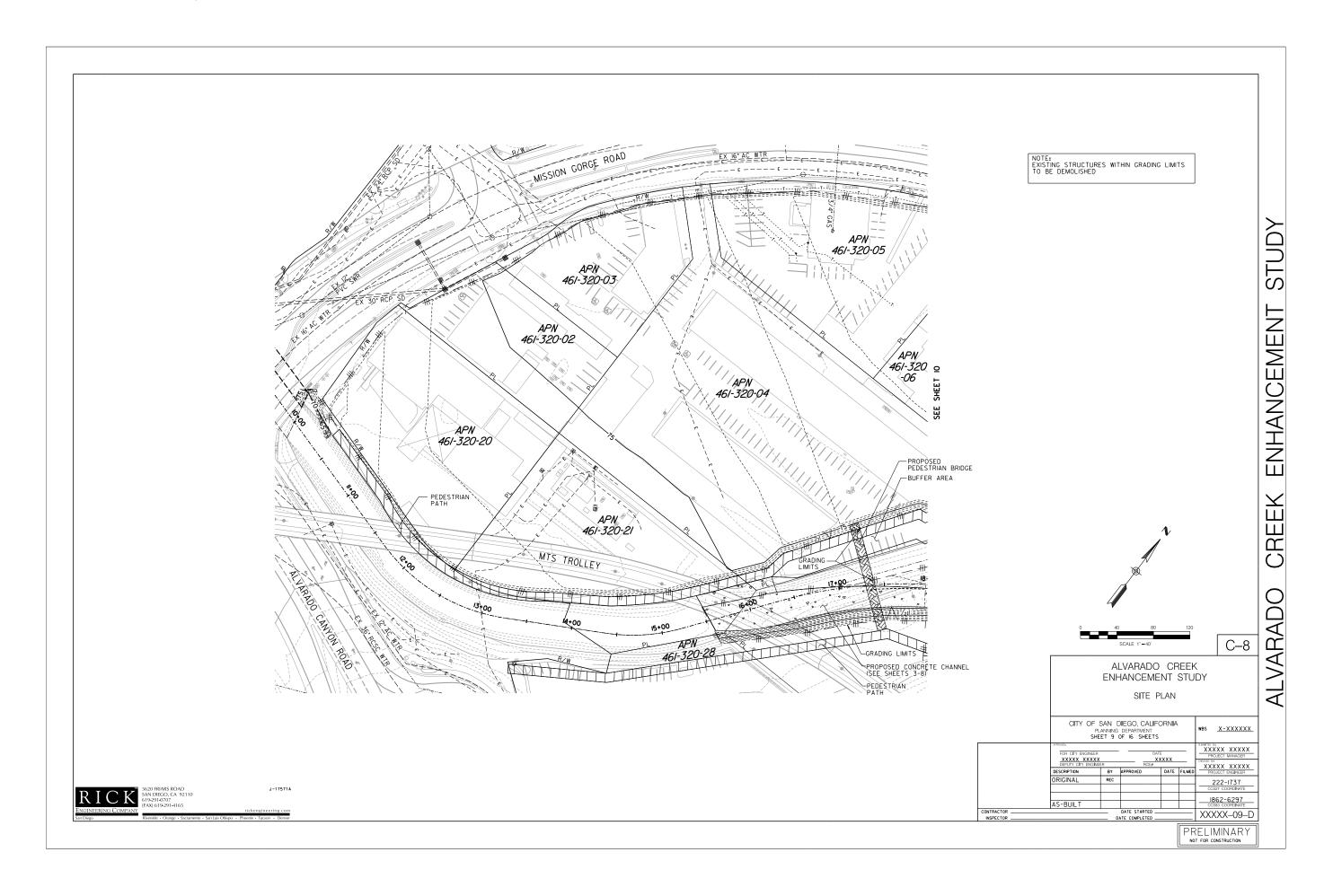


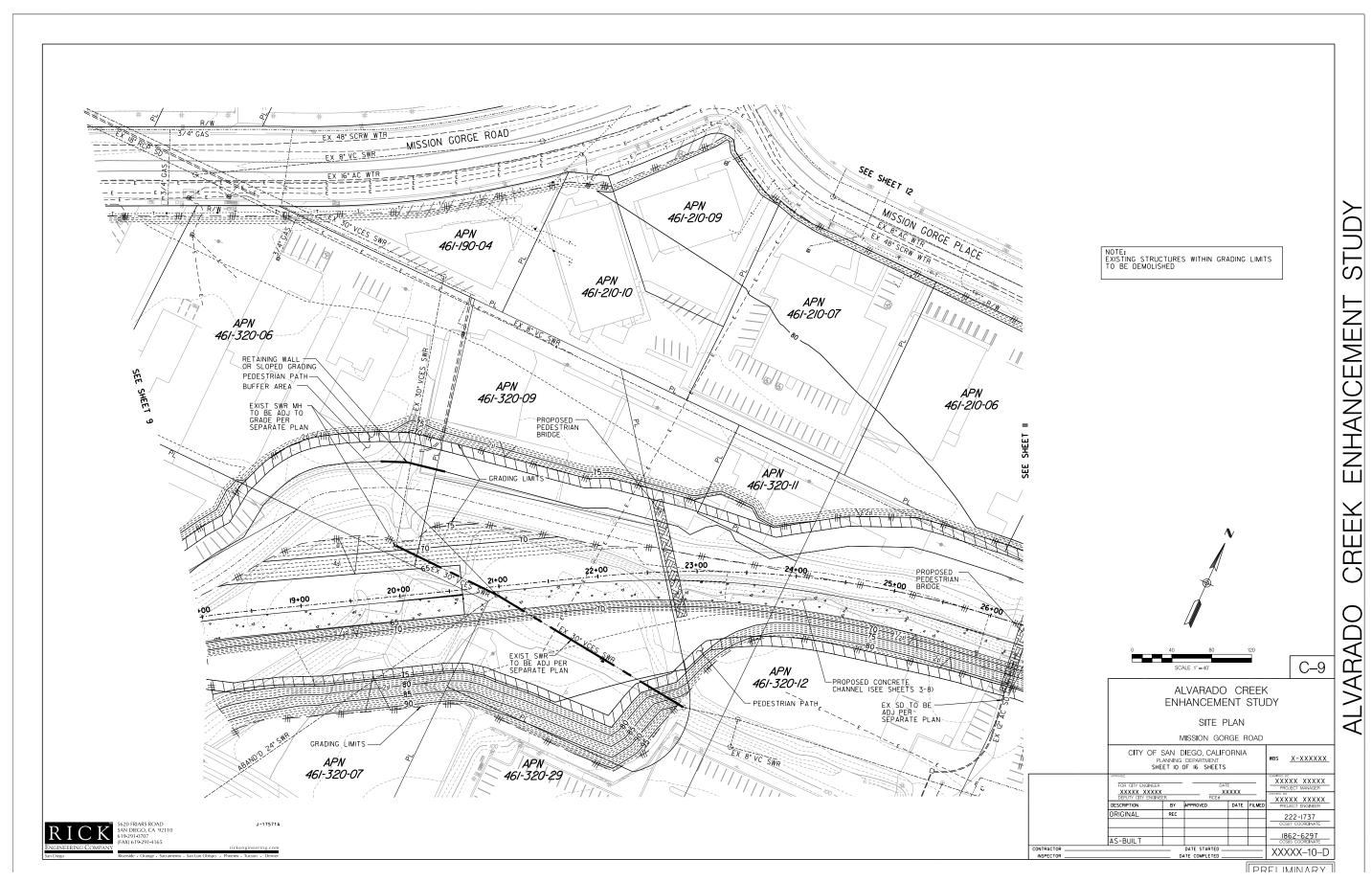


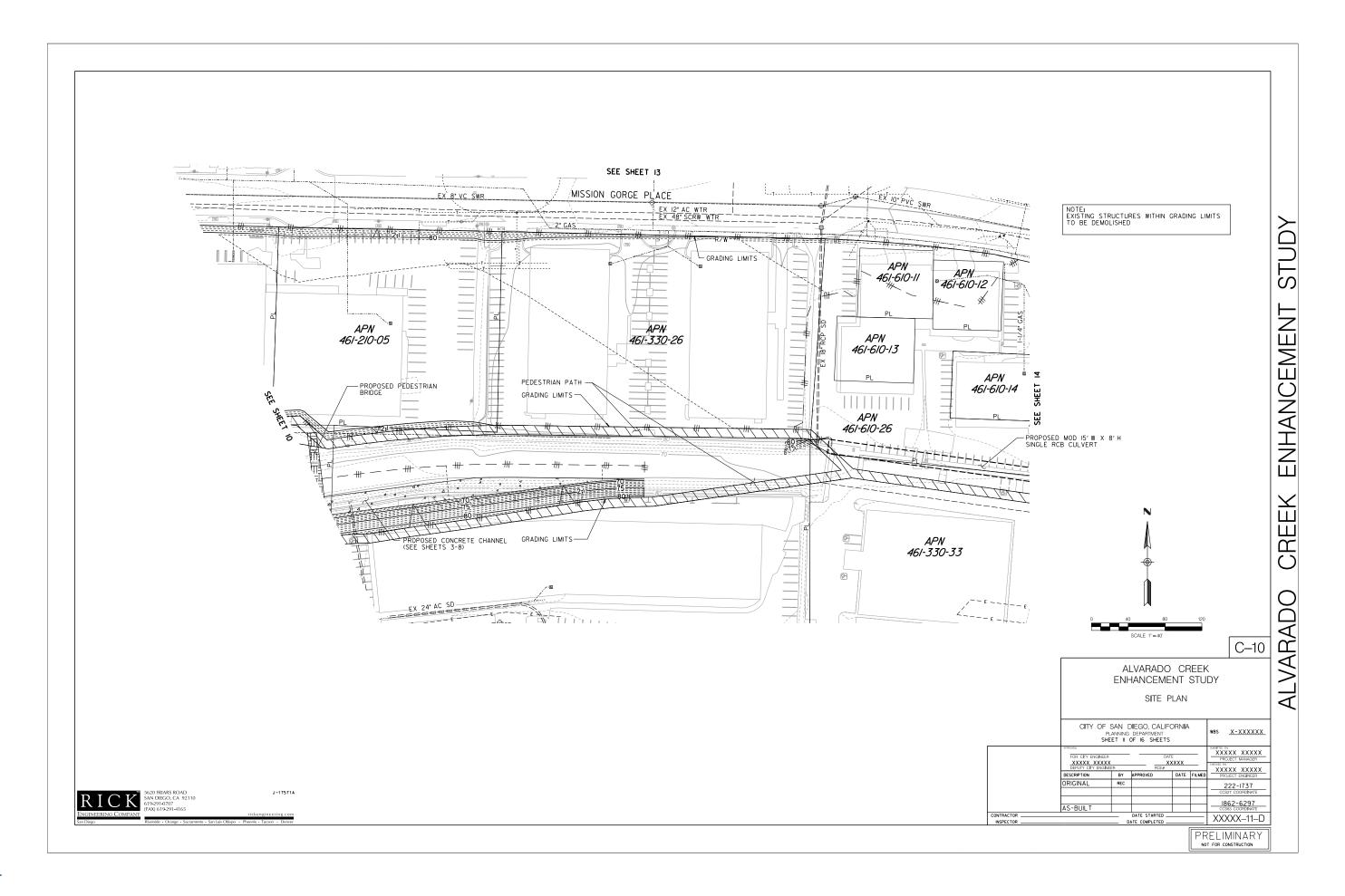


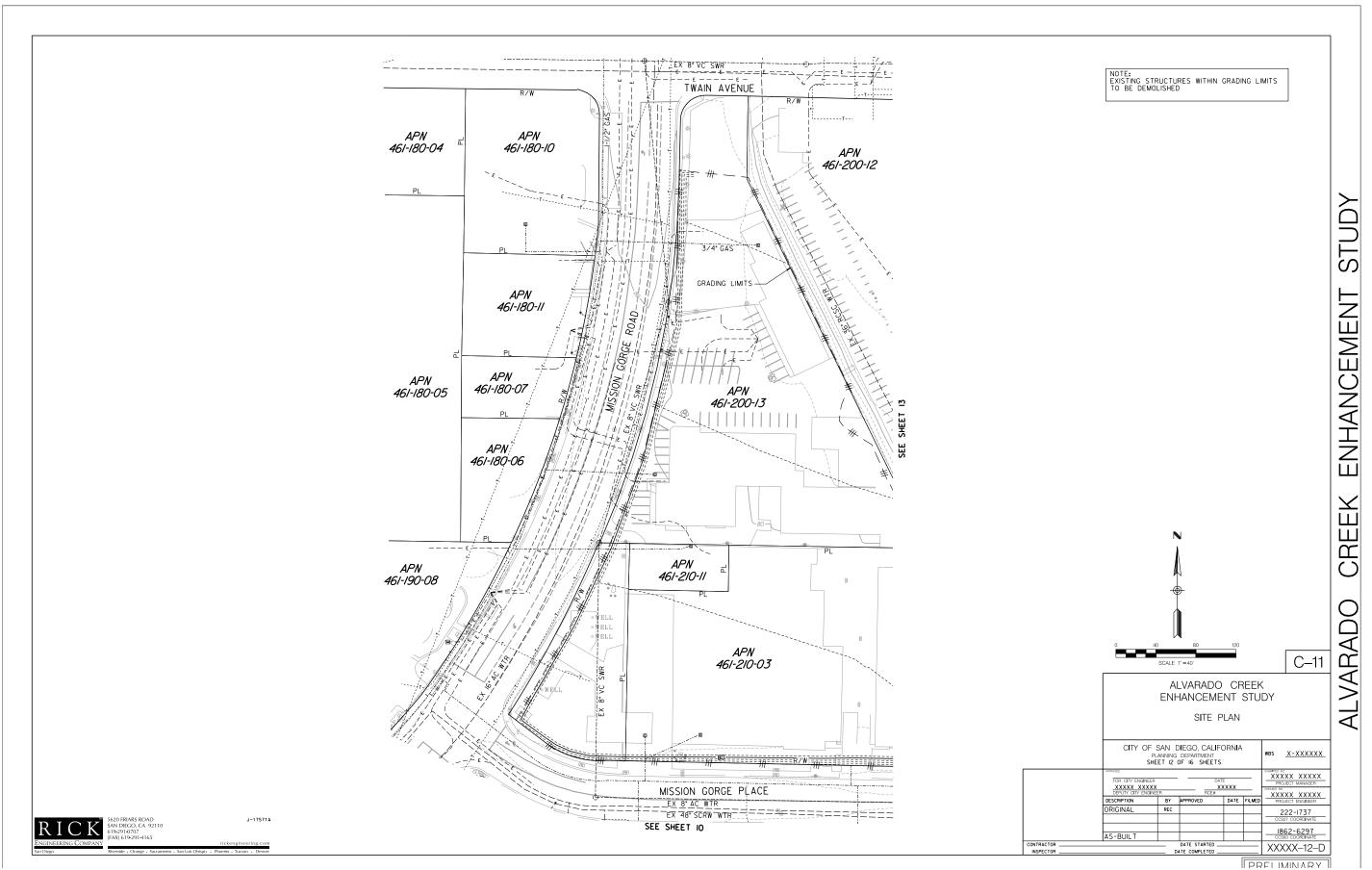


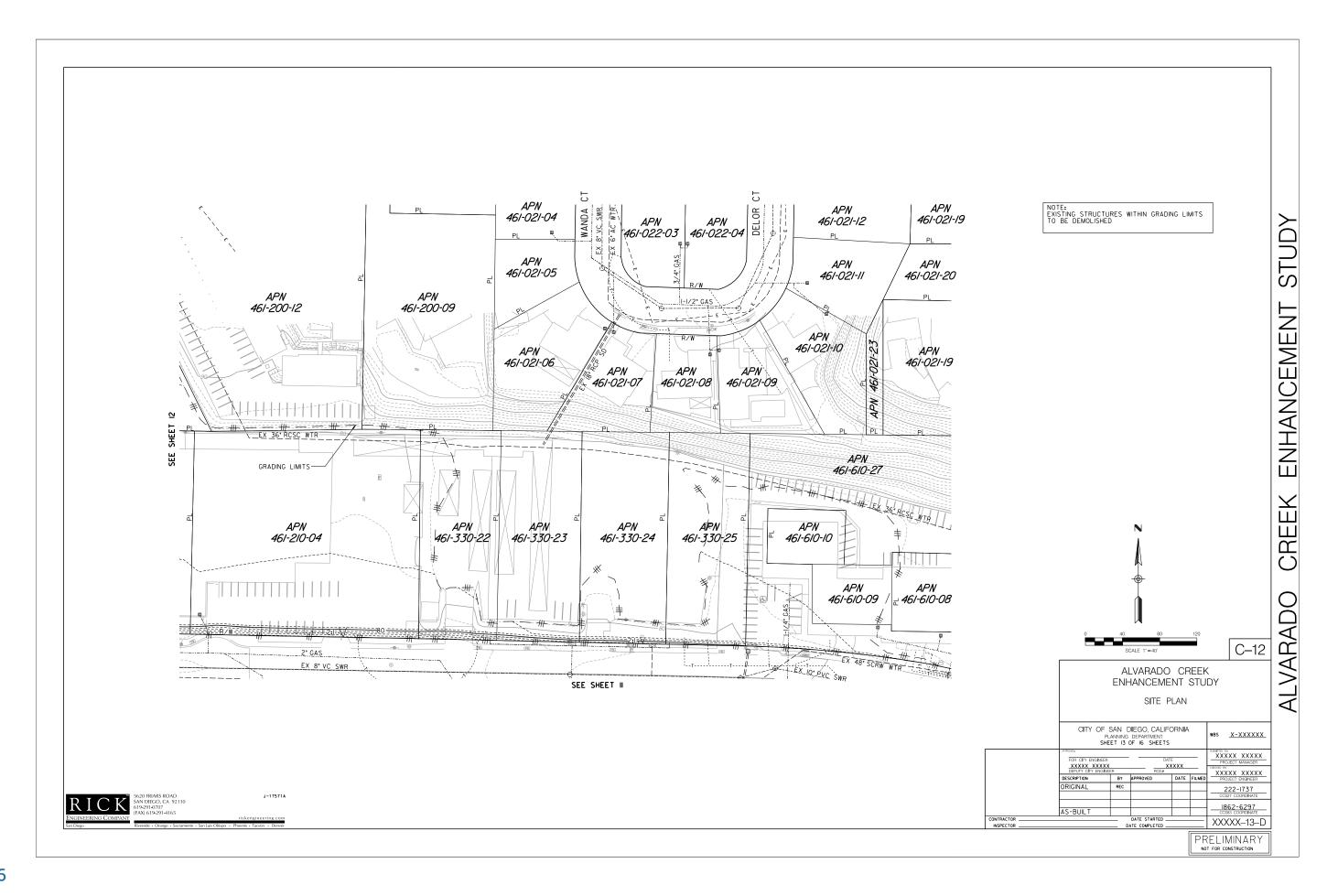


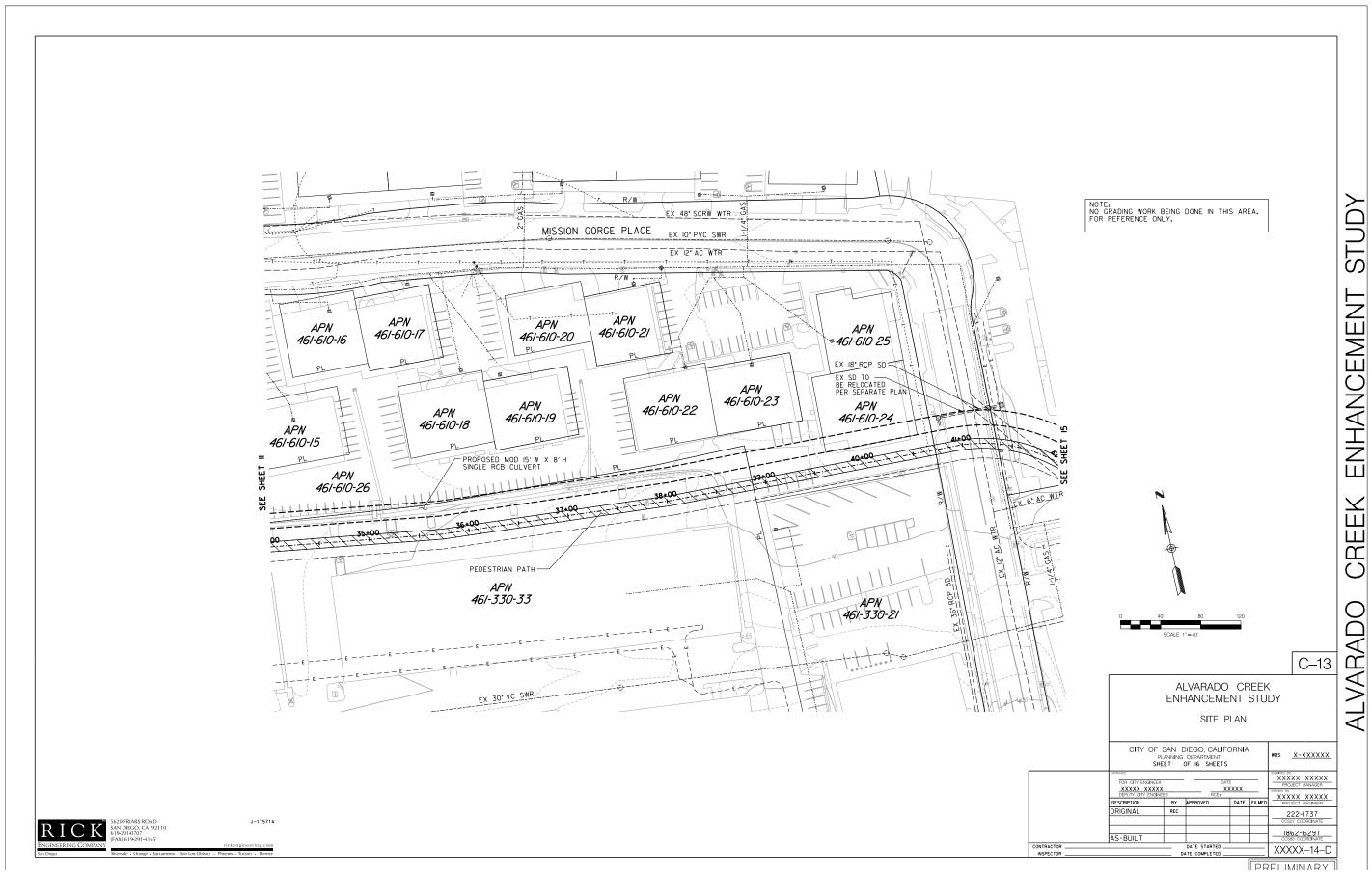


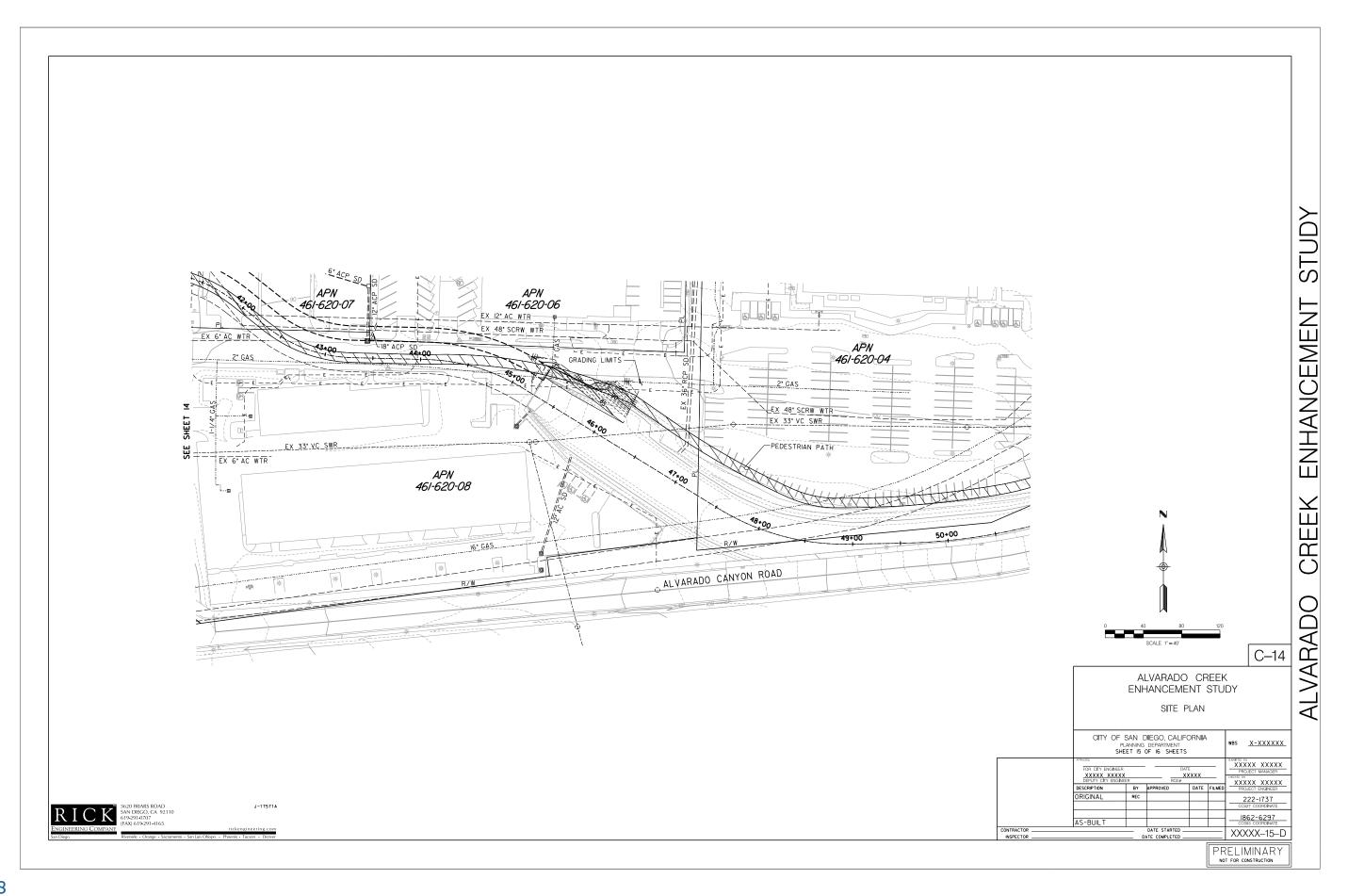


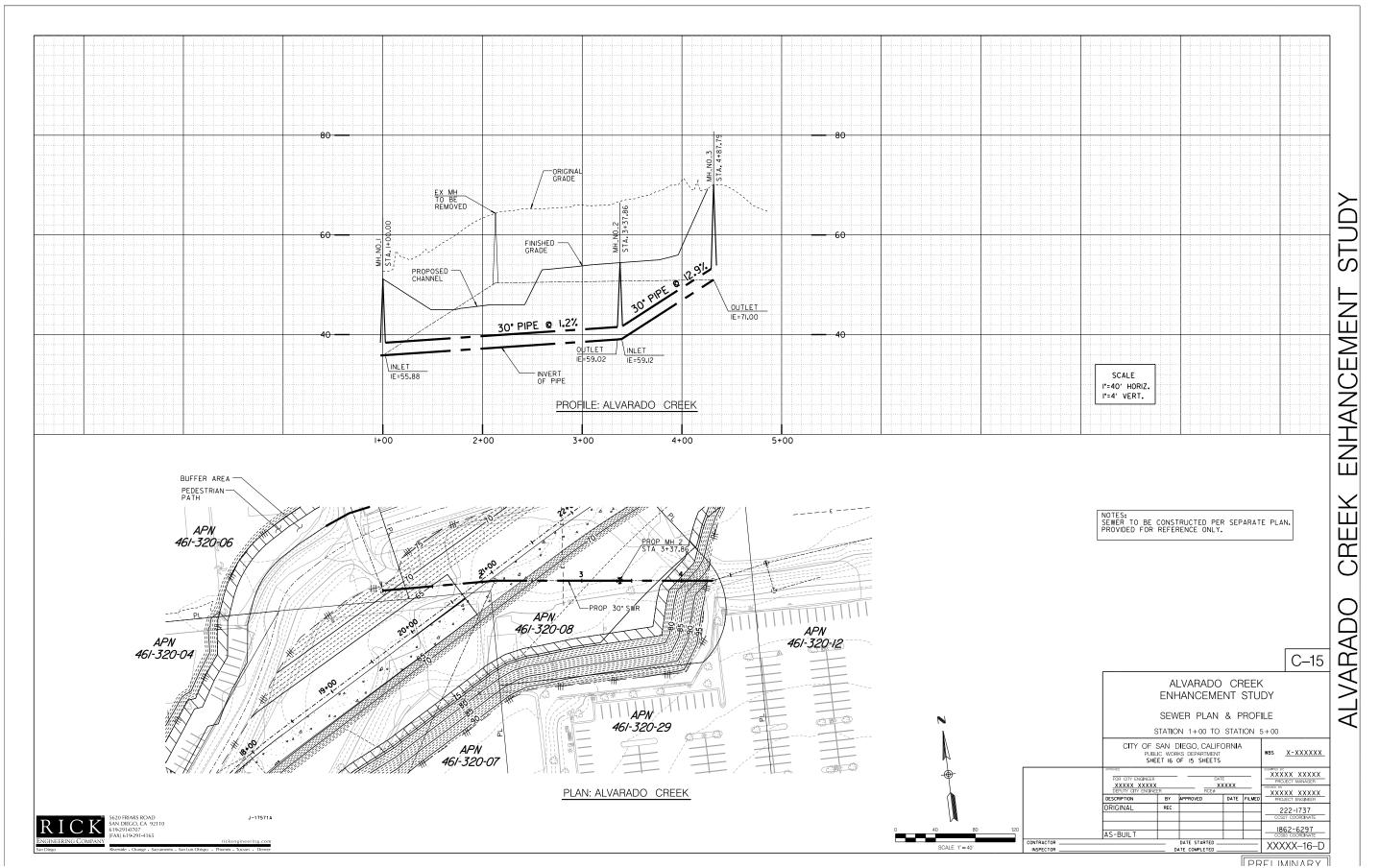




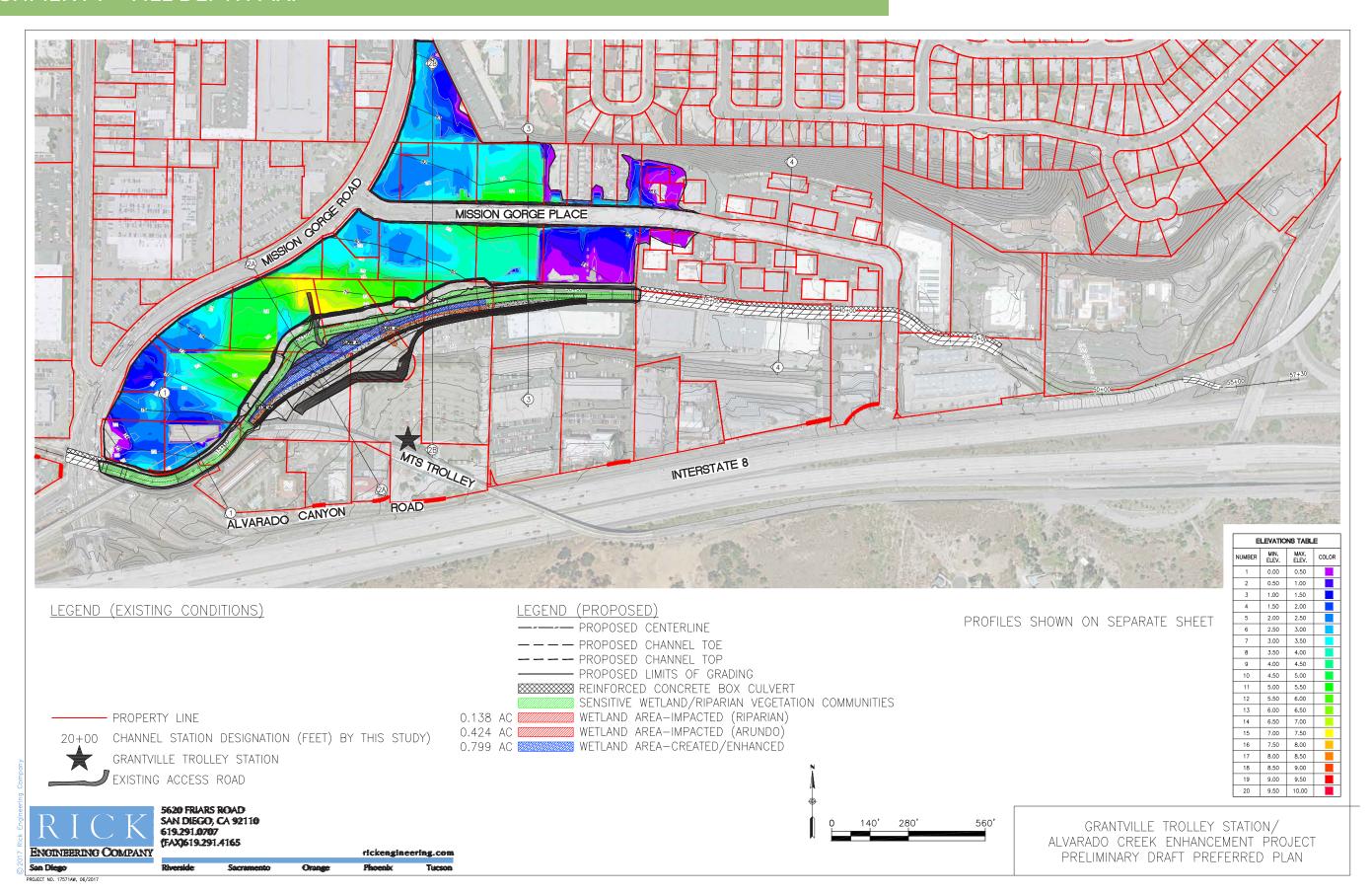








ATTACHMENT 7 - FILL DEPTH MAP





APPENDIXE

COST ESTIMATE

TABLE E-1: ITEMIZED COST ESTIMATE

TITLE:	CDANTVILLE TROLLEY STATION/ALVADADO CREEK	THE ANGEMENT C	TUDY			
IIILE:	GRANTVILLE TROLLEY STATION/ALVARADO CREEK E Perferred Design Alternative - Preliminary Estimate	ENHANCEWEN 1 3	זעטוז			
CONTD.	ACT NO.:					
	EER'S OPINION:			\$10,392,200		
_	ERS OPINION. CT MANAGER:			ΦΙ Ο,332,200		
FROJEC	71 WANAGER.					
				ENGINEER'S	ENGINEER'S OPINION	
ITEM	DESCRIPTION	QUANTITY		UNIT	AMOUNT	
	Preconstruction Services					
	Mobilization	1	LS	\$4,000.00	\$4,000.00	
	Subtotal				\$4,000.00	
	Landscape Irrigation					
	Temporary Irrigation	3,580	SF	\$0.50	\$1,790.00	
	Subtotal				\$1,790.00	
	Miscellaneous Surface Improvements					
<u> </u>	Porous Concrete	1,310	CY	\$450.00	\$589,500.00	
	Subtotal				\$589,500.00	
	Grading					
	Clear and Grub	3,580	SF	\$0.91	\$3,265.23	
	Excavate and Export	8,330	CY	\$20.00	\$166,600.00	
	Subtotal				\$169,865.23	
	Miscellaneous Improvements	<u> </u>				
	Pedestrian Bridge	3,600	SF	\$320.00	\$1,152,000.00	
	Subtotal				\$1,152,000.00	
	Drainage	<u> </u>				
	PCC Box Culvert	1,470	LF	\$2,700.00	\$3,969,000.00	
	PCC Drainage Channel	1,510	LF	\$1,072.20	\$1,619,022.00	
	Subtotal	<u> </u>			\$5,588,022.00	
	Sewer		_			
	30" PVC Sewer Main	340	LF	\$194.00	\$65,960.00	
	Sewer Manhole	1	EA	\$1,135.00	\$1,135.00	
					\$67,095.00	
			Cons	truction Subtotal	\$7,572,300	
		SWPPP			\$28,000	
		Field Orders (if applicable)			\$20,000	

NOT FOR CONSTRUCTION.
FOR STUDY PURPOSES ONLY.

Construction Subtotal	\$7,572,300
SWPPP	\$28,000
Field Orders (if applicable)	\$20,000
Eng. Design (5% of Const. Subtotal)	\$378,700
Contingency (20% of Const. Subtotal)	\$1,514,500
Const. Bond (5% of Const. Subtotal)	\$378,700
Environmental Permitting/Monitoring	\$500,000
Project Total	\$10,392,200

Notes:

^{1.} Details of fill area not included in this study. Cost associated with fill import, landscaping, improvements, etc. for the fill area north of Alvarado Creek are not included in this cost estimate.



APPENDIXF

SUMMARY OF COMMUNITY ENGAGEMENT



NAVAJO COMMUNITY PLANNERS: FLOODING SUBCOMMITTEE MEETING

September 7, 2016 | 1-3 PM

AGENDA

1-1:15pm: Welcome and Introductions

1:15pm: Meeting begins

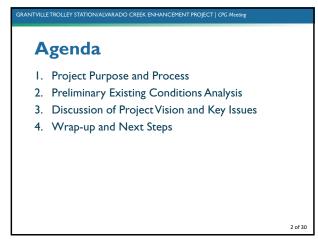
Meeting Purpose

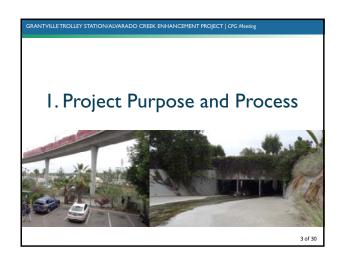
- Review the Alvarado Creek Enhancement Project update process and purpose
- Discuss planning group members' vision for the Project and key concerns
- Solicit input from community members on their priorities for the Project

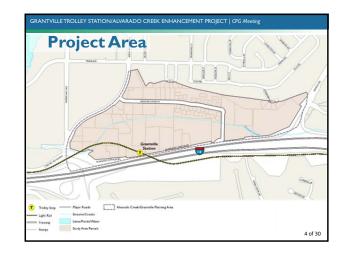
Agenda

- 1. Alvarado Creek Enhancement Project Purpose and Process (10 minutes)
 - a. Scope and schedule
 - b. Relationship to other planning efforts
 - c. Public outreach program
- 2. Existing Conditions Analysis (15 minutes)
- 3. Discussion of Project Vision and Key Issues (up to one hour)
- 4. Any Other Public Comment (10 minutes)
- 5. Wrap-Up and Next Steps



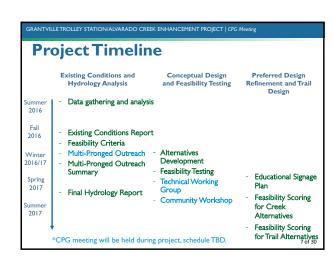


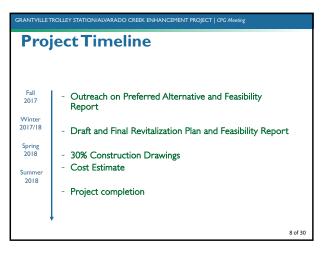


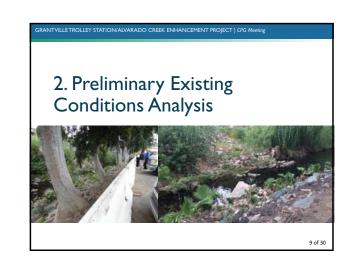






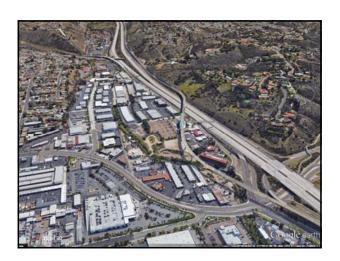


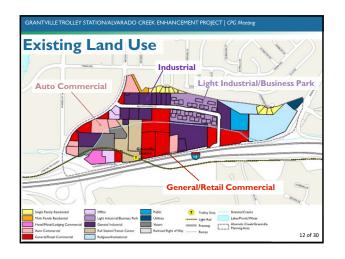


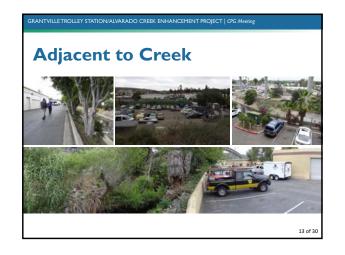


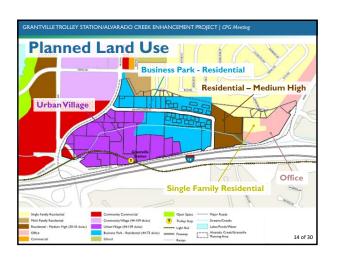


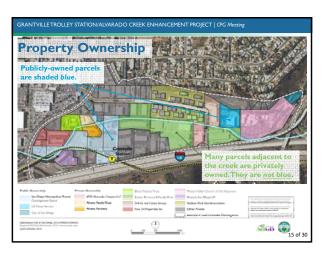
6 of 30

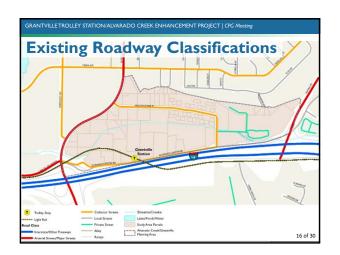


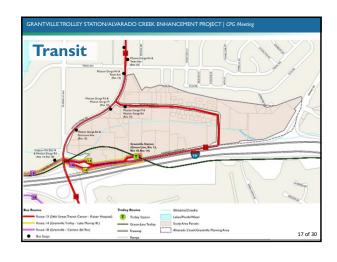


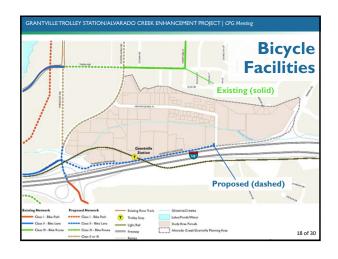




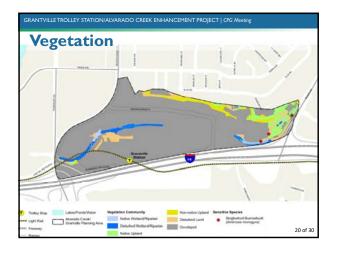


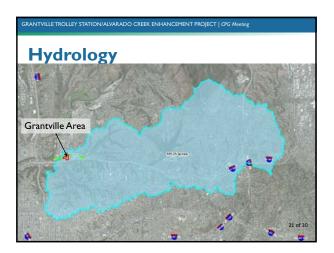


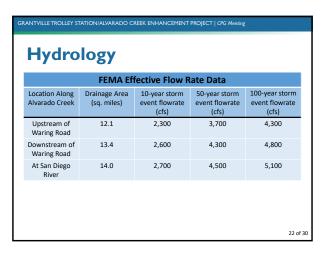


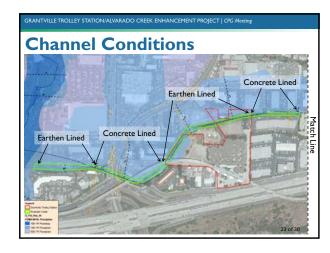


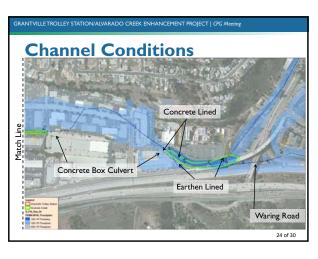


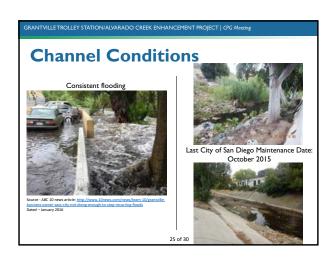




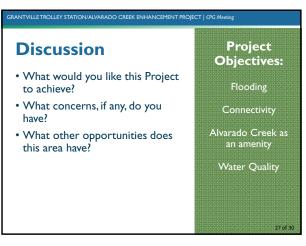


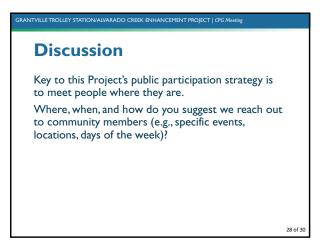












4. Next Steps

• Existing Conditions Report (Fall 2016)

• Multi-Pronged Outreach (Fall 2016)

• Develop channel improvement and trail alignment alternatives (Winter 2016 – 2017)

29 of 30

Thank you and keep in touch!

Seth Litchney, Project Manager
SLitchney@sandiego.gov
(619) 236-6892

2

NAVAJO COMMUNITY PLANNERS: FLOODING SUBCOMMITTEE MEETING

January 11, 2017 | 6:30-8 PM

AGENDA

1-1:15pm: Welcome and Introductions

1:15pm: Meeting begins

Meeting Purpose

- Review the Alvarado Creek Enhancement Project update process and purpose
- Discuss planning group members' vision for the Project and key concerns
- Solicit input from community members on their priorities for the Project

Agenda

- 1. Call to Order
- 2. Roll Call of Board Members
- 3. Modifications to Agenda (2/3 vote requirement to change Information items to Action Items prior to agenda adoption)
- 4. Minutes: Approval of the December 14, 2016 Meeting Minutes

Officers Report

Chair – Matt Adams Vice Chair – Jay Wilson Treasurer – Marilyn Reed

Elected Officials' Reports

5. Public Comment on Non-Agenda Items (3 minutes each)

- 6. Informational Presentations:
 - a. Alvarado Storm Drain/Creek Project: Lisa Lind
 - b. San Carlos Library Design: David Pfeifer, architect

Action Items:

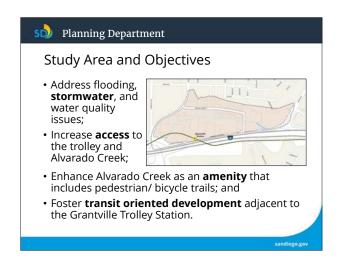
Election Committee – Creation of the NCPI Election Committee for the Wednesday, March 8, 2017 Board of Directors election.

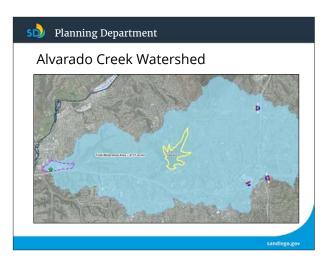
Community Group Reports:

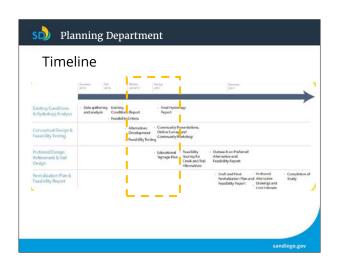
- Allied Gardens Community Council
- Del Cerro Action Council
- San Carlos Area Council
- Mission Trails Regional Park Advisory Board

Old Business / New Business / Adjourn

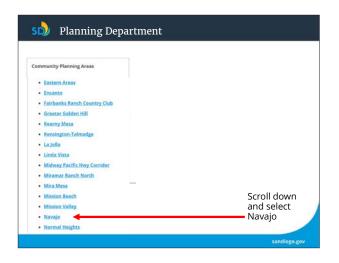


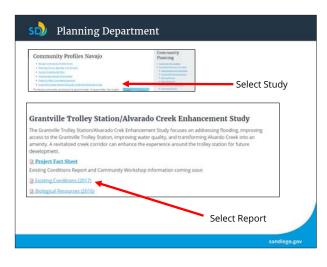


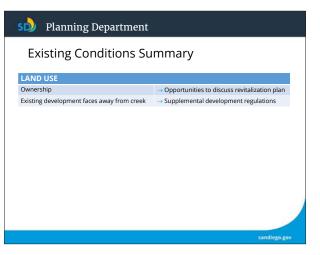


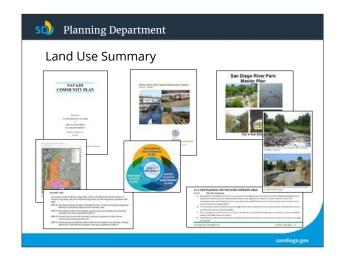






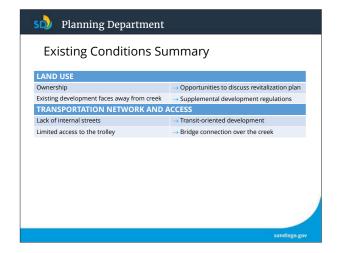








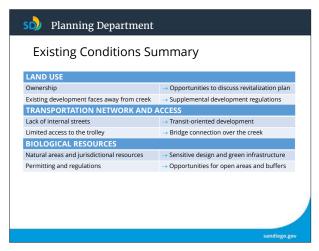




5D Planning Department

Hydrology and Water Quality

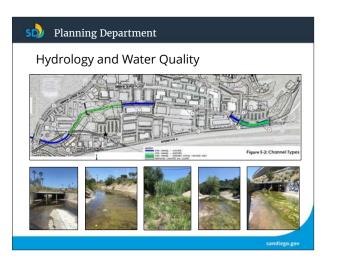














DEL CERRO ACTION COUNCIL AGENDA

January 26, 2017 | 7 PM

AGENDA

- 1. Call meeting to Order
- 2. Pledge of Allegiance
- 3. Approval of January Agenda
- 4. Approval of the October DCAC General Meeting Minutes
- 5. Community Police Report
- SDPD Community Relations Officer John Steffen
- SDSU Police Community Resource Officer Mark Peterson and Nicole Borunda, Community Relations Manager for SDSU
- 6. DCAC Officers Report
- President Report
- Vice President Report
- Treasurer Report
- 7. Elected Officials Report:
- Councilmember Scott Sherman: Liz Saidkhanian, Council Representative for Del Cerro
- Congresswoman Susan Davis Representative

- 8. Main Topics:
- Lisa Lind, Senior Planner for the City and Project Manager for resolving the Alvarado Creek/
 Storm Drain Flooding issue in Grantville, will provide a Power Point presentation on the "Grantville Trolley Station/Alvarado Creek Enhancement Study"
- Mark Rawlins, Friends of Del Cerro, provide update on the Del Cerro Maintenance Assessment District.
- 9. Old Business
- 10. New Business
- 11. Next Meeting Thursday, April 27 7 p.m. in Temple Emanu-El
- 12. Adjourn



COMMUNITY WORKSHOPS

February 13 & 15, 2017



You're invited to participate

Grantville Trolley Station/ Alvarado Creek Enhancement Study

Community Workshops

February 13TH 5:30-7:00 PM

Mission Trails Regional Park Visitor Center One Father Junipero Serra Trail

AND

February 15[™] 11:30 AM − 1:00 PM

Junior Achievement San Diego 4756 Mission Gorge Place

(Directions provided on back)

The study is being led by the City of San Diego, and is funded by a Smart Growth Incentive Program grant from the San Diego Association of Governments (SANDAG). An Existing Conditions Report is available on the study area at this website:

www.sandiego.gov/planning/community/profiles/navajo#Grantville Trolley Station/Alvarado Creek Enhancement Study







Objectives

Address flooding, stormwater, and water quality

Increase access to the Grantville Station and Alvarado Creek

Enhance Alvarado Creek as an amenity that includes pedestrian/bicycle trails

Foster transit oriented development adjacent to the Grantville Station



Contact Lisa Lind (619) 236-6531 ind@sandiego.gov

1010 Second Ave. Suite 1100, San Diego, CA 92101 | (619) 235–5200 | planning@sandiego.gov | sandiego.gov/planning

Directions to Community Workshops

February 13th: Mission Trails Regional Park Visitor Center, One Father Junipero Serra Trail

The entrance to the Visitor Center parking lot is on the left, just before the pipe gate, off Father Junipero Serra Trail.

Visitor Center

<u>From I-8</u> - Take Interstate 8 to the Mission Gorge Road/Fairmount Avenue exit. Turn north onto Mission Gorge Road and proceed for 4.2 miles. When you pass the Jackson Drive intersection, move into



the left lane. Look for the large wooden Mission Trails Regional Park sign on the left side of the road, and turn left onto Father Junipero Serra Trail.

<u>From Highway 52</u> - Take Highway 52 to the Mast Blvd. exit. Driving from the west, turn left onto Mast Blvd, go under the freeway to the first traffic signal at West Hills Parkway, and turn right. Driving from the east, exit Mast Blvd. and turn right. Turn right again at the first traffic signal onto West Hills Parkway and right onto Mission Gorge Road. Proceed south on Mission Gorge Road for 2.4 miles. Look for the large wooden Mission Trails Regional Park sign on the right side of the road, and turn right onto Father Junipero Serra Trail.

February 15th: Junior Achievement San Diego, 4756 Mission Gorge Place

Park in space that does not have a "Reserved" sign. Alternate parking is available at the Mission Church of the Nazarene at the bottom of the hill. Please contact the Junior Achievement office should you have any trouble finding the building: (619) 682–5155.

Junior Achievement

<u>From I-8 Heading West</u> - Exit Waring Road, heading north. Take a sharp right onto Adobe Falls Road. Continue over the



bridge, heading west, onto what is now Alvarado Canyon Road. Make a right turn at the first stop sign, which is Mission Gorge Place. Make a right into the second driveway, heading east. Pass the post office and take the next left turn up the hill to the large white building.

<u>From I-8 Heading East</u> - Take Interstate 8 East to Mission Gorge Road. Make a left onto Mission Gorge Road. Make 1st right under the bridge onto Alvarado Canyon Road. (Turn away from Starbucks and Chili's.) Follow the freeway to your right, obeying the yield sign (rather than the stop sign to the left). Continue on Alvarado Canyon to the stop sign at Mission Gorge Place. Make a left turn onto Mission Gorge Place. Take an immediate right onto the second driveway, heading east. Pass the post office and take the next left turn up the hill to the large white building.

1010 Second Ave. Suite 1100, San Diego, CA 92101 | (619) 235-5200 | planning@sandiego.gov | sandiego.gov/planning

BOARDS

GRANTVILLE TROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY

FLOOD CONTROL/CAPACITY AND CREEK AMENITIES

One goal of this study is to address the issue of flooding in the Study Area. The following is a list of flood control strategies and creek amenities that will help mitigate flooding in the Study Area.

CREEK RE-ALIGNMENT

DEEPEN CHANNEL

RAISE BANK HEIGHT WIDEN CHANNEL REACHES

NATURAL CHANNEL BOTTOM

ENHANCE CHANNEL SIDES (GABIONS)

REDUCE BARRIERS/ CULVERTS

TRAIL ALIGNMENT

LINEAR PARKS POCKET PARKS

HABITAT BUFFERS GREEN STREETS

FLOOD CONTROL INFRASTRUCTURE PROJECT EXAMPLES







Habitat Buffers



Pocket Park



Natural Channel Bottom



Trail Alignment



Green Street



Linear Park



Reduce Barriers



Enhance Channel Sides (gabions)

GRANTVILLETROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY

ALVARADO CREEK OVERVIEW

Within the Study Area, the Alvarado Creek channel extends approximately 4,400 feet from Mission Gorge Road to Waring Road onramp to Westbound I-8. The portion of the creek in the Study Area is a series of open channels and box culverts and is divided into several reaches, identified as 1 through 6 and Mission Gorge Road.

Opportunities for Enhancement

The Navajo Community Plan and the San Diego River Park Master Plan have provided the following recommendations for Alvarado Creek:

- Improve water flow for Alvarado Creek to connect to the San Diego River.
- Enhance pedestrian and bicycle connectivity to the creek and to Grantville Trolley Station
- Improve open space and trail connections with Alvarado Canyon and Navajo Canyon.

REACH CAPACITY SUMMARY				
Reach	Channel Type	Bottom Width	Approx. Length	Existing Capacity
Mission Gorge Road	Triple RCB	33	128	10-year
1	Concrete Trapezoidal	29.5	550	25-year
2	Vegetated	13 to 16	1,100	2-year
3	Concrete Trapezoidal	29.5	550	25-year
4	Triple RCB	33	1,340	25-year
5	Concrete Trapezoidal	45	175	25-year
6	Concrete & Vegetated Trapezoidal	44	550	<25-year



GRANTVILLETROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY

ALTERNATIVE 1 - Linear Park

FLOOD MANAGEMENT

- Widening the creek channel bottom in Reaches 1 through 3*
- New underground culvert adjacent to the existing underground culvert in Reach 4
- Floodable linear park adjacent to the creek in Reaches 1 through 3

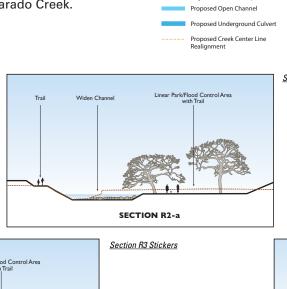
ACCESS AND AMENITY

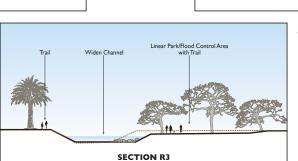
- Linear park adjacent to the creek in Reaches 1 through 3
- Recreational corridor over the new and proposed culverts in Reach 4
- Pedestrian/bicycle bridges over the creek in Reaches 1, 2, and 3
- Multi-use trails along the creek*
- Trail connections to the San Diego River and Navajo CanyonTrail*
- Green street along Mission Gorge Place*
- * Indicates a feature common to all alternatives.

SECTION RI

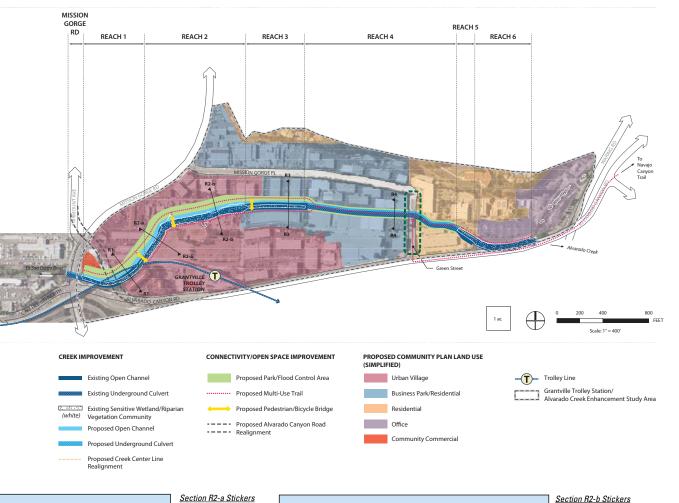
EXERCISE

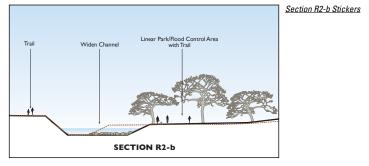
Place a sticker next to the Cross-Section that indicates, out of the three alternatives, your preferred treatment for each Reach of Alvarado Creek.

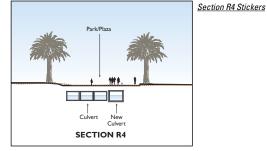




Section R1 Stickers







GRANTVILLETROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY GORGE **ALTERNATIVE 2 - Pocket Parks** REACH 1 REACH 2 REACH 3 REACH 6 **FLOOD MANAGEMENT** • Widening the creek channel bottom in Reaches 1 through 3* • New open channel with a predominantly natural bottom, adjacent to the existing underground culvert in Reach 4 • Floodable pocket parks to the north and the south of the creek channel in Reaches 1 and 2 **ACCESS AND AMENITY** • Pocket parks to the north and the south of the creek channel in Reaches 1 and 2 • Pedestrian/bicycle bridges in Reaches 1 through 4 • Multi-use trails along Alvarado Creek* • Trail connections to the San Diego River and Navajo Canyon Trail* • Green street along Mission Gorge Place* PROPOSED COMMUNITY PLAN LAND USE (SIMPLIFIED) * Indicates a feature common to all alternatives. Urban Village Grantville Trolley Station/ Alvarado Creek Enhancement Study Area ···· Proposed Multi-Use Trail Business Park/Residential Existing Underground Culvert **EXERCISE** ---- Proposed Alvarado Canyon Road Office Place a sticker next to the Cross-Section that indicates, out of the three alternatives, your preferred treatment for each Reach of Alvarado Proposed Creek Center Line Realignment Creek. Section R2-b Stickers Section R1 Stickers Section R2-a Stickers SECTION RI SECTION R2-a SECTION R2-b Section R3 Stickers Section R4 Stickers **SECTION R4 SECTION R3**

GRANTVILLETROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY

ALTERNATIVE 3 - Grand Park

FLOOD MANAGEMENT

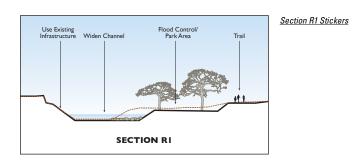
- Widening the creek channel bottom in Reaches 1 through 3*
- New concrete, open channel adjacent to the existing underground culvert in Reach 4
- Large, floodable parks on both sides of the channel in Reaches 1 through 3

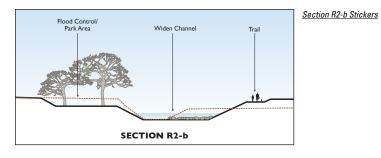
ACCESS AND AMENITY

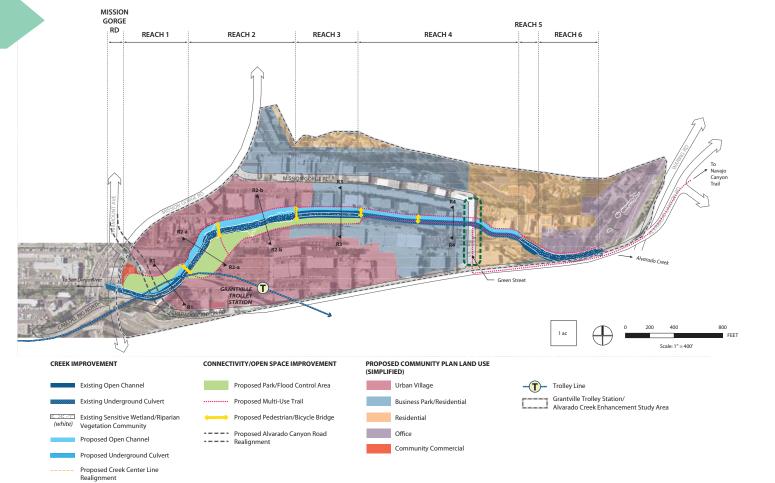
- Large parks to the north of Reach 1 and to the south of Reach 2 and 3
- Pedestrian/bicycle bridges in Reaches 1 through 4
- Multi-use trails along Alvarado Creek*
- Trail connections to the San Diego River and Navajo Canyon Trail*
- Green street along Mission Gorge Place*
- * Indicates a feature common to all alternatives.

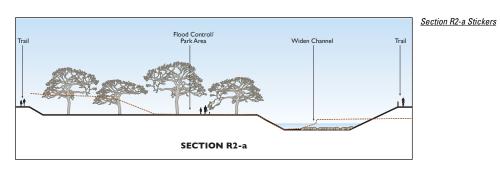
EXERCISE

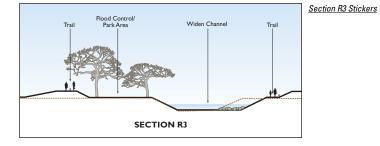
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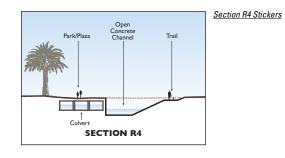






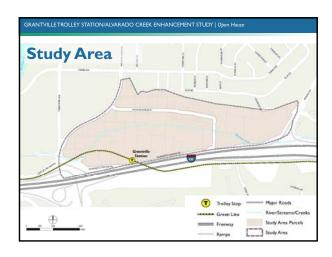


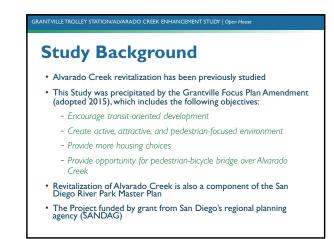


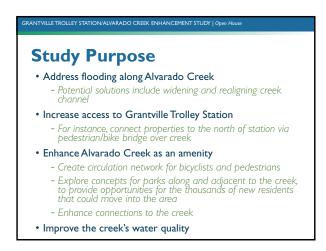




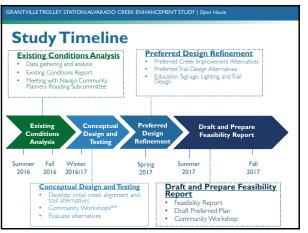


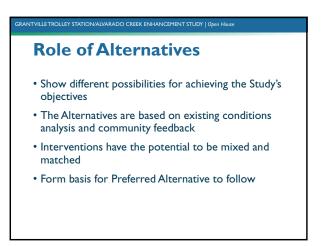






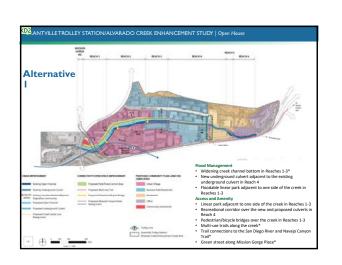


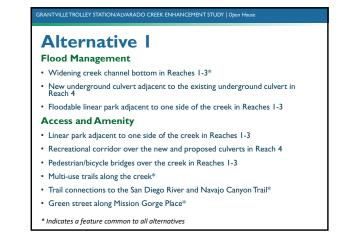


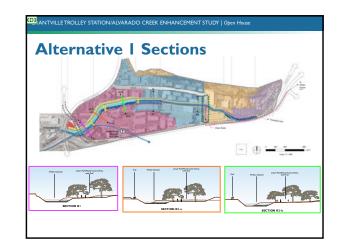


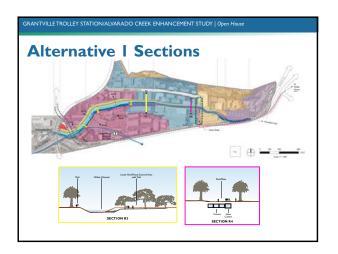
Open House Format • Please visit each of the stations around the room • City staff and consultants will be present at each station to answer questions and facilitate discussion • Fill out a comment card for additional feedback and thoughts on the alternatives!



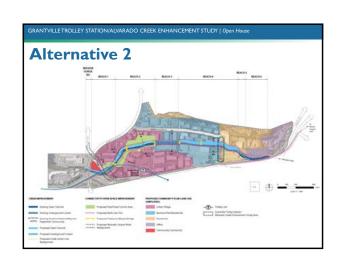


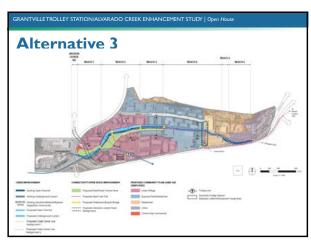


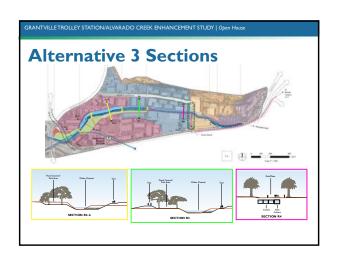


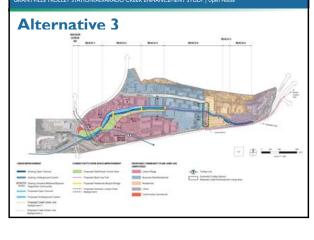


Alternative 2 Sections



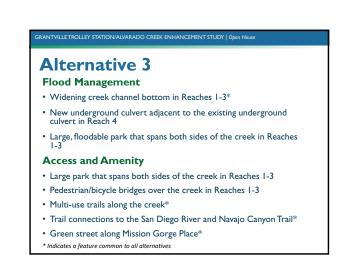


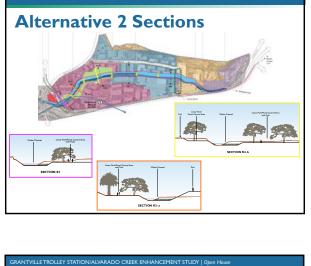


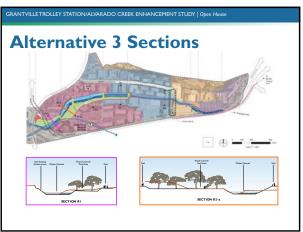




Alternative 2 Flood Management • Widening creek channel bottom in Reaches I-3* New open channel with a predominantly natural bottom, adjacent to the existing underground culvert in Reach 4 - Floodable pocket parks to the north and the south of the creek channel in Reaches 1 and 2 $\,$ **Access and Amenity** Pocket parks to the north and the south of the creek channel in Reaches I and 2 • Pedestrian/bicycle bridges over the creek in Reaches I-4 Multi-use trails along the creek* • Trail connections to the San Diego River and Navajo Canyon Trail* • Green street along Mission Gorge Place* Indicates a feature common to all alternatives











NAVAJO COMMUNITY PLANNERS: FLOODING SUBCOMMITTEE MEETING

April 14, 2017 | 10:30 AM - 12 PM

AGENDA

10:30-10:40am: Introductions

10:40pm: Meeting begins

Meeting Purpose

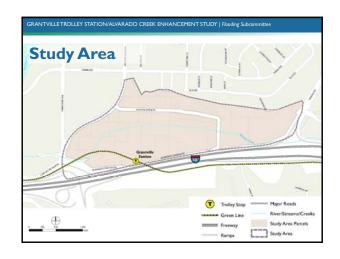
- Provide Schedule and Status Update for Alvarado Creek Enhancement Study
- Provide Summary of Community Workshops and Alternatives Development
- Discuss Flooding Subcommittee members' input on feasibility criteria and Draft Preliminary
- Preferred Alternative

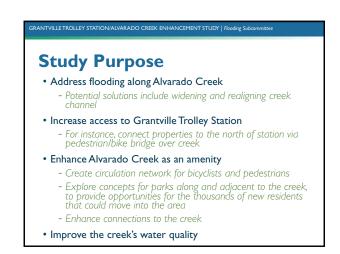
Agenda

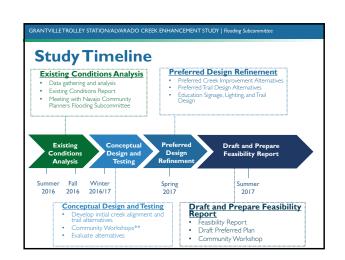
- 1. Alvarado Creek Update (10 minutes)
 - a. Study Objectives
 - b. Schedule
 - c. Review of Alternatives Development and Community Workshops Results
- 2. Discussion of Feasibility Criteria (15 minutes)
- 3. Discussion of Draft Preliminary Preferred Alternative (40 minutes)
- 4. Any Other Public Comment (10 minutes)
- 5. Wrap-Up and Next Steps (5 minutes)

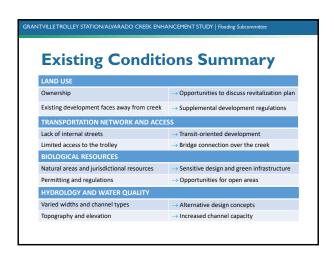


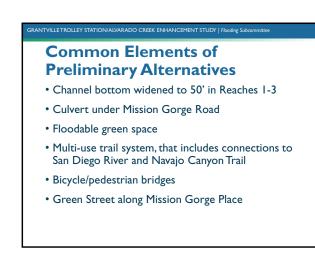


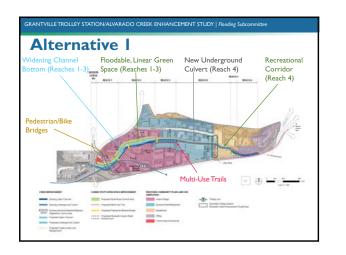


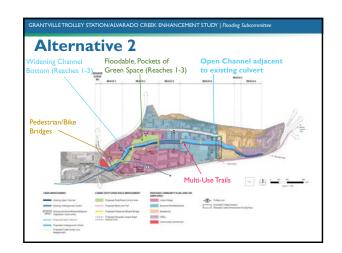


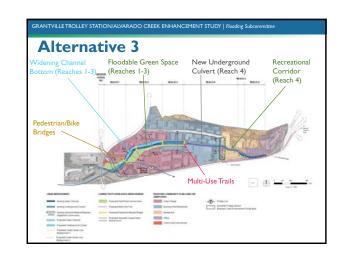




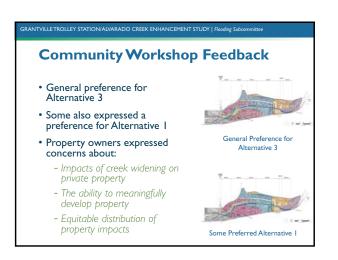












Feasibility Criteria

- Flooding Is Managed
- Water quality is improved and stormwater is managed
- Impacts to private property are minimized
- Proposed creek alignment minimizes, to the extent feasible, impacts on adjacent private property
- To the extent proposed creek alignment impacts private property, owners maintain the ability to meaningfully develop property

Feasibility Criteria (cont'd)

- Biological impacts are minimized
- Impacts to the sensitive vegetative communities in the Study Area (Reach 2) are mitigated
- There is no net loss to wetland communities and a wetland buffer is provided
- Connections within the Study Area are increased
- This includes:
- Increasing access to the Trolley Station
- Providing bicycle/pedestrian bridges over the creek
- Incorporating a continuous 10'-wide multi-use trail

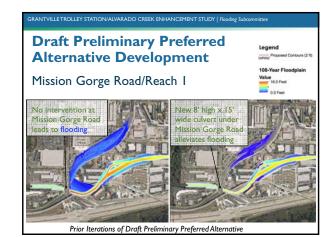
Feasibility Criteria (cont'd)

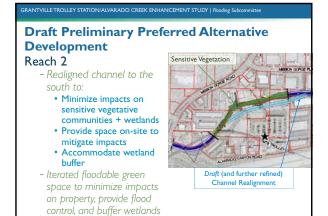
- Alvarado Creek is transformed into an amenity
- Open space for new residents
- Views are opened up
- Educational opportunities incorporated into trail design
- The proposed design is context-sensitive
 - Materials in the channel are natural or use existing infrastructure
- The proposed improvements are feasible
 - Estimated costs
- Permitting/regulatory concerns
- Concerns of property owners and community members are taken into account

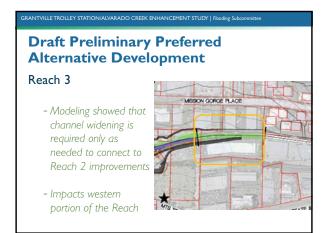
Feasibility Criteria Exercise

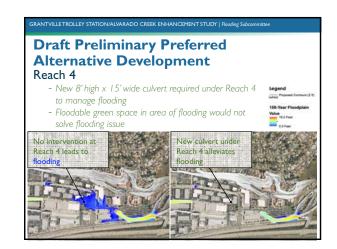
Draft Preliminary Preferred Alternative Development

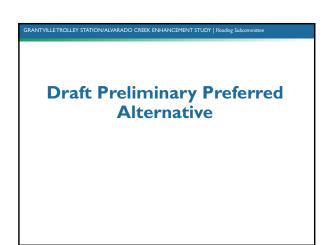
- Draft Preliminary Preferred Alternative based on:
 - Existing conditions analysis
 - Community feedback
 - Stakeholder feedback
- Feasibility criteria
- Hydraulic modeling
- Mixed, matched, and adjusted Reaches from the 3 preliminary alternatives

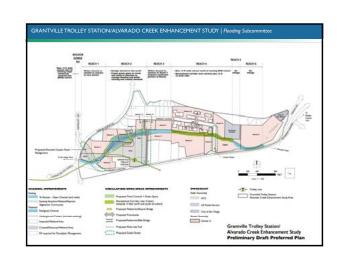


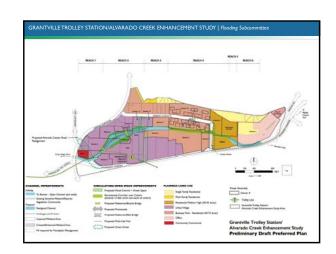


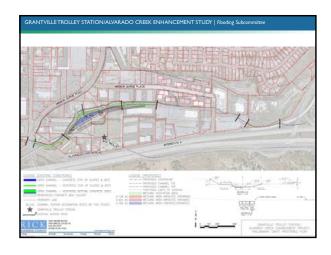


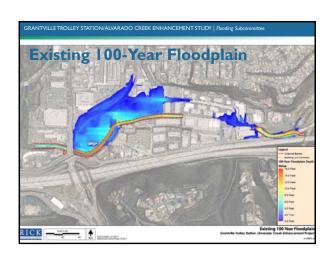


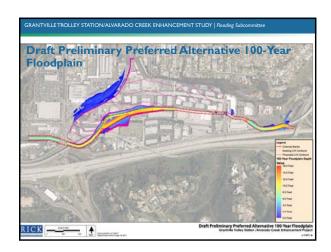












ANTVILLETROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY | Flooding Subcommitte

Grant Options

- SANDAG- fall/winter 2017-2018
- Smart Growth Incentive Program (SGIP) capital funds
- Active Transportation Grant Program (ATGP)
- Proposition I funding
- California Urban Rivers Grant Program- fall 2017
- Storm Water Grant Program- spring 2018
- Storm Water Resource Plan
- Capital Improvements Program

GRANTVILLE TROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY | Flooding Subcommittee

Questions/Comments

GRANTVILLETROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY | Flooding Subcommitted

To provide additional feedback and keep in touch:

Lisa Lind, Project Manager <u>LLind@sandiego.gov</u> (619) 236-6531

Visit the Study website to access materials! http://tiny.cc/alvaradocreekstudy

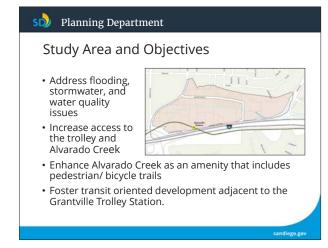


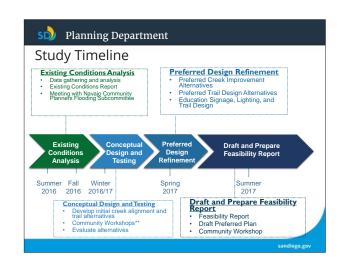
NAVAJO COMMUNITY PLANNERS MEETING

May 10, 2017

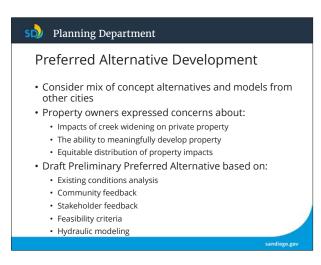
PRESENTATION















NAVAJO COMMUNITY PLANNERS: FLOODING SUBCOMMITTEE MEETING

June 9, 2017 | 1-2:30 PM

AGENDA

1:00 - 1:10pm: Introductions

1:10pm: Meeting begins

Meeting Purpose

- Present Refinements to Preferred Alternative
- Discuss Flooding Subcommittee members' comments and questions on Draft Preferred Alternative

Agenda

- 1. Preferred Alternative Presentation (20 minutes)
- 2. Discussion of submitted comments and questions (40 minutes)
- Density and Supplemental Development Regulations
- Cross section components and uses in green space/flood control area
- Coordination between property owners and other agencies
- 3. Additional comments (10 minutes)
- 4. Revitalization Study Schedule (10 minutes)

2:30pm: Meeting adjourns

Summer Fall Winter 2016 2016 2016/17

Conceptual Design and Testing





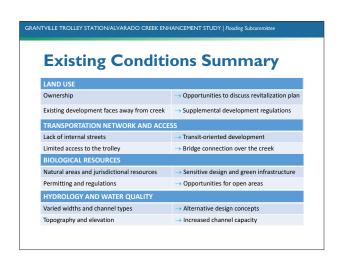
2017

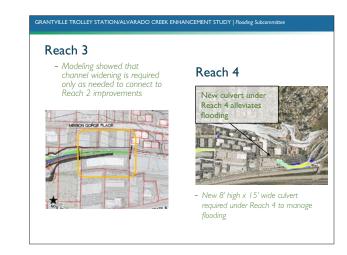
Draft and Prepare Feasibility Report

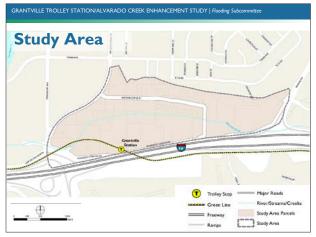
Draft Preferred Pla

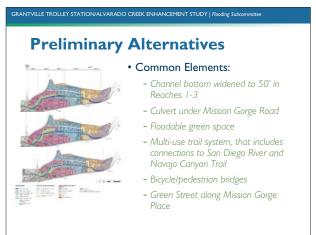


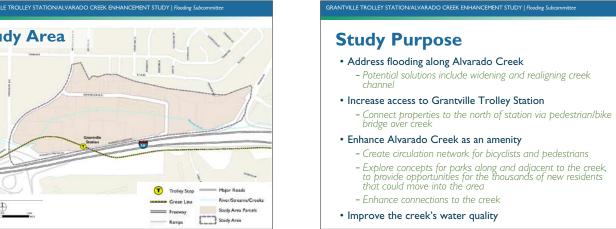






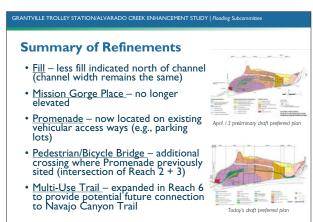












RANTVILLE TROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY | Flooding Subcommittee

Summary of Additional Questions

- Allowed density
- Alternate materials and flood control with vertical walls
- Uses/Activities in Buffer and Green Space Areas:
- Landscaping
- Storm water retention and treatment
- Paths, observation decks, structures, etc.

RANTVILLE TROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY \mid Flooding Subcommittee

Summary of Additional Questions

- Multi-use Paths and Bridge Connections
- Multiple benefits
- Proximity to creek, wetland buffer, floodable green space, and development area
- Location of bridge crossings
- Permitted placement of bridge pillars
- Concerns with public access
- Permitting and Resource Agencies:
- Coordination and certainty

ANTVILLE TROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY | Flooding Subcommitt

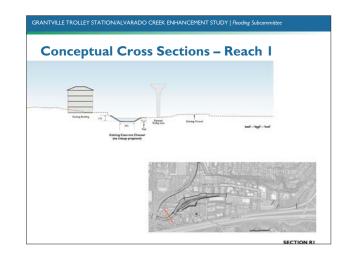
Summary of Additional Questions

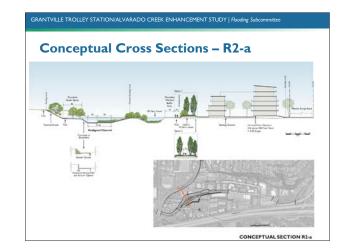
- Discussion of Conceptual Cross Section for Reach 1
- Additional clarification and discussion:
- Setback requirement from the wetland buffer
- Preference for Option 1 for Reach 2, with fire lanes, private streets and plaza/paseos up to the pedestrian trail or the edge of the wetland buffer.
- Wetland Buffer areas as Environmentally Sensitive Lands (ESL)
- Distinguish "Floodable Green Space" and "Wetland Buffer"

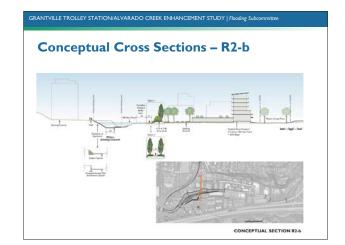
ANTVILLE TROLLEY STATION/ALVARADO CREEK ENHANCEMENT STUDY | Flooding Subcomm

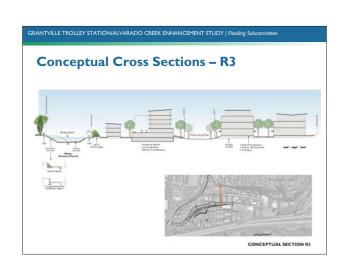
Summary of Additional Questions

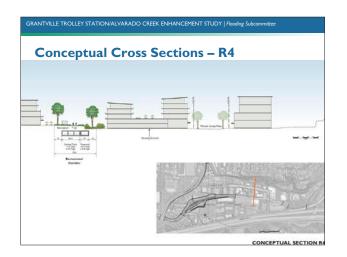
- Additional clarification and discussion (cont'd):
- Understand location of multi-use trail on north and south side of creek in relation to proposed wetland buffer
- Difference between Section Reach 1 and Section Reach 2a transition
- Gabion or terraced walls allowed within the buffer areas or only the realigned channel walls
- Cantilever decks, patios, etc. be allowed to hang over the wetland buffer area or floodable green space
- Responsibility for the maintenance
- Wetland buffer areas to comply with storm water requirements and other BMPs
- Incentives to property owners











Questions/Comments

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ALVARADO CREEK REVITALIZATION STUDY

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