

City of San Diego La Jolla Area of Special Biological Significance Site Specific Dilution and Dispersion Model June 2013

City of San Diego





Prepared by: AMEC Environment and Infrastructure, Inc. Project No. 5025121039



LA JOLLA AREA OF SPECIAL BIOLOGICAL SIGNIFICANCE SITE SPECIFIC DILUTION AND DISPERSION MODEL

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May 2013

AMEC Project No. 5025121039



This special study report is prepared for the City of San Diego, Transportation and Storm Water Department (City of San Diego) to complement multiple, ongoing storm water monitoring programs within the La Jolla Area of Special Biological Significance, No 29, located in La Jolla, California. In particular, this study is designed to provide a quantitative, site specific dilution and dispersion model to aid in determination of appropriate dilution factors per guidance provided in the California Ocean Plan (2009).

1.0 REGULATORY BACKGROUND

On October 18, 2004, the State Water Resources Control Board (State Water Board) notified the City of San Diego, as a responsible party, to cease storm water and non point source waste discharges into Areas of Biological Significance (ASBS) or to request an exception from the California Ocean Plan waste discharge prohibition. On December 15, 2004, the City of San Diego requested an exception for ASBS No. 29. On March 20, 2012, the State Water Board adopted Resolution No 2012-0012, approving an exception to the California Ocean Plan, General Exception to the California Ocean Plan for Areas of Special Biological Significance Waste Discharge Prohibition for Storm Water and Nonpoint Source Discharges, with Special Protections (herein referred General Protections). These general protections are in accordance with the Porter-Cologne Water Quality Control Act, California Water Code §13000 et seq., and implementing regulations, including the current Ocean Plan (2009).

Under the California Ocean Plan (2009), Section III, C.4.a, Effluent limitations for water quality objectives listed in Table B shall be determined through the use of the following equation:

Ce = Co + Dm (Co - Cs)

where (in ug/l):

Ce = the effluent concentration limit,

Co = the concentration (water quality objective) to be met at the completion of initial dilution

Cs = background seawater concentration (references in Table C of Ocean Plan)

Dm = minimum probable initial dilution expressed as parts seawater per part wastewater.

Furthermore, the Ocean Plan provides guidance for the identification of dilution models for use in determining the initial dilution, Dm. Specifically, per Section III, C.4.a;

The Executive Director of the SWRCB shall identify standard dilution models for use in determining Dm, and shall assist the Regional Board in evaluating Dm for specific waste discharges. Dischargers may propose alternative methods of calculating Dm, and the Regional Board may accept such methods upon verification of its accuracy and applicability.



2.0 HYDRODYNAMIC MODELING DESIGN

This study is designed to provide site specific dilution and dispersion model results for ASBS No. 29 to the San Diego Regional Water Quality Control Board (SDRWQCB). The effluents from three permitted outfalls within the La Jolla ASBS were studied using the SEDXPORT hydrodynamic modeling system. The model is designed to numerically simulate dry weather and wet weather case scenarios. The dilution study incorporated historical site specific outfall data on water mass boundary properties (bathymetry, salinity, temperature, ocean level/tides) and forcing functions (waves, currents and winds).

The SEDXPORT modeling system was developed at Scripps Institution of Oceanography (SIO) for the US Navy's Coastal Water Clarity System and Littoral Remote Sensing Simulator. The model has been reviewed and vetted by multiple regulatory agencies and has been calibrated for six previous water quality projects in the Southern California Bight. Due to the large number of oceanographic measurements that have been made in ASBS No 29 and the adjacent ASBS No 31, the model for this site is considered particularly robust. These include long term water quality, wind, wave, water height, tidal and temperature measures at the SIO pier as well as multiple special studies of currents and storm water inputs into La Jolla Bay and environs.

This model was selected as the most relevant model available for the following reasons;

- 1) The State Water Board required SIO to perform a similar study to determine the initial dilution and dispersion of the discharge during storm and non-storm periods at the adjacent ASBS No. 31 (San Diego-Scripps) as a requirement of SIO's NPDES Permit Exception, Order No. R9-2005-0008. SIO used the SEDXPORT modeling system to fulfill this requirement and the study was provided on February 9 2007 to the San Diego Regional Board and the California SWRCB for approval.
- 2) The model design and results of the SIO study were evaluated by the Natural Water Quality Committee (NWQC), and deemed appropriate (Summation of Findings, Natural Water Quality Committee, 2006-2009, SCCWRP Technical Report 625, September 2010). The NWQC was a scientific oversight committee established as a requirement of the SIO permit to evaluate monitoring results and special studies. With technical review and input from the NWQC, the SDRWQCB revised the initial 2:1 dilution factor in the initial waste discharge requirements (WDR) to a 7:1 dilution factor (representative of the minimum dilution ratio) and this 7:1 dilution factor was incorporated into the November 2008 revision of SIO's permit.
- 3) This study utilizes the same modeling program (SEDXPORT) and modeling assumptions and integrating the most recent long term historical trend data. In addition, the model calibration and outputs were generated by the same SIO scientists (Scott Jenkins, Ph.D. and Joseph Wasyl) that published the SDRWQCB approved 2007 SIO study.



4) The mass flow model inputs are based on actual discharge data (flow and mass) measured from permitted outfalls; SDL-157, SDL-062 and SDL-186. To model the beach discharges, flow and chemistry results collected in November 2011 provided data representative of the worse case proxy scenario. These data were merged with the long term probability assessment in order to bracket possible dilution outcomes. The study provide the same data endpoints (dilution ranges of the outer far field [>-10m] of ASBS No. 29 and the near field [<-10m] surf zone dilution ranges) for wet weather discharges under various ocean mixing conditions as used in the aforementioned SIO study. In addition, the extreme worse case probabilities (high storm water flow/low surf and currents) at the zone of initial dilution (ZID) were evaluated to determine minimum dilution factors.

3.0 HYDRODYNAMIC MODELING RESULTS

On behalf of the City of San Diego, AMEC Earth and Infrastructure, Inc. contracted with Dr. Scott A Jenkins Consulting to provide hydrodynamic modeling of storm drain discharges in the vicinity of ASBS No.29. The complete hydrodynamic modeling report is provided as Attachment A.

In order to model possible dilution scenarios, long term trends for boundary and force function data (i.e., 32 year record from 1980-2012) provides a representative basis for the model to incorporate the Pacific Decadal Oscillation (aka alternating periods of strong and weak El Niño). Site specific storm water flow and mass data (three events spanning from November 2011 to March 2012) were obtained from three monitored outfalls within ASBS No 29; SDL-157, SDL-062 and SDL-186.

These data sets (parameters for long term functions and outfall flux/hydrographs) are coupled and the model produces the dilution and dispersion outcomes (aka ranges of dilution factors) by analyzing the possible daily outcomes (maximum and minimum) of the input variables over 32 years (a total of 11,688 distinct combinations). This analysis was performed for each of the outfalls.

These probabilities can be plotted as a density functions and the dilution factors presented relative to the likelihood of occurrence. The resultant density plots span from the highest modeled dilution factor (low flow/high energy ocean conditions) to the lowest "worst case" dilution (high storm water flow/calm ocean conditions). The likelihood of occurrence corresponding to these conditions decreases for both the best and worst case scenarios as the model search criteria plots the possible combinations. See Attachment A, Table 3 for search criteria.

These wet weather worse case scenarios for the dilution and mass are plotted as dilution contour maps, by merging the results for both the offshore and surfzone bathymetry regions. See Attachment A, Figures 30-36. In general, dilutions for storm water range between a minimum of 10² to 10⁴ in the nearshore and dilution magnitude of 10⁴ to 10⁷ characterize the outer half offshore of ASBS No. 29.



In the immediate zone of initial dilution, where there is irreversible turbulent mixing, the dilution factor for 90% of the potential outcomes produced a minimum dilution of 20 to 1 for the 48-inch Outfall SDL-157, and 15 to 1 for the 72-inch SDL-062 outfall using the worse case November 2011 storm event. Of the three flow monitored outfalls, the 36-inch SDL-186 (the Devil's slide) has significantly lower storm water discharges, and therefore correspondingly higher near shore dilution factors. Dilution factors for the SDL-186 near shore area were in the range of 10³.

The largest and single contributing dilution footprint was found at SDL-062, the discharge at the end of Avenita de la Playa Street. This is the largest outfall within ASBS No. 29 and drains the largest fraction of the La Jolla Shores watershed. Therefore, this outfall generates the lowest modeled dilution factors. The median outcome minimum surfzone dilution for SDL-062 is 22 to 1. The minimum worst case value for SDL062, with a probability of occurrence of 0.13%, is 13 to 1 (12.6:1 calculated value) in a peak storm water flow and low energy ocean mixing condition.

4.0 STUDY CONCLUSIONS

Using the SEDXPORT hydrodynamic model, the storm water discharges from monitored outfalls into the La Jolla ASBS No 29 generated dilution factors ranging from 10² in the near shore, to 10⁷ in the seaward boundary during wet weather. Further resolution of the model at the zone of initial dilution (ZID) produced a worse case dilution factor of 15 to 1 for 90% of the possible outcomes for the largest discharge outfall, SDL-062. The extreme worst case (0.13% probability in conditions of high discharge, calm sea state) generated a 13:1 dilution factor for this outfall.

It was this extreme worse case dilution value that was approved by the SDRWQCB as described in hydrodynamic modeling section, item 2 above.

In addition, it is fortuitous that the lowest flow outfall, SDL-186 at the Devils Slide, has the most high value, hard bottom marine habitat and that dilution outcomes are greater than 3 orders of magnitude in the nearshore area There is a hard bottom tidal flat in the immediate vicinity of the Devils Slide intertidal zone that is exposed at low tide, that may be subject to acute fresh water exposure during storm events. Conversely, the higher flow SDL-062 discharges across the southern border of La Jolla Shores beach and discharges into the surf zone onto sandy soft bottom marine habitat.

The high dilution factors expected in the vicinity of the Devils Slide area of ASBS No.29 is consistent with other localized water quality measurements during storm water discharge from Outfall SDL-186. These include the La Jolla Shores ASBS Protection Implementation Program administered under Prop 84 funding (Grant agreement No 10-413-550, La Jolla Shores Watershed Management Group [LJSWMG], 2008) and, ASBS No. 29 compliance monitoring under the General Protections, and a corresponding bioaccumulation study (AMEC, 2013).



APPENDIX A

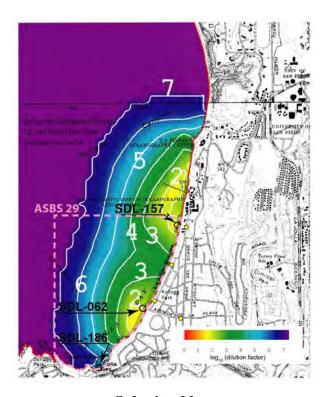
HYDRODYNAMIC MODELING OF STORM DRAIN DISCHARGES IN THE NEIGHBORHOOD OF ASBS 29 IN LA JOLLA, CALIFORNIA



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HYDRODYNAMIC MODELING OF STORM DRAIN DISCHARGES IN THE NEIGHBORHOOD OF ASBS 29 IN LA JOLLA, CALIFORNIA

by: Scott A. Jenkins, Ph. D. and Joseph Wasyl



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Final: 12 May 2013

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Revised: 13 March 2013

EXECUTIVE SUMMARY

The La Jolla Shores Coastal Watershed consists of 1,639 acres, primarily residential and institutional (e.g., the University of California, San Diego [UCSD] campus) land uses. Subdrainages within the watershed boundary drain west into two areas of special biological significance (ASBS): the San Diego-Scripps ASBS (ASBS 31) and the La Jolla ASBS (ASBS 29). The majority of the runoff from the watershed is conveyed through a network of storm drains before it is discharged at several locations along the beach. Hydrodynamic model analysis of the dilution and dispersion of shoreline discharges into ASBS 31 was previously done by Jenkins and Wasyl (2007) for the University of California San Diego. The present hydrodynamic analysis applies those same analysis methods in evaluating shoreline discharges of storm water into nearshore waters surrounding ASBS 29. The central and largest portion of the La Jolla watershed affecting ASBS 29 drains to only two storm drain outfalls, SDL-157 at the northern boundary of ASBS 29 and SDL-062 at Avenida de la Playa. During the 2011-2012 wet weather monitoring season (October 1, 2011 through April 30, 2012), storm water samples were collected from outfall discharge and receiving water mixing zone monitoring locations during three events (AMEC, 2012). These data were used to initialize the source loading inputs to a hydrodynamic model (SEDXPORT) used in the present receiving water analysis of storm water dilution and dispersion.

The *SEDXPORT* hydrodynamic modeling system was developed at Scripps Institution of Oceanography for the US Navy's *Coastal Water Clarity System* and *Littoral Remote Sensing Simulator*. This model has been peer reviewed multiple agencies and has been calibrated and validated in the Southern California Bight for six previous water quality and design projects.

Based on previous protocols established with the San Diego Regional Water Quality Control Board for storm drain dilution analysis in ASBS 31 (Jenkins and Wasyl, 2007), the numerical modeling study of the beach discharges is based on a wet weather worst-case scenario with a long-term probability assessment to bracket the envelope of the possible outcomes. Among the three wet-weather events monitored by AMEC (2012), Wet Weather Event-1 (20-21 November, 2011) comes closest to matching a computer search of marine environmental conditions for the wet-weather worst-case proxy. Analysis of the worst-case wet weather proxy was conducted over the entire coastal domain of La Jolla Bay and the Torrey Pines Littoral Sub-Cell.

The footprint of the dilution field for the wet weather worst case scenario, (involving the simultaneous discharge from storm drains SDL-157, SDL-062 and SDL-186), spreads about a kilometer seaward of the shoreline and about 2 kilometers along the shoreline, covering most of ASBS 29 and intruding into the southern portion of ASBS 31. The dilution factors of storm water in both ASBS 29 and ASBS 31 range between a minimum of 10^2 near shore to 10^7 along the seaward boundaries during the wet weather worst case. Dilution factors of 10^4 to 10^7 characterize the outer one-half of ASBS 29, while dilution factors 10^2 to 10^4 characterize the inner one-half. In the immediate neighborhood of SDL-157 and SDL-062, very near the shoreline of ASBS 29, dilution factors are as low as 40 to one.

AMEC, Environment & Infrastructure, Inc. Hydrodynamic Modeling of Storm Drain Discharges in the Neighborhood of ASBS 29 in La Jolla, California

Revised: 13 March 2013

The preponderance of the storm water discharge plume during the wet weather worst case scenario is from SDL-157 and SDL-062, while SDL-186 at Devil's Slide makes a very minor contribution. This is a fortuitous outcome in the sense that a significant amount of high-value, hard bottom marine habitat lives in the southern end of ASBS 29, and in the immediate neighborhood of SDL-186. Dilution factors of storm water discharged from SDL-186 are at most 10^3 near the shore in the nearfield of SDL-186, and more typically 10^4 to 10^5 elsewhere along the bluff-faced shoreline of the southern end of ASBS 29. The largest single contributing footprint to the combined storm water plume appears to be from SDL-062 at the end of Avendia De La Playa that produces a large patch with 10^2 minimum dilution very near the shoreline that spreads south along the shore to the La Jolla Beach and Tennis Club. The region influenced by this feature from SDL-062 is sandy, soft-bottom marine habitat. SDL-157 produces another patch with 10^2 minimum dilution, but this feature moves offshore and intrudes into the southern end of ASBS 31, where again the marine habitat is a sandy soft-bottom type.

Maximum concentrations of storm water born total suspended solids (TSS) in the receiving water due to SDL-186 storm water discharges are at most 0.001 mg/L near the shore in the nearfield of SDL-186, and more typically 0.0001 mg/L elsewhere along the bluff-faced shoreline of the southern end of ASBS 29, where significant high-value, hard bottom marine habitat resides. The highest TSS concentrations in the receiving water around SDL-062 (at the end of Avendia De La Playa), were found to be 3 mg/L in a nearshore patch that that spreads south along the shore to the La Jolla Beach and Tennis Club. Most of the shoreline impacted by SDL-157 experiences maximum TSS concentrations from storm water in the range of 0.3 mg/L to 0.03 mg/L, all of which is sandy soft- bottom marine habitat. At the northern end of ASBS 29, and extending into ASBS 31, maximum TSS concentrations in the receiving from SDL-157 are in the range of 4 mg/L to 0.4 mg/L, the highest anywhere in La Jolla Bay. These relatively higher TSS values are probably a consequence of the steep land forms that comprise the watershed of SDL-157, with portions of both developed and undeveloped coastal bluffs. Regardless, the marine habitat subjected to these TSS loadings from SDL-157 is a sandy, soft-bottom type.

Because of the disparity in length scales between dilution in the offshore region versus the surfzone, a separate analysis was performed on a nested fine scale grid covering the surf zone for a long shore reach that extended 1000 ft (305 meters) either side of the beach outfalls. The size of this grid was based on precedent already set for the definition of a zone of initial dilution (ZID) by the Regional Water Quality Control Board, San Diego. Thirty two years of receiving water variables (involving 11,688 distinct combinations) were input for daily simulations of dilution. The ZID was searched for the minimum dilution after averaging across the width of the surf zone. Source loading for these simulations were based on the discharges measured during Wet-Weather Event-1 from outfalls SDL-157, SDL-062 and SDL-186. The surfzone dilution results are then compared to minimum dilution on the sea surface at an offshore control point in ASBS 29 where local water depth is -10 m MSL.

AMEC, Environment & Infrastructure, Inc. Hydrodynamic Modeling of Storm Drain Discharges in the Neighborhood of ASBS 29 in La Jolla, California Revised: 13 March 2013

This probability analysis procedure (based on the historic combination of 11,688 environmental variables) inevitably produces some combinations of wet weather discharges with dry weather ocean mixing conditions, and thereby results in certain worst case dilution scenarios that are more extreme, with even less dilution, than the wet weather worst case proxy derived from the monitoring program (Wet-Weather Event-1). For storm drain SDL-157, the median outcome for minimum dilution factor within the surfzone ZID is 32 to 1; however the potential range of minimum dilution goes as high as 88 to 1, and as low as 15 to 1. Low energy, dry weather ocean mixing produced the 15 to 1 minimum surfzone dilution outcome, which had a probability of occurrence of 0.13%. Altogether, 90% of the potential outcomes produce minimum dilutions in the surfzone ZID greater than 20 to 1.

Minimum surfzone dilutions in the ZID off SDL-062 are slightly less. This is due to the wave shadow found chronically in the refraction pattern of La Jolla Shores in the neighborhood of Avendia De La Playa. The minimum surfzone dilution of discharges from SDL-062 is found to be 12.6 to 1, again a low energy, dry weather ocean mixing result. The median outcome of minimum surfzone dilution for SDL-062 is 22 to 1; while the potential range of minimum dilution goes as high as 64 to 1, based on the 32-year historical sequence of receiving water variables. Ninety percent of the potential outcomes for SDL-062 produce minimum surfzone dilutions greater than 15 to 1.

For comparison, probability statistics for minimum dilution at an offshore control point in ASBS 29 (where local water depth is -10 m MSL) discovered a worst case minimum dilution of storm water of 13,200 to 1; while the median outcome is 52,000 to 1. The highest minimum dilutions of storm water at the offshore control point were found to be 4.2 x 10⁵. Consequently, storm water dilution is sufficiently high in the offshore regions of ASBS 29 that concentrations of storm water runoff constituents should be well below quantifiable detection limits.

Hydrodynamic Modeling of Storm Drain Discharges in the Neighborhood of ASBS 29 in La Jolla CA

By: Scott A. Jenkins, Ph. D. and Joseph Wasyl

1.0 INTRODUCTION

The La Jolla Shores Coastal Watershed consists of 1,639 acres, primarily residential and institutional (e.g., the University of California, San Diego [UCSD] campus) land uses. Subdrainages within the watershed boundary drain west into two *areas of special biological significance* (ASBS): the San Diego—Scripps ASBS (ASBS 31) and the La Jolla ASBS (ASBS 29). The majority of the runoff from the watershed is conveyed through a network of storm drains before it is discharged at several locations along the beach. Hydrodynamic model analysis of the dilution and dispersion of shoreline discharges into ASBS 31 was previously done by Jenkins and Wasyl (2007) for the University of California San Diego. The present hydrodynamic analysis applies those same analysis methods in evaluating shoreline discharges of storm water into nearshore waters surrounding ASBS 29.

The central and largest portion of the La Jolla watershed affecting ASBS 29 drains to a single storm drain outfall that discharges at Avenida de la Playa.

Monitoring within ASBS 29 and the drainage area discharging to the ASBS has been conducted over a number of years at multiple locations. The La Jolla Shores Watershed Urban Runoff Characterization and Watershed Characterization Study (Weston Solutions, Inc., Weston, 2007) was conducted under Proposition 50 - Funding for Public Water Systems (Prop 50). The Weston (2007) study assessed data collected between 2005 and 2007 at locations within the watershed and receiving water to identify constituents of interest (COI) for ASBS 29 for future studies. Monitoring based on the Special Protections Document has been conducted since the 2008-2009 wet weather monitoring season. Monitoring activities were conducted by Weston for the wet weather monitoring seasons from fall 2008 through spring 2011 (Weston, 2009, 2010, 2011). Monitoring by Weston included a variety of sample collection programs, including pre-storm, during-storm, and post-storm sample collections. Sample collection occurred at receiving water mixing zone, outfall discharge, and reference beach locations. A summary of previous studies was published by AMEC Earth and Infrastructure, Inc. (AMEC) in a draft conceptual ASBS 29 Characterization Summary and is provided in Appendix A of AMEC (2012). During the 2011-2012 wet weather monitoring season (October 1, 2011 through April 30, 2012), storm water samples were collected from outfall discharge and receiving water mixing zone monitoring locations during three events (AMEC, 2012). Sediment samples were collected from receiving water mixing zone monitoring locations after the third monitored event. These water quality data were used to initialize the source loading inputs to the hydrodynamic model used in the present storm water dilution and dispersion analysis.

Dilution analysis of three storm drain discharges (SDL-157, SDL-062 and SDL-186) were studied in numerical simulation for wet weather extreme and average case scenarios using the **SEDXPORT** hydrodynamic modeling system that was developed at Scripps Institution of Oceanography for the US Navy's *Coastal Water Clarity System* and *Littoral Remote Sensing Simulator*. This model has been peer reviewed multiple times and has been calibrated and validated in the Southern California Bight for 4 previous water quality and design projects (cf. Section 2).

The locations of the monitoring locations storm drain outfalls (SDL-157, SDL-062 and SDL-186) are shown in Figure 1 and are characterized by the following locations and descriptions:

Table 1.

Outfall Discharge Monitoring Locations and Descriptions

Site ID	Pipe Diameter (in)	Number of Events / Sample Type	Latitude	Longitude	Description
SDL-062-OD	72	3 / Flow-weighted Composite	32.85384	-117.25491	Upstream of outfall at intersection of Avenida de la Playa and Paseo del Ocaso.
SDL-157-OD	48	3 / Flow-weighted Composite	32.86278	-117.25429	Upstream of outfall on east side of El Paseo Grande.
SDL-186-OD	36	2 / Time-weighted composite of 2 - 4 grab samples	32.84813	-117.26507	Upstream of outfall on Torrey Pines Road
SDL-063-OD	36	Not monitored	32.85563	-117.25818	End of Vallecitos Road
SDL-063B-OD	36	Not monitored	32.85913	-117.25608	On seawall approximately 300 feet north of northern end of Kellogg Park parking lot

Storm Drain SDL-062 – This 72-inch storm drain discharges approximately 44 % of the La Jolla Shores Coastal Watershed runoff. This percentage was revised from 49 % stated in the Work Plan based on a recalculated drainage area for the modified flow model. The SDL-062 outfall is located at the end of Avenida de la Playa and discharges to La Jolla Shores Beach. Flow-weighted composite samples were collected approximately one-quarter mile upstream of the outfall at a point where more accurate flow data could be collected. The monitoring location was within a manhole at the intersection of Avenida de la Playa and Paseo del Ocaso (Figure 1).

Storm Drain SDL-157 – This 48-inch outfall discharges directly to La Jolla Shores Beach and drains approximately 12 % of the La Jolla Shores Watershed. Flow-weighted samples were collected approximately 180 feet upstream of the outfall at a point where more accurate flow data could be collected. The monitoring location was within a manhole on the eastern side of El Paseo Grande (Figure 1).

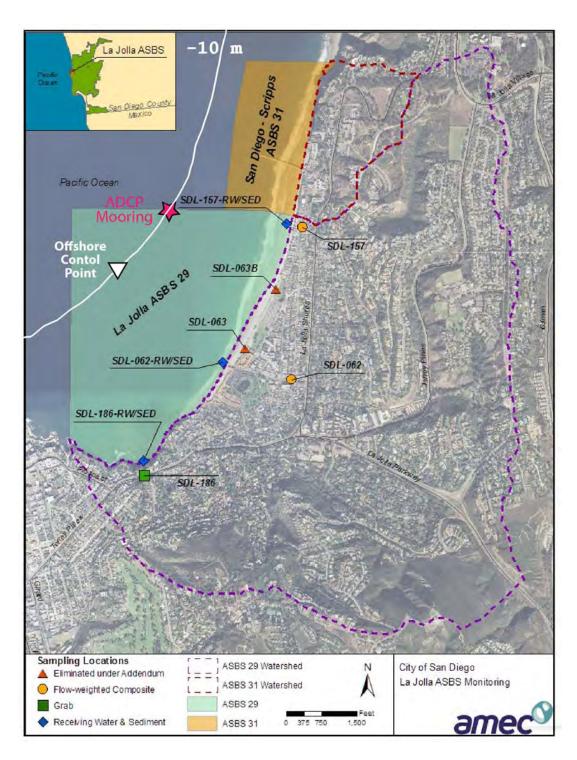


Figure 1. Location Map for La Jolla Storm Drains SDL-157, SDL-062 and SDL-186 in relation to ASBS 29 and ASBS 31, (from AMEC 2012).

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Storm Drain SDL-186 – This 36-inch outfall discharges directly to the rocky intertidal area of the ASBS south of La Jolla Shores Beach, known as Devil's Slide. This outfall drains approximately 3 % of the La Jolla Shores Coastal Watershed (Figure 1). This percentage was revised from 4 % stated in the Work Plan (MACTEC. 2011a. & b.)based on a recalculated drainage area for the modified flow model. Access to this outfall is restricted during high tide conditions. Given the lack of consistent, safe access to the outfall, the storm drain upstream was sampled instead. This manhole is located on Torrey Pines Road between Prospect Place and Amalfi Street, approximately 160 feet upstream of the outfall.

The quantification of dilution requires solving the hydrodynamic transport equations for the spatial and temporal variation of dilution factor throughout the effected receiving water. The dilution factor analysis will be evaluated at two distinct worse case scenarios: 1) base flows during dry weather with low mixing rates in the receiving waters due to quiescent ocean/atmosphere conditions; and 2) storm water runoff and discharge during high energy conditions typical of a winter storm event.

It is sensible to bifurcate the model problem into these dry and wet weather scenarios based on the multi-decadal dry/ wet cycles that the regional climate undergoes. The California coast is subject to climate cycles of about 20-30 years duration known as the Pacific/ North American pattern (for atmospheric pressure) or the Pacific Decadal Oscillation (for sea surface temperature). These dry/ wet cycles are apparent in the historic rainfall record of San Diego shown in Figure 2a. A dry period extended from about 1945-1977, followed by an episodically wet period from 1978-1998 that included the occurrence of 6 strong El Niño events (Inman and Jenkins 1999; and Goddard and Graham 1997). Based on the historic duration of these cycles, 1998 was likely the end of the wet cycle of climate in California with a return to the dry climate that prevailed from 1945-1977.

To illustrate the historical evidence for these dry and wet climate cycles, the rainfall record in Figure 2a was analyzed for climate trends using the Hurst (1951, 1957) procedure that was first used for determining decadal climate effects on the storage capacity of reservoirs (Inman and Jenkins, 1999). Climate trends become apparent when the data are expressed in terms of cumulative residuals of rainfall RF_n taken as the continued cumulative sum of departures of annual values of a time series RF_i from their long term mean value RF_a such that

$$RF_n = \sum_{i=0}^{n} (RF_i - RF_a)$$
 where *n* is the sequential value of the time series. When this procedure

was applied to the rainfall record in Figure 2b, dry periods are revealed by segments of the cumulative residuals having negative (downward) slopes while the wet periods have positive (upward) slopes. A dry period is found from 1945-1997, (negative slopes) while a wet period (positive slope) is shown from 1978-1998. The wet period of the climate cycle is more irregular caused by 6 strong El Niño events (water years 1978, 80, 83, 93, 95, and 98) and one 4 year period (1987-1990) of low rainfall. The analysis shows that the average annual rainfall increased by about 38% from the dry to the wet portions of the cycle. Furthermore, both the minimum and

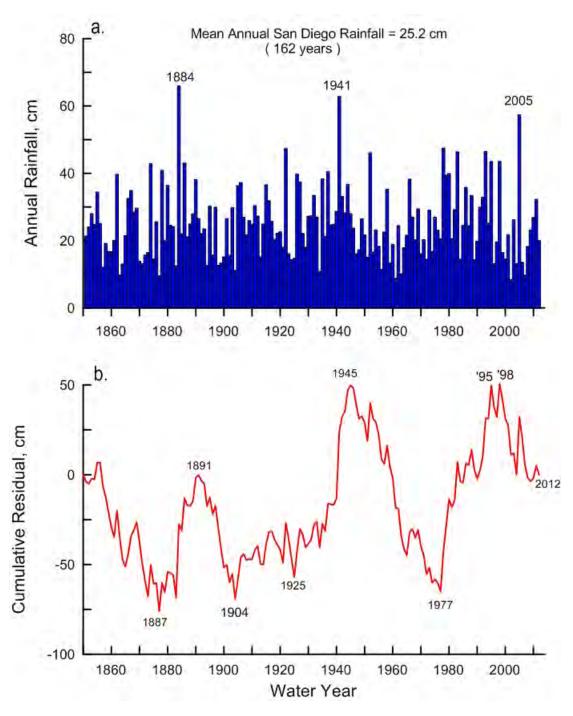


Figure 2. a) Period of record of San Diego rainfall and b) cumulative residual

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maximum ranges in rainfall are higher in the wet period, while the averages of the 6 major rainfall events in 21 year periods before and after the climate change (1977/78) are about 8 to 9 inches greater during the wet period. Since 1998, Figure 2b indicates that San Diego climate appears to have regressed into a developing multi-decadal dry period. These sharp distinctions in the statistics of dry vs wet climate periods lead to posing the model problem of discharge dilution in terms of dry and wet extreme case scenarios in order to bracket the envelope of potential variability.

Altogether there are seven primary variables that enter into a solution for resolving the dispersion and dilution of shoreline discharges such as those found along Scripps and La Jolla Shores Beach and the rocky shores of the Devil's Slide area. The statistics of these seven variables all change between dry and wet climate periods. These seven variables may be organized into *boundary conditions* and *forcing functions*. The boundary conditions include: ocean salinity, ocean temperature, ocean water levels and discharge flow rates. There is an additional boundary condition associated with offshore bathymetry beyond closure depth (typically greater than 12-15 m) that we will treat as constant. These boundary condition variables are developed in Section 3.1. The forcing function variables include: waves, currents, and winds and are developed in Section 3.2.

Overlapping 32 year long records containing 11,688 consecutive days between 1980 and 2012 are reconstructed in Sections 3.1 and 3.2 for each of the seven controlling variables. We search this 20.5 year period for the historical combination of these variables that give an historic extreme day in the sense of benign ocean conditions that minimize mixing and dilution rates, and a high energy day giving mixing and dilution rates typical of winter storm conditions with rainfall. We then overlay the peak discharge rates of seawater and storm water on those environmental conditions, respectively. The criteria for an extreme dry weather day was based on the simultaneous occurrence of the environmental variables having the highest combination of absolute salinity and temperature during the periods of lowest mixing and advection in the local ocean environment. These conditions coincide with the simultaneous occurrence of the lowest 5% wave, wind, currents, and ocean water levels averaged over a 24 hour period. The extreme wet weather scenarios were found by a statistical search of these records for the simultaneous occurrence of the highest 5% wave, wind, currents, and ocean water levels averaged over a 24 hour period. This procedure produced the model scenarios defined in Section 3.3. We also set up the model in Section 3.3 to solve for the minimum dilution in the surf zone for all 7523 combinations of the 7 controlling variables in order to establish the statistical properties the zone of initial dilution (ZID).

The technical approach used to evaluate these scenarios for historical extremes and average case conditions involved the use of the SEDXPORT hydrodynamic transport models. The pedigree and physics of this modeling system is described briefly in Section 2, with additional details provided in Appendices A &B. The dilution fields simulated by this model are presented in Section 4, giving results for the wer weather extreme in Section 4.1, and the long term minimum dilution in the ZID in Section 4.2. Dilution fields for an offshore control point in ASBS 29 are also given in Section 4.2.

2.0 MODEL DESCRIPTION AND CAPABILITIES

This study utilizes a coupled set of numerical tidal and wave transport models to evaluate dilution and dispersion of the discharges from the three storm drain outfalls discharging into ASBS 29 (Figure 1). The numerical model used to simulate tidal currents in the nearshore and shelf region offshore of La Jolla Shores is the finite element model TIDE_FEM. Wave-driven currents are computed from the shoaling wave field by a separate model, OCEANRDS. The dispersion and transport of concentrated seawater and storm water discharge by the wave and tidal currents is calculated by the finite element model known as SEDXPORT.

The finite element research model, **TIDE_FEM**, (Jenkins and Wasyl, 1990; Inman and Jenkins, 1996) was employed to evaluate the tidal currents within La Jolla Bay and in particular, the ventilation of the ASBS 29. **TIDE_FEM** was built from some well-studied and proven computational methods and numerical architecture that have done well in predicting shallow water tidal propagation in Massachusetts Bay (Connor and Wang, 1974) and along the coast of Rhode Island, (Wang, 1975), and have been reviewed in basic text books (Weiyan, 1992) and symposia on the subject, e.g., Gallagher (1981). The governing equations and a copy of the core portion of the **TIDE_FEM** FORTRAN code are found in Appendix A. **TIDE_FEM** employs a variant of the vertically integrated equations for shallow water tidal propagation after Connor and Wang (1975). These are based upon the Boussinesq approximations with Chezy friction and Manning's roughness. The finite element discretization is based upon the commonly used **Galerkin weighted residual method** to specify integral functionals that are minimized in each finite element domain using a variational scheme, see Gallagher (1981). Time integration is based upon the simple **trapezoidal rule** (Gallagher, 1981).

The computational architecture of **TIDE_FEM** is adapted from Wang (1975), whereby a transformation from a **global** coordinate system to a **natural** coordinate system based on the unit triangle is used to reduce the weighted residuals to a set of order-one ordinary differential equations with constant coefficients. These coefficients (**influence coefficients**) are posed in terms of a **shape function** derived from the natural coordinates of each nodal point in the computational grid. The resulting systems of equations are assembled and coded as banded matrices and subsequently solved by **Cholesky's method**, see Oden and Oliveira (1973) and Boas (1966). The hydrodynamic forcing used by **TIDE_FEM** is based upon inputs of the tidal constituents derived from Fourier decomposition of tide gage records. Tidal constituents are input into the module **TID_DAYS**, which resides in the hydrodynamic forcing function cluster (see Appendix B for a listing of **TID_DAYS** code). **TID_DAYS** computes the distribution of sea surface elevation variations in La Jolla Bay based on the tidal constituents derived from the Scripps Pier tide gage station (NOAA #941-0230). Forcing for **TIDE_FEM** is applied by the distribution in sea surface elevation across the deep water boundary of the computational domain.

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Wave driven currents were calculated from wave measurements by Scripps SAS station at Torrey Pines (Pawka, 1982) and by the CDIP arrays and/or buoys at Scripps Pier, La Jolla Bay, Huntington Beach, San Clemente, and Oceanside, CA, see CDIP (2005). These measurements were back refracted out to deep water to correct for island sheltering effects between the monitoring sites and Scripps Beach. The waves were then forward refracted onshore to give the variation in wave heights, wave lengths and directions throughout the nearshore around Scripps Beach. The numerical refraction-diffraction code used for both the back refraction from these wave monitoring sites out to deep water, and the forward refraction to the La Jolla Shores site is **OCEANRDS** and may be found in Appendix C. This code calculates the simultaneous refraction and diffraction patterns of the swell and wind wave components propagating over bathymetry replicated by the OCEANBAT code found in Appendix D. OCEANBAT generates the associated depth fields for the computational grid networks of both TID_FEM and OCEANRDS using packed bathymetry data files derived from the National Ocean Survey (NOS) depth soundings. The structured depth files written by **OCEANBAT** are then throughput to the module OCEANRDS, which performs a refraction-diffraction analysis from deep water wave statistics. **OCEANRDS** computes local wave heights, wave numbers, and directions for the swell component of a two-component, rectangular spectrum.

The wave data are throughput to a wave current algorithm in **SEDXPORT** (Appendix E) which calculates the wave-driven longshore currents, v(r). These currents were linearly superimposed on the tidal current. The wave-driven longshore velocity, v(r), is determined from the longshore current theories of Longuet-Higgins (1970). Once the tidal and wave driven currents are resolved by **TIDE_FEM** and **OCEANRDS**, the dilution and dispersion of storm water runoff and seawater discharge is computed by the stratified transport algorithms in **SEDXPORT**. The **SEDXPORT** code is a time stepped finite element model which solves the advection-diffusion equations over a fully configurable 3-dimensional grid. The vertical dimension is treated as a two-layer ocean, with a surface mixed layer and a bottom layer separated by a pycnocline interface. The code accepts any arbitrary density and velocity contrast between the mixed layer and bottom layer that satisfies the Richardson number stability criteria and composite Froude number condition of hydraulic state.

The combined discharge of seawater and storm water from the 3 La Jolla Shores beach outfalls is represented as sources in the surface mixed layer. The source initializations for these beach discharges are handled by a companion dilution code called **MULTINODE** (Appendix F) that couples the computational nodes of **TIDE_FEM** and **OCEANRDS** with **SEDXPORT**. The codes do not time split advection and diffusion calculations, and will compute additional advective field effects arising from spatial gradients in eddy diffusivity, (the so-called "gradient eddy diffusivity velocities" after Armi, 1979). Eddy mass diffusivities are calculated from momentum diffusivities by means of a series of Peclet number corrections based upon TSS and TDS mass and upon the mixing source. Peclet number corrections for the surface and bottom boundary layers are derived from the work of Stommel (1949) with modifications after Nielsen (1979), Jensen and Carlson (1976), and Jenkins and Wasyl (1990). Peclet number correction for the wind-induced mixed layer diffusivities are calculated from algorithms developed by Martin

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and Meiburg (1994), while Peclet number corrections to the interfacial shear at the pycnocline are derived from Lazara and Lasheras (1992a;1992b). The momentum diffusivities to which these Peclet number corrections are applied are due to Thorade (1914), Schmidt (1917), Durst (1924), and Newman (1952) for the wind-induced mixed layer turbulence and to Stommel (1949) and List, et al. (1990) for the current-induced turbulence.

In its most recent version, **SEDXPORT** has been integrated into the Navy's Coastal Water Clarity Model and the Littoral Remote Sensing Simulator (LRSS) (see Hammond, et al., 1995). The **SEDXPORT** code has been validated in mid-to-inner shelf waters (see Hammond, et al., 1995; Schoonmaker, et al., 1994). Validation of the **SEDXPORT** code was shown by three independent methods: 1) direct measurement of suspended particle transport and particle size distributions by means of a laser particle sizer; 2) measurements of water column optical properties; and, 3) comparison of computed stratified plume dispersion patterns with LANDSAT imagery.

Besides being validated in coastal waters of Southern California, the **SEDXPORT** modeling system has been extensively peer reviewed. Although some of the early peer review was confidential and occurred inside the Office of Naval Research and the Naval Research Laboratory, the following is a listing of 6 independent peer review episodes of **SEDXPORT** that were conducted by 8 independent experts and can be found in the public records of the State Water Resources Control Board, the California Coastal Commission and the City of Huntington Beach.

1997 – Reviewing Agency: State Water Resources Control Board

Project: NPDES 316 a/b Permit renewal, Scripps Beach, Carlsbad, CA

Reviewer: Dr. Andrew Lissner, SAIC, La Jolla, CA

1998 – Reviewing Agency: California Coastal Commission

Project: Coastal Development Permit, San Dieguito Lagoon Restoration

Reviewers: Prof. Ashish Mehta, University of Florida, Gainesville; Prof. Paul Komar, Oregon State University, Corvallis; Prof. Peter Goodwin, University of Idaho, Moscow

2000 – Reviewing Agency: California Coastal Commission

Project: Coastal Development Permit, Crystal Cove Development

Reviewers: Prof. Robert Wiegel, University of California, Berkeley; Dr. Ron Noble,

Noble Engineers, Irvine, CA

2002 – Reviewing Agency: California Coastal Commission

Project: Coastal Development Permit, Dana Point Headland Reserve

Reviewers: Prof. Robert Wiegel, University of California, Berkeley; Dr. Richard

Seymour, University of California, San Diego

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2003 – Reviewing Agency: City of Huntington Beach

Project: EIR Certification, Poseidon Desalination Project **Reviewer:** Prof. Stanley Grant, University of California, Irvine

2006 - Reviewing Agency: Regional Water Quality Control Board, San Diego Region

Project: UCSD Storm Drain Dilution Study and ASBS 31 Impacts **Reviewer:** Mr.John Robertus and Dr. Charles Chen, RWQCB

SEDXPORT has been built in a modular computational architecture with a set of subroutines divided into two major clusters: 1) those which prescribe hydrodynamic forcing functions; and, 2) those which prescribe the mass sources acted upon by the hydrodynamic forcing to produce dispersion and transport. The cluster of modules for hydrodynamic forcing ultimately prescribes the velocities and diffusivities induced by wind, waves, and tidal flow for each depth increment at each node in the grid network. The subroutines RIVXPORT and BOTXPORT in SEDXPORT solve for the mixing and advection of the seawater and buoyant storm water discharge in response to the wave and tidal flow using an rms vorticity-based time splitting scheme. Both BOTXPORT and RIVXPORT solve the eddy gradient form of the advection diffusion equation for the water column density field:

$$\frac{\partial \rho}{\partial t} = (\vec{u} \bullet \nabla \varepsilon) \bullet \nabla \rho - \varepsilon \nabla^2 \rho + \rho_0 Q_0 \tag{1}$$

where \vec{u} is the vector velocity from a linear combination of the wave and tidal currents, ε is the mass diffusivity, Λ is the vector gradient operator and ρ is the water mass density in the nearshore dilution field; and ρ_0 is the density of the water discharged by the outfall at a flow

rate $\frac{dV_0}{dt}$. The density of the discharge is a function of the bulk density of the suspended solids

 ρ_s and the density of the discharge fluid ρ_f that transports those solids, or:

$$\rho_0 = \rho_s + (1 - N) \rho_f = \rho_q N + (1 - N) \rho_f$$
(2)

where N is the volume concentration of suspended solids equal to the ratio of suspended solids to sample volume; and $\rho_q = 2.65 \text{ g/cm}^3$ is the density of the suspended solid particles taken to be fine-grained quartz.

Both the density of the receiving water ρ and the density of the discharge fluid ρ_f is a function of temperature, T, and salinity, S, according to the equation of state expressed in terms of the specific volume, $\alpha = 1/\rho$ and $\alpha_f = 1/\rho_f$ or:

$$\frac{d\alpha}{\alpha} = \frac{1}{\alpha} \frac{\partial \alpha}{\partial T} dT + \frac{1}{\alpha} \frac{\partial \alpha}{\partial S} dS
\frac{d\alpha_f}{\alpha_f} = \frac{1}{\alpha_f} \frac{\partial \alpha_f}{\partial T} dT + \frac{1}{\alpha_f} \frac{\partial \alpha_f}{\partial S} dS$$
(3)

The factor $1/\alpha \ \partial \alpha \ / \partial T$, which multiplies the differential temperature changes, is known as the coefficient of thermal expansion and is typically 2 x 10^{-4} per $^{\rm o}$ C for seawater; the factor $1/\alpha \ \partial \alpha \ / \partial S$ multiplying the differential salinity changes, is the coefficient of saline contraction and is typically 8 x 10^{-4} per part per thousand (ppt) where 1.0 ppt = 1.0 g/L of total dissolved solids (TDS). For a standard seawater, the specific volume has a value $\alpha = 1/\rho = 0.97264$ cm $^3/$ g. If the percent change in specific volume by equation (3) is less than zero, then the water mass is heavier than standard seawater, and lighter if the percent change is greater than zero.

The dilution ratio is given by the volume concentration of the discharged suspended solids in the receiving water and follows from the sediment continuity equation:

$$\frac{\partial N}{\partial t} = (\vec{u} \cdot \nabla \varepsilon) \cdot \nabla N - \varepsilon \nabla^2 N - W_0 \frac{dN}{dz}$$
(4)

where W_0 is the settling velocity of suspended particles. It is necessary to correct dilution ratio calculations for the loss of suspended particles due to deposition, so that loss is not included as a pseudo-dilution. The deposition flux is the net between settling and re-suspension and is found from a sub-set of solutions to (4) at the seabed as originally posed for steady flow by Krone (1962) and expanded to oscillatory flow by Jenkins and Wasyl (1990):

$$\Lambda = \frac{-K_s g N_c \left[W_0 N_c - \varepsilon \left(\frac{\partial N}{\partial z} \right)_{z=0} \right]}{\left(1 - N_c / N_s \right)}$$
(5)

where $K_s = 4 \times 10^{-14}$ sec is the sedimentation coefficient after Fujita (1962); g is the acceleration of gravity; N_c is the volume concentration at the top of the wave boundary layer; and N_s is the volume concentration of the seabed sediments. If N_0 is the volume concentration of suspended solids at the point of discharge (end-of-pipe); and N(x, y, z) is the volume concentration at any

location in the receiving waters, then the dilution factor at that location (corrected for deposition losses) is:

$$D(x, y, z) = \frac{N_0}{N(x, y, z)} - \frac{\Lambda}{W_0}$$
 (6)

Hence, net deposition (Λ >0) in the ASBS acts to increase the retention of discharged particulate, and consequently reduces the apparent dilution factor.

In (1) and (4) the term $\nabla \varepsilon$ acts much like an additional advective field in the direction of high to low eddy diffusivity. This additional "gradient eddy diffusivity velocity" is the result of local variations in current shear and wave boundary layer thickness. Both are bathymetrically controlled and the latter is associated with the refraction/diffraction pattern and is strongest in the wave shoaling region nearshore.

The settling velocity W_0 in (4) and (5) is particle size dependent. The **SEDXPORT** code is configured to accept up to nine particle size bins which for fine-grained particulate are assigned according to the particle size distribution after Jerlov (1976):

$$N(d) = \hat{N} \left(\frac{d}{\hat{d}}\right)^{\gamma} \tag{7}$$

where

$$N_0 = \sum_{d} N(d) \tag{8}$$

Here \hat{d} is the reference grain size, typically taken as 1.0 microns; and γ is the slope of the particle size distribution on log-log scale where $\gamma \cong 2.5$ for the global average, and \hat{N} is the volume concentration of the reference grain size that is adjusted such that (8) satisfies the measure value N_0 at end-of-pipe.

Solutions for the density and concentration fields calculated by the **SEDXPORT** codes from equations (1)-(4), (7) & (8) are through put to the dilution codes of **MULTINODE** to resolve dilution factors according to (5) & (6). These codes solve for the dilution factor (mixing ratio) for each cell in the finite element mesh of the nearshore computational domain based on a mass balance between imported exported and resident mass of that cell (see Appendix F). The diffusivity, ε , in (1) controls the strength of mixing and dilution of the seawater and storm water constituents in each cell and varies with position in the water column relative to the pycnocline interface. Vertical mixing includes two mixing mechanisms at depths above and below the pycnocline: 1) fossil turbulence from the bottom boundary layer, and 2) wind mixing in the surface mixed layer. The pycnocline depth is treated as a zone of hindered mixing and varies in

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response to the wind speed and duration. Below the pycnocline, only turbulence from the bottom wave/current boundary layer contributes to the local diffusivity. Nearshore, breaking wave activity also contributes to mixing. The surf zone (zone of initial dilution) is treated as a line source of turbulent kinetic energy by the subroutine **SURXPORT** (Appendix E). This subroutine calculates seaward mixing from fossil surf zone turbulence, and seaward advection from rip currents embedded in the line source. Both the eddy diffusivity of the line source and the strength and position of the embedded rip currents are computed from the shoaling wave parameters evaluated at the breakpoint, as throughput of **OCEANRDS**.

3.0 MODEL INITIALIZATION

Uninterrupted, long-term monitoring of ocean properties has been conducted at the nearby Scripps Pier. The Scripps Pier has been the site of both a NOAA tide gage station (NOAA #941-0230) as well as a monitoring station of the Coastal Data Information Program. It has also been the site where many new monitoring techniques have been developed and validated. We will take advantage of these long term observations to develop the data bases for initializing the boundary conditions and forcing functions used in the model. Statistical searches of these data bases will be performed to extract the dry and wet weather extreme case scenarios.

These dry and wet weather model scenarios are proxies for the extremes of the long term climate variability of the region, and serve to bracket the envelope of potential discharge effects on ASBS 29. Climate variability begins with seasonal variations in Earth's exposure to the sun, producing inter-annual variations in atmospheric pressure fields which in turn cause the Earth's inter-annual seasons. Upon occasion, the typical seasonal weather cycles are abruptly and severely modified on a global scale. These intense global modifications are signaled by anomalies in the pressure fields between the tropical eastern Pacific Ocean and Australia/Malaysia known as the *El Niño Southern Oscillation*, commonly referred to as *ENSO*. The intensity of the oscillation is often measured in terms of the *Southern Oscillation Index* (*SOI*), defined as the monthly mean sea level pressure anomaly in mb normalized by the standard deviation of the monthly mean pressures for the period 1951-1980 at Tahiti minus that at Darwin, Australia. (Because the SOI is a ratio of terms that all have units of atmospheric pressure, it is a non-dimensional number).

The Southern Oscillation is in turn, modulated over multi-decadal periods by the *Pacific* **Decadal Oscillation**, which results in alternating decades of strong and weak El Niño. The longterm variability of the Pacific Decadal Oscillation (PDO) is shown in Figure 3 and the cumulative residual of the Southern Oscillation Index, between 1882 and 1996, is plotted in Figure 4. Southern Oscillation effects give rise to enhancements and protractions of the interannual seasonal cycles, and their two extremes are referred to as El Niño (SOI negative) and La Niña (SOI positive). Inspection of Figures 3 and 4 reveals a number of large positive oscillations in the SOI between 1944 and 1978 corresponding to La Niña dominated climate; and a series of very large negative oscillations occurring between 1978 and 1998 which correspond with El Niño dominated climate. Along the southern California coast, a period of mild-stable weather occurred during the 30 years between the mid-1940's and mid-1970's when La Niña dominated pressure systems prevailed. The average SOI for this period was +0.1, with strong La Niña events in 1950, (SOI = ± 1.4); 1955/56, (± 1.2); 1970/71, (± 1.0); 1973/74, (± 1.0); and 1975/76 (+1.4). Winters were moderate with low rainfall (see Figure 2), and winds were predominantly from the west-northwest. The principal wave energy was from Aleutian lows having storm tracks which usually did not reach southern California. Summers were mild and dry with the largest summer swells coming from very distant southern hemisphere storms.

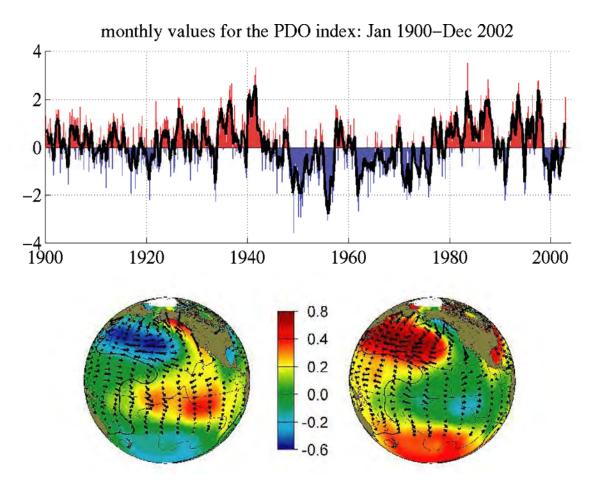


Figure 3. Typical wintertime Sea Surface Temperature (colors), Sea Level Pressure (contours) and surface wind stress (arrows) anomaly patterns during warm and cool phases of PDO. Red colors indicate warm, blue indicates cool.

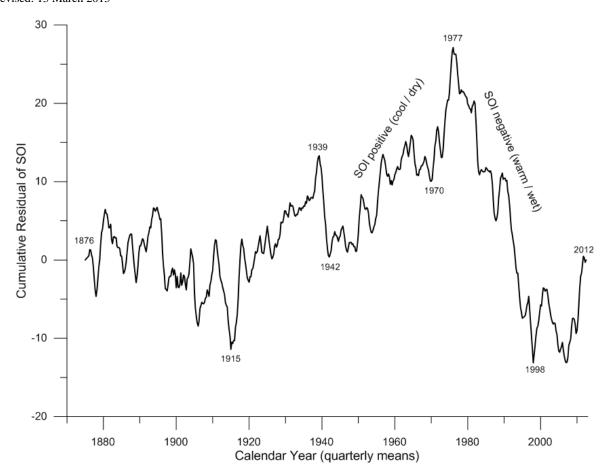


Figure 4. Cumulative residual of quarterly values of Southern Oscillation Index (SOI) [data from Australian Commonwealth Bureau of Meteorology].

Beginning in 1978, the southern California climate began transitioning into a warmer wetter period characterized by a succession of powerful El Niños, particularly those in, 1978, 1980, 1983, 1993, 1995 and 1998, [Inman & Jenkins, 1997]. The average SOI for this period was -0.5, with the 1978/79 El Niño averaging -1.2, the 1982/83 El Niño averaging a record -1.7 and the 1993/94 El Niño recording a mean of -1.0. Heavy rainfall accompanied each of these El Niño events (see Figure 2) causing flood run off to exceed many times the long term mean for all the major rivers and tributaries throughout Southern California (Inman and Jenkins, 1999). The wave climate in southern California also changed, beginning with the El Niño years of 1978/79 and extending until 1998. The prevailing northwesterly winter waves were replaced by high energy waves approaching from the west or southwest, and the previous southern hemisphere swell waves of summer have been replaced by shorter period tropical storm waves during late summer months from the more immediate waters off Central America. Other strong El Niño events of the past have also been accompanied by extreme wave events, although none of these have been as sustained as the succession of El Niños from 1978 to 1995. The 1939/42 El Niño had an average SOI of -1.3 and was associated with a series of destructive wave events in the Southern California Bight, the most intense being the 24/25 September 1939 storm which seriously

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damaged the breakwater system at Long Beach, CA. The El Niño of 1904/05 had a mean SOI of -1.4 and was attended by a series of damaging west swells in March 1904 and again in March 1905 [Horrer, 1950; Marine Advisors, 1961].

A similar succession of El Niño floods also preceded the cool/dry period of 1944-77, causing major episodes of sediment yield in 1927, 1937, 1938, 1941 and 1943.

From Figures 2 - 4 it is apparent that an inter-decadal pattern of rainfall and SOI has persisted for at least the last century an a half, characterized by alternating cool/dry La Niña dominated periods with little or no sediment yield, followed by warm/wet El Niño dominated periods when heavy rainfall produces most of the total sediment runoff. This kind of inter-decadal climate variability is observed throughout the west coast of the Americas and is now known as the *Pacific Interdecadal Oscillation (PDO)*, see Mantua et al (1997) and Zhang et al (1997). In this study we attempt to capture the potential range of PDO variability in the discharge problem by constructing the longest possible time series of the seven controlling model inputs from existing data bases, and then invoke statistical searches of those time series for the wet and dry extremes.

3.1 Boundary Conditions

A) Bathymetry: Bathymetry provides a controlling influence on all of the coastal processes at work in both the nearfield and farfield of La Jolla Shores. The bathymetry consists of two parts:

1) a stationary component in the offshore where depths are roughly invariant over time, and 2) a non-stationary component in the nearshore where depth variations do occur over time. The stationary bathymetry generally prevails at depths that exceed *closure depth* which is the depth at which net on/offshore transport vanishes. Closure depth is typically -15 m MSL in the Oceanside Littoral Cell, [Inman et al. 1993]. The stationary bathymetry was derived from the National Ocean Survey (NOS) digital database as plotted in Figure 5 seaward of the 15m depth contour. Gridding is by latitude and longitude with a 3 x 3 arc second grid cell resolution yielding a computational domain of 15.4 km x 18.5 km. Grid cell dimensions along the x-axis (longitude) are 77.2 meters and 92.6 meters along the y-axis (latitude). This small amount of grid distortion is converted internally to Cartesian coordinates, using a Mercator projection of the latitude-longitude grid centered on Scripps Pier. The convention for Cartesian coordinates uses x-grid spacings for longitude and y-grid spacings for latitude.

For the non-stationary bathymetry data inshore of closure depth (less than -15 m MSL) we use the equilibrium beach algorithms from Jenkins and Inman (2006). Depth contours generated from these algorithms vary with wave height, period and grain size and are plotted in Figure 5 landward of the 15m depth contour for the wave parameters of the wet weather extreme scenario (see Section 3.3).

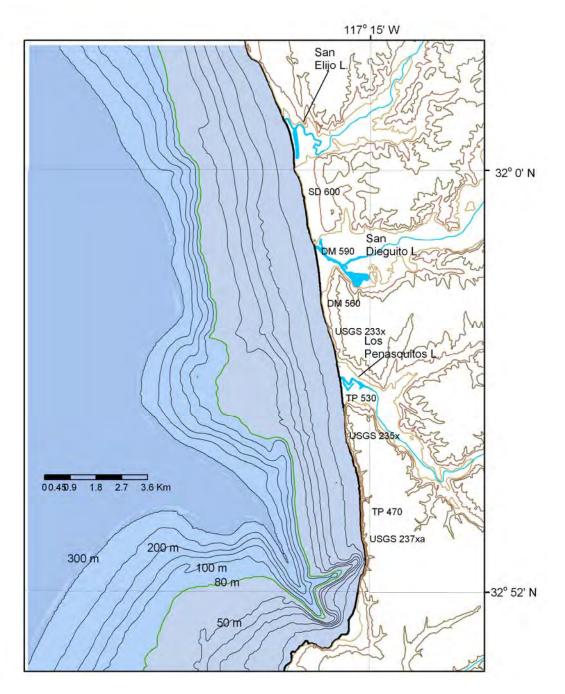


Figure 5. Composite bathymetry from NOS data base and equilibrium profiles after Jenkins and Inman (2006) for wave conditions of wet weather scenario.

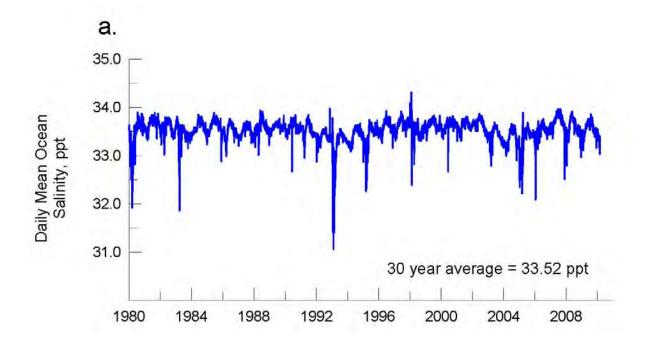
Depth contours shown in meters mean sea level.

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B) Ocean Salinity: Since the three La Jolla Shores outfalls (SDL-157, SDL-062 and SDL-186) discharge predominantly fresh water into a surfzone, the ocean salinity variation used in equation (3) to initialize SEDXPORT is of fundamental importance to the mixing of storm water in the receiving waters. Figure 6a shows the variation in daily mean sea surface salinity in the coastal waters off La Jolla Shores while Figure 7 shows daily mean seafloor salinity, both plotted from 32 years of monitoring data derived from the archival data bases of Scripps Institution of Oceanography (Scripps Pier Shore Station, SIO, 2012) and the Coastal Data Information Program (CDIP, 2012), supplemented by site monitoring data from MBC Applied Environmental Sciences (MBC), MBC (2012). The period of these unbroken archival sources extends from 1980 until March 2012. In the period of record from 1980 to 2012, the ocean salinity varies naturally by 10% between summer maximums and winter minimums, with a long term average value of 33.52 parts per thousand (ppt) on the surface and 33.49 ppt on the seafloor. Average sea surface salinity is slightly higher due to evaporation. Maximum salinity was 34.3 ppt on the sea surface and seafloor during the 1998 summer El Nino when southerly winds transported high salinity water from southern Baja up into the Southern California Bight. Minimum salinity was 31.06 ppt on the sea surface and seafloor 30.4 ppt on the seafloor during the 1992 floods. The variation between maximum and minimum salinity is about 3.2 ppt to 3.9 ppt, which is about 10 % of the depth-averaged value of 33.5 ppt. The ocean salinity exceeded the 33.5 ppt average value during 3,736 days during the period of record, and was below average during 2,259 days. Therefore above average salinities are more common than below average salinities. Average salinities were observed a total of 5,022 days of the period of record, or about 46 % of the time. (These data are also confirmed by long term salinity monitoring at Scripps Pier NOAA Station #941-0230, and by 55 CalCOFI cruises in the Southern California Bight between 1984 and 1997, see SIO, 2005; Roemmich, 1989, and Bograd, et al, 2001).

C) Ocean Temperature: The ocean temperature effects the buoyancy of the storm water discharge through the absolute temperature of the discharge. This buoyancy effect is calculated by the specific volume change of the discharge relative to the ambient ocean water. The buoyancy of the plume exerts a strong effect on the mixing and rate of assimilation of the sea salts and backwash constituents by the receiving waters.

We use the average of temperature records from the archival data bases of Scripps Institution of Oceanography (Scripps Pier Shore Station, SIO, 2012) and the Coastal Data Information Program (CDIP, 2012), supplemented by site monitoring data from MBC (2012). An 11,688 point record of daily mean sea surface temperatures are plotted in Figure 6b, while daily mean seafloor temperatures are plotted in Figure 7b. These temperature data were throughput to dilution model as detailed in Section 2. A pronounced seasonal variation in these temperatures is quite evident with the maximum recorded daily mean temperature reaching 25.4 °C on the sea surface and 24.4 °C on the seafloor during the summer of the 1993 El Niño; and the minimum falling to 9.9 °C on the sea surface and 11.0 °C on the seafloor during the winter of the 1999-2000 La Niña. The mean temperature was found to be 17.7 °C on the sea surface and 17.2 °C on the seafloor. On a percentage basis, the natural variability of the temperature of coastal waters of La Jolla Bay is significantly greater than that of salinity, where temperature variability is on the order of $\Delta T = 86\%$ vs salinity variability of $\Delta S = 10\%$.



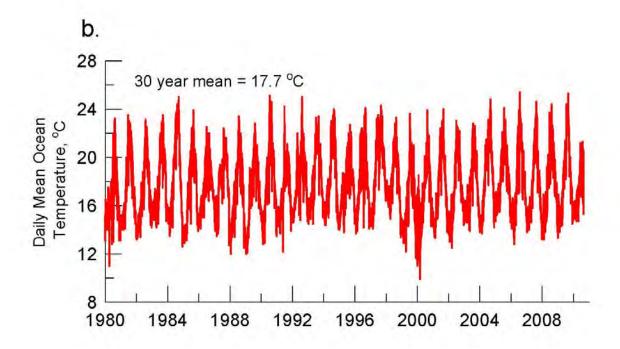
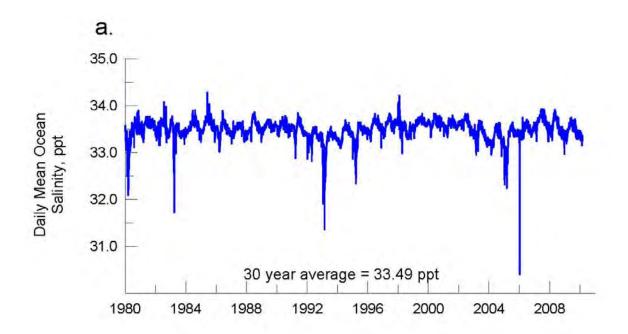


Figure 6. Daily mean ocean surface salinity (a) and daily mean ocean surface temperature at La Jolla, CA; from CDIP (2012), SIO, (2010), and MBC (2012).



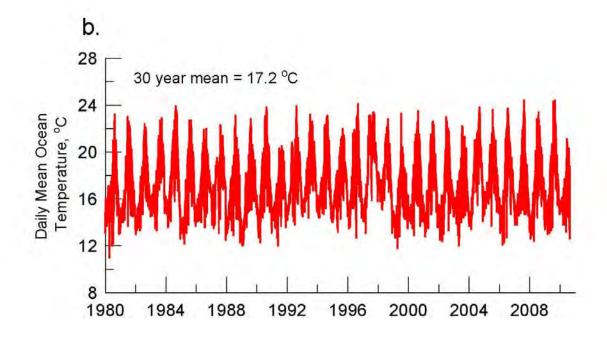


Figure 7. Daily mean ocean surface salinity (a) and daily mean ocean bottom temperature at La Jolla, CA; from CDIP (2012), SIO, (2010), and MBC (2012).

D) Ocean Water Levels: In the shallow nearshore environment off La Jolla Shores Beach, the dilution volume is limited by the time variation in ocean water level. Ocean water level varies in response to both tidal and climate oscillations such as ENSO. The ocean water level is monitored by the tide gage station located on Scripps Pier, La Jolla, CA. This tide gage (NOAA #941-0230) was last leveled using the 1983-2001 tidal epoch. Elevations of tidal datums referred to Mean Lower Low Water (MLLW), in **METERS** are as follows:

HIGHEST OBSERVED WATER LEVEL (11/13/1997) = 2.332 m

MEAN HIGHER HIGH WATER (MHHW) = 1.624 m

MEAN HIGH WATER (MHW) = 1.402 m

MEAN TIDE LEVEL (MTL) = 0.839 m

MEAN SEA LEVEL (MSL) = 0.833 m

MEAN LOW WATER (MLW) = 0.276 m

NORTH AMERICAN VERTICAL DATUM-1988 (NAVD) = 0.058 m

NGVD29 = 0.700 m

MEAN LOWER LOW WATER (MLLW) = 0.000 m

LOWEST OBSERVED WATER LEVEL (12/17/1933) = -0.874 m

Water levels measured by the Scripps Pier Tide Gage (#941-0230) have been archived by NOAA (2005) for the period of record, 1980 to 2012. Reconstruction of a water level time series was performed on the entire set of 1980-2012 NOAA measurements. The resulting time series of daily maximum and minimum ocean water levels is plotted in Panel-b of Figure 8. The positive sea level anomalies are a persistent and sustained occurrence in the observations of ocean water levels during this period of record and are another signature of El Niño. The warming of the coastal ocean during El Niño events causes thermal expansion of seawater by the second term in the equation of state, equation (3). A very significant number of diurnal tide cycles during the 32 period of record (466) have produced water level elevations well in excess of the extreme higherhigh water levels (EHHW) of the astronomic tides, (where EHHW = +4.28 ft. NGVD for a perigean spring tide occurring once every 4.5 years). In the period of record shown by the blue trace in Figure 8b, the maximum ocean water level was +5.35 ft. NGVD occurring during the 1997 El Niño, 1.31 ft. higher than the astronomic tides of the tide tables. These high water levels promote initial dilution of any beach discharge because they provide additional water depth and dilution volume where these discharges enter the sea. On the other hand, the minimum ocean water level was -4.66 ft. NGVD, occurring during the 1988 winter. These low water levels shown in the green trace of Figure 8b reduce initial dilution of beach discharge.

E) Discharge Flow Rates and Constituent Mass Loads: Wet weather monitoring was performed during three storm events during the 2011-2012 wet weather monitoring season, (AMEC, 2012). Table 2 presents the dates, sites monitored, and total rainfall for each of the three significant monitored events. Appendix G gives tabular listings of the AMEC monitoring data.

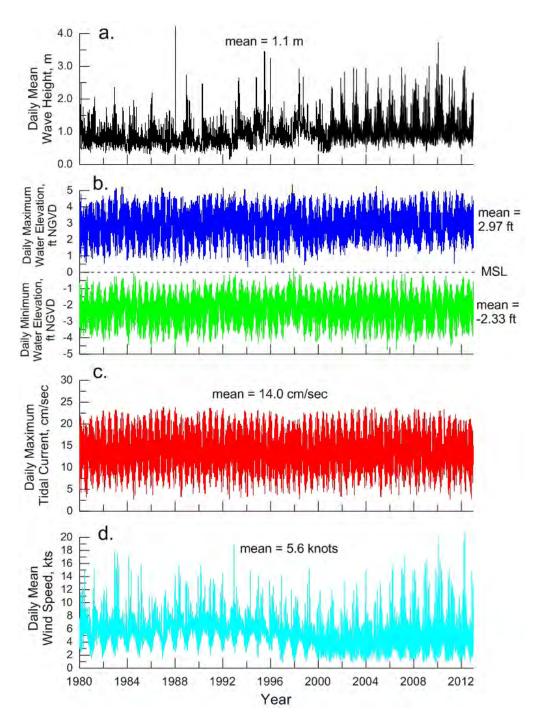


Figure 8. Controlling ocean forcing functions for La Jolla Bay: a) daily mean significant wave height, b) daily high and low water levels, c) daily maximum tidal current, d) daily mean wind speed. (data from CDIP, 2012; SIO, 2012; and NCEP, 2012).

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Table 2. Monitoring Events Summary

Event	Date	Total Rainfall (inches)	Sites Monitored
Event 1	11/20 to 21/2012	0.89	SDL-062-OD/RW
			SDL-157-OD/RW
Event 2	2/7/2012	0.23	SDL-062-OD/RW
			SDL-157-OD/RW SDL-186-OD/RW
Event 3	3/17 to 18/2012	0.54	SDL-062-OD/RW/SED
			SDL-157-OD/RW/SED SDL-186-
			OD/RW/SED

Notes:

OD - Outfall Discharge RW - Receiving Water SED - Sediment

Per the Special Protections Document Core Discharge Monitoring program, storm water outfalls 18 inches in diameter or greater must be monitored. Of the four outfalls within the La Jolla Shores Coastal Watershed that meet these criteria, the two largest outfalls (SDL-062 and SDL-157) were monitored during three events listed in Table 2. During these wet weather events, flow-weighted composites were collected, in addition to time-weighted composites composed of two to four grab samples that were collected during two storm events at a third outfall monitoring location (SDL-186). (The remaining two outfalls (SDL-063 and SDL-063B) were not sampled). The concentrations in mass per unit volume $\rho_{0i}(t)$ that were measured at time t for constituent i were applied to the measured flow rates $Q_j(t)$ of outfall j to obtain the flux $J_{ji}(t)$ of that constituent from outfall j, or $J_{ji}(t) = \rho_{0i}(t)Q_j(t)$. The cumulative mass loading $M_{ji}(\Delta t)$ of constituent i from outfall j over time period Δt is simply the time integral of the fluxes $M_{ji}(\Delta t) = \int_{\Delta t} J_{ji}(t) dt$. These data are used to define the variation the wet conditions that the La

Jolla Shores storm drain discharges exert on the receiving water and ASBS 29. These data are also used to identify the maximum discharge rates and concentration of discharge constituents, with primary emphasis here on total suspended solids (TSS) and the cumulative mass loading of total suspended solids, $M_{ji}(\Delta t)$. The dilution modeling of these point sources presented herein does not consider combined effects from non-point source runoff, such as from Peñasquitos Canyon.

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Wet Weather Event 1: Rainfall began at 5:01 pm on November 20, 2011 and ended at 12:21 am on November 21, 2011, totaling 0.89 inches. Flow began shortly after the onset of rainfall at both monitored locations, SDL-062 and SDL-157. Figure 9 presents the hydrographs for storm drains SDL-062 (black) and SDL- 157 (blue) as compared against rainfall (red) during Wet Weather Event-1. Peak flow rates were for storm drain SDL-157 were 3.5 cfs with a storm total flow volume of 53,425 cubic feet. Peak flow rates were on the order of 11 cubic feet per second (cfs) for storm drain SDL-062 while the storm total flow volume was 120,569 cubic feet. Figure 10 gives the flux of total suspended solids in a 15 minute interval (red) and cumulative mass loading of total suspended solids (black) for storm drain SDL-157 during Wet Weather Event-1. Peak TSS fluxes for SDL-157 were 28 kg in a 15 minute interval, or 1.9 kg/min; and the cumulative TSS mass loading from this storm drain for the entire event was 515 kg. Figure 11 gives the corresponding TSS fluxes and cumulative mass loading from storm drain SDL-062 for Wet Weather Event-1. Peak TSS fluxes for SDL-062 were significantly higher, on the order of 70 kg in a 15 minute interval, or 4.7 kg/min; and the cumulative TSS mass loading from SDL-062 gave a storm total of 888 kg. This is the highest TSS total load measured during any event or for any storm drain during the entire 2011-2012 monitoring period, as a consequence of Wet Weather Event-1 having the highest rainfall totals and representing a first-flush type of event.

Wet Weather Event 2: Rainfall began at 3:06 pm on February 7, 2012 and ended at 5:46 pm on February 7, 2012, totaling 0.23 inches. Flow began shortly after the onset of rainfall at the three monitored locations, SDL-062, SDL-157, and SDL-186. Figure 12 presents the hydrographs for storm drains SDL-062 (black) and SDL- 157 (blue) as compared against rainfall (red) during Wet Weather Event-2. Peak flow rates were for storm drain SDL-157 were 3.5 cfs with a storm total flow volume of 25,248 cubic feet. Peak flow rates were on the order of 7 cubic feet per second (cfs) for storm drain SDL-062 while the storm total flow volume was 27,052 cubic feet. Figure 13 gives the flux of total suspended solids in a 15 minute interval (red) and cumulative mass loading of total suspended solids (black) for storm drain SDL-157 during Wet Weather Event-2. Peak TSS fluxes for SDL-157 were 20 kg in a 15 minute interval, or 1.3 kg/min; and the cumulative TSS mass loading from this storm drain for the entire event was 164 kg. Figure 14 gives the corresponding TSS fluxes and cumulative mass loading from storm drain SDL-062 for Wet Weather Event-1. Peak TSS fluxes for SDL-062 were slightly greater, on the order of 22 kg in a 15 minute interval, or 1.4 kg/min; but the cumulative TSS mass loading from SDL-062 was less, about 100 kg, due to a rapid decline in TSS fluxes during the latter portion of the storm hydrograph for SDL-062 during Wet Weather Event-2.

Wet Weather Event 3: Rainfall began at 3:55 am on March 17, 2012 and ended at 7:31 pm on March 18, 2012, totaling 0.54 inches. Flow began shortly after the onset of rainfall at the three monitored locations, SDL-062, SDL-157, and SDL-186. Figure 15 presents the hydrographs for storm drains SDL-062 (black) and SDL- 157 (blue) as compared against rainfall (red) during Wet Weather Event-3. Peak flow rates were for storm drain SDL-157 were about 3.0 cfs with a storm total flow volume of 49,414 cubic feet. Peak flow rates were on the order of 7 cubic feet per second (cfs) for storm drain SDL-062 while the storm total flow volume was 89,485 cubic feet. Figure 16 gives the flux of total suspended solids in a 15 minute interval (red) and

cumulative mass loading of total suspended solids (black) for storm drain SDL-157 during Wet Weather Event-3. Peak TSS fluxes for SDL-157 were 13 kg in a 15 minute interval, or 0.9 kg/min; and the cumulative TSS mass loading from this storm drain for the entire event was 238 kg. Figure 17 gives the corresponding TSS fluxes and cumulative mass loading from storm drain SDL-062 for Wet Weather Event-3. Peak TSS fluxes for SDL-062 were slightly greater, on the order of 15 kg in a 15 minute interval, or 1.0 kg/min; but the cumulative TSS mass loading from SDL-062 was less, about 205 kg; again due to a rapid decline in TSS fluxes during the latter portion of the storm hydrograph for SDL-062 during Wet Weather Event-3.

Discharge from storm drain SDL-186 was not measured during the monitoring program, but was modeled in AMEC (2012). For the purposes of initializing SDL-186 in the SEDXPORT model in the present study, we use these modeled discharges. For Wet Weather Event-1, flow volume for SDL-186 was taken as 28,237 cubic feet. For Wet Weather Event-2, flow volume for SDL-186 was taken as 4,525 cubic feet. For Wet Weather Event-3, flow volume for SDL-186 was taken as 12,137 cubic feet.

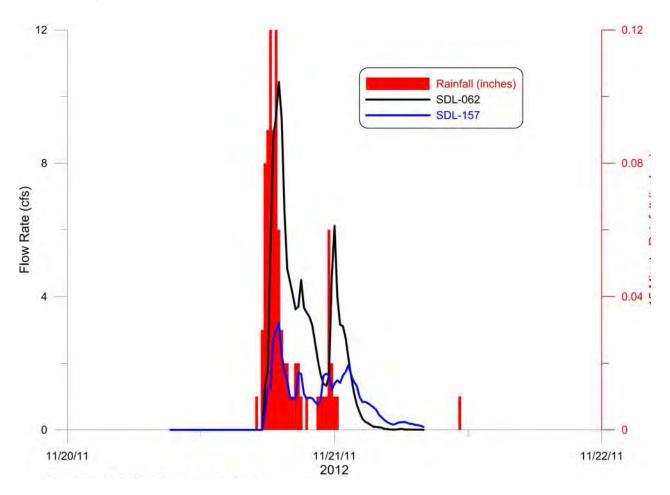


Figure 9. Hydrographs for storm drains SDL-062 (black) and SDL- 157 (blue) as compared against rainfall (red) during Wet Weather Event-1, 20 November 2011 to 21 November 2011.

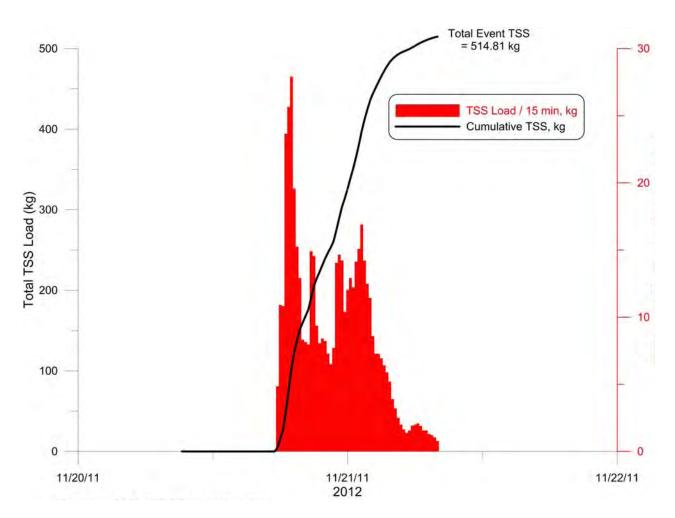


Figure 10. Flux of Total Suspended Solids in a 15 minute interval (red) and Cumulative Mass Loading of Total Suspended Solids (black) for storm drain SDL-157 during Wet Weather Event-1, 20 November 2011 to 21 November 2011.

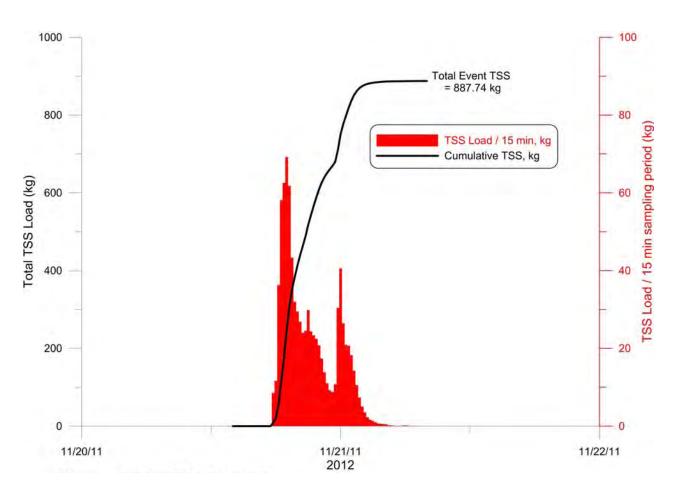


Figure 11. Flux of Total Suspended Solids in a 15 minute interval (red) and Cumulative Mass Loading of Total Suspended Solids (black) for storm drain SDL-062 during Wet Weather Event-1, 20 November 2011 to 21 November 2011.

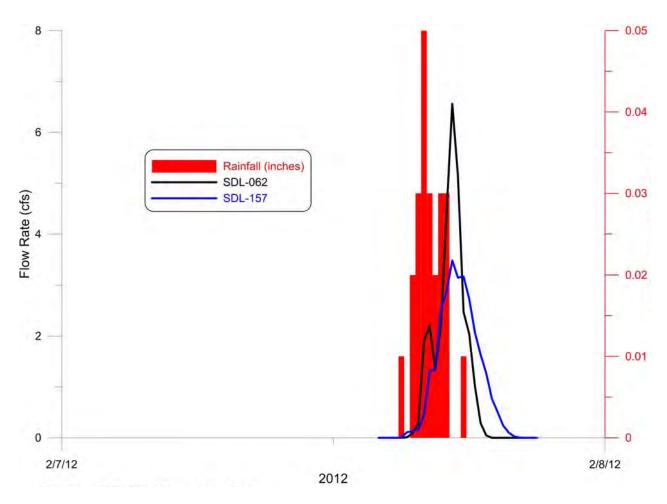


Figure 12. Hydrographs for storm drains SDL-062 (black) and SDL- 157 (blue) as compared against rainfall (red) during Wet Weather Event-2, 7 February 2012.

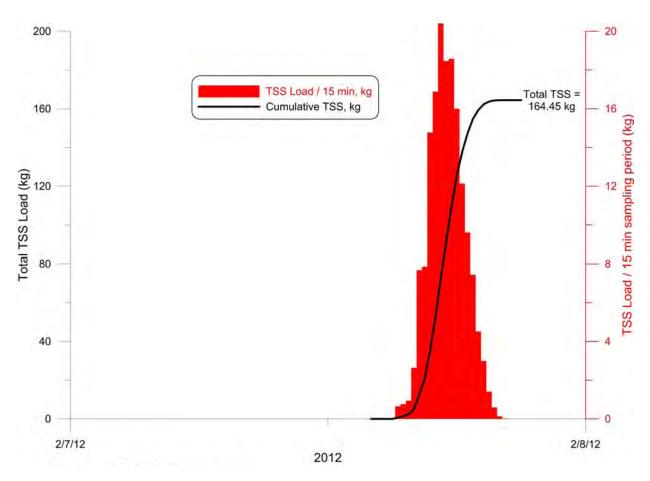


Figure 13. Flux of Total Suspended Solids in a 15 minute interval (red) and Cumulative Mass Loading of Total Suspended Solids (black) for storm drain SDL-157 during Wet Weather Event-2, 7 February 2012.

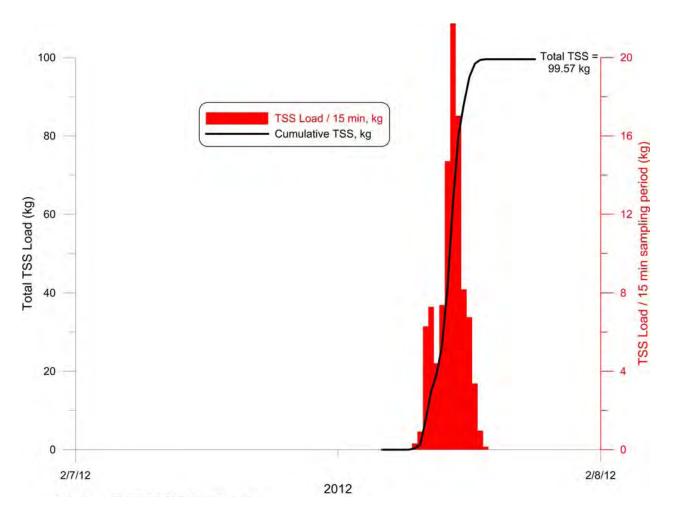


Figure 14. Flux of Total Suspended Solids in a 15 minute interval (red) and Cumulative Mass Loading of Total Suspended Solids (black) for storm drain SDL-062 during Wet Weather Event-2, 7 February 2012.

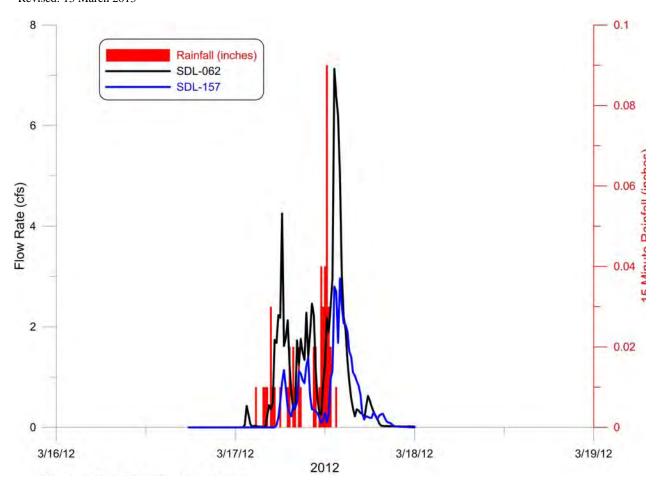


Figure 15. Hydrographs for storm drains SDL-062 (black) and SDL- 157 (blue) as compared against rainfall (red) during Wet Weather Event-3, 17 March 2012 to 18 March 2012.

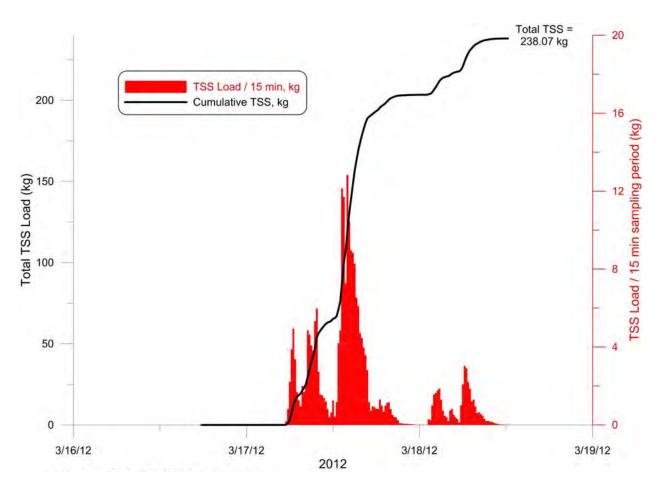


Figure 16. Flux of Total Suspended Solids in a 15 minute interval (red) and Cumulative Mass Loading of Total Suspended Solids (black) for storm drain SDL-157 during Wet Weather Event-3, 17 March 2012 to 18 March 2012.

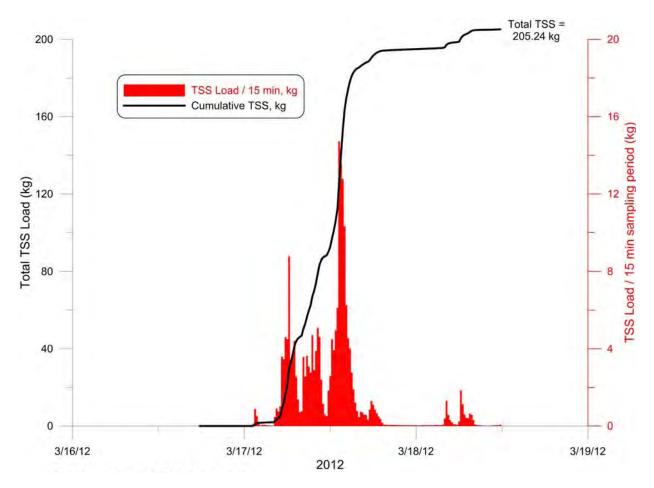


Figure 17. Flux of Total Suspended Solids in a 15 minute interval (red) and Cumulative Mass Loading of Total Suspended Solids (black) for storm drain SDL-062 during Wet Weather Event-3, 17 March 2012 to 18 March 2012.

3.2 Forcing Functions

A) Waves: Because the combined discharges of storm water from the La Jolla Shores storm drains are discharged into the surfzone, the wave climate exerts leading order control on the initial dilution and dispersion of TSS as well as the copper or TCDD constituents adsorbed on the surfaces of the fine grained sediments that make up the suspended solids loading. The wave forcing records are derived from measurements during the Coastal Data Information Program, (CDIP). This program routinely monitored waves at several locations in the lower Southern California Bight since 1980. The nearest CDIP *directional* wave monitoring sites for the Oceanside Littoral Cell and Torrey Pines Sub-Cell (Figure 18) are:

- a. Oceanside Array
- Station ID: 00401
- Location:

- 33 11.4⁰ North, 117 23.4⁰ West
- 500 feet SW of pier
- Water Depth (m): 10
- Instrument Description:
 - Underwater Directional Array
- Measured Parameters:
 - Wave Energy
 - Wave Direction
- b. San Clemente
- Station ID: 05201
- Location:
 - 33 25.2⁰ North, 117 37.8⁰ West
 - 1000 ft NW of San Clemente Pier
- Water Depth (m): 10
- Instrument Description:
 - Underwater Directional Array
- c. Measured Parameters:
 - Wave Energy
 - Wave Direction
- d. Huntington Beach Array
- Station ID: 07201
- Location:
 - 33 37.9⁰ North, 117 58.7⁰ West
 - Approximately 1 mile west of lifeguard headquarters at Huntington Beach, CA
- Water Depth (m): 10
- Instrument Description:
 - Underwater Directional Array
- Measured Parameters:
 - Wave Energy
 - Wave Direction

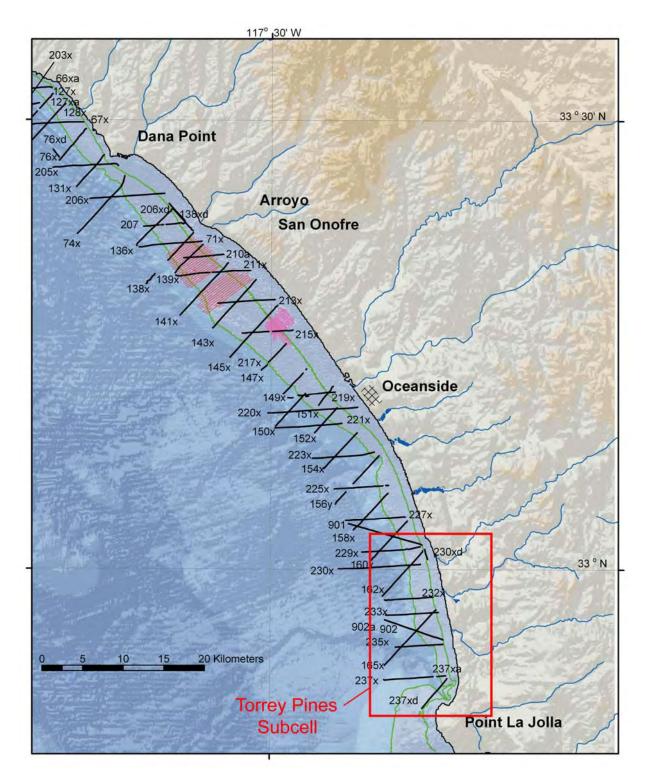


Figure 18. Oceanside Littoral Cell and Torrey Pines Sub-Cell.

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Data for these CDIP wave monitoring sites is available beginning January 1980 through March 2012 [CDIP, 2012]. In addition to the CDIP sites, waves have been monitored at Torrey Pines Beach from 1972 until 1984 by the SAS Stations deployed by Scripps Institution of Oceanography, (SIO), Pawka (1982). The ensemble of data from the CDIP and SAS monitoring stations were pieced together into a continuous record from 1980-2012 and entered into a structured preliminary data file.

The data in the preliminary file represent partially shoaled wave data specific to the local bathymetry around Oceanside, San Clemente or Huntington Beach. To correct these data to the Torrey Pine Sub-Cell in which the dilution modeling is performed, the data are entered into a refraction/diffraction numerical code, back-refracted out into deep water, and subsequently brought onshore into the immediate neighborhood of the Torrey Pines Sub Cell (as delineated in Figure 5 and 18). CDIP wave data are shoaled into Scripps Beach and the neighboring beaches of the Torrey Pines Sub-Cell using the OCEANRDS refraction-diffraction computer codes. The primitive equations for this code are lengthy, listings of the FORTRAN codes of OCEANRDS appear in Appendix C. These codes calculate the simultaneous refraction and diffraction patterns propagating over a Cartesian depth grid. "OCEANRDS" uses the parabolic equation method (PEM), Radder (1979), applied to the mild-slope equation, Berkhoff (1972). To account for very wide-angle refraction and diffraction relative to the principle wave direction, "OCEANRDS" also incorporates the high order PEM Pade approximate corrections modified from those developed by Kirby (1986a-c). Unlike the recently developed REF/DIF model due to Dalrymple et al (1984), the Pade approximates in "OCEANRDS" are written in tesseral harmonics, per Jenkins and Inman (1985); in some instances improving resolution of diffraction patterns associated with steep, highly variable bathymetry along the shelf break. These refinements allow calculation of the evolution and propagation of directional modes from a single incident wave direction; which is a distinct advantage over the more conventional directionally integrated ray methods that are prone to caustics (crossing rays) and other singularities in the solution domain where bathymetry varies rapidly over several wavelengths.

An example of a reconstruction of the back-refracted wave field throughout the Bight is shown in Figure 19 using the CDIP data from the San Clemente array. Wave heights are contoured in meters according to the color bar scale and represent 6 hour averages, not an instantaneous snapshot of the sea surface elevation. Note how the sheltering effects of Catalina and San Clemente Islands have induced longshore variations in wave height throughout the Southern California Bight. These variations (referred to as shadows and bright spots) induce longshore transport away from areas of high waves (bright spots, red) and toward areas of low waves (shadows, dark blue). Figure 20 shows the deep water significant wave heights, periods and directions resulting from the series of back-refraction calculations for the complete CDIP and SIO data set at $\Delta t = 6$ hour intervals over the 1980-2012 period of record. The data in Figure 20 are the values used as the deep water boundary conditions of the forward refraction computations into the Torrey Pines Sub Cell (Figure 18). The deep water wave angles are plotted with respect to the direction (relative to true north) from which the waves are propagating at the deep water boundary of Figure 18. Inspection of Figure 20 reveals that a number of large swells lined up with the wave windows open to the Torrey Pine Sub-Cell during the El Niño's of 1980-83, 1986-

88, 1992-95, and 1997-98. The largest of these swell events was the 18 January 1988 storm, producing 4.5 m deep water swells off Scripps Beach (see event #6 in Figure 20).

Figure 21 gives an example of the forward refraction calculation over the Torrey Pines Sub-Cell and La Jolla Bay region for the low energy waves that characterize the low mixing conditions of the dry weather modeling scenario. These particular waves were observed on 22 August 2011, and had a daily mean wave height of only 0.2 m, approaching La Jolla Shores from 210° with 10 sec period. In contrast, the refraction/diffraction pattern for a wet weather scenario is shown in Figure 22 for 1.8 m high storm swells shoaling onto La Jolla Shores from 283° with 14 sec period during 21 November 2011 (Wet-Weather Event-1). The longer period northwest swells of the stormy wet weather scenario produce a pronounced pattern of shadows (regions of locally smaller waves) and bright spots (regions of locally higher waves). Wave driven nearshore currents flow away from bright spots and converge on shadows. Inspection of Figure 22 reveals that the Avenedia de La Playa storm drain SDL-062 is in a shadow zone flanked by bright spots to the north near storm drain SDL-157. Consequently the zone of initial dilution for the SDL-062 storm drain is located in a region of converging longshore currents during the wet weather scenario. Such a convergence of drift results in rip currents and offshore flow, acting to disperse the beach discharges into deep water.

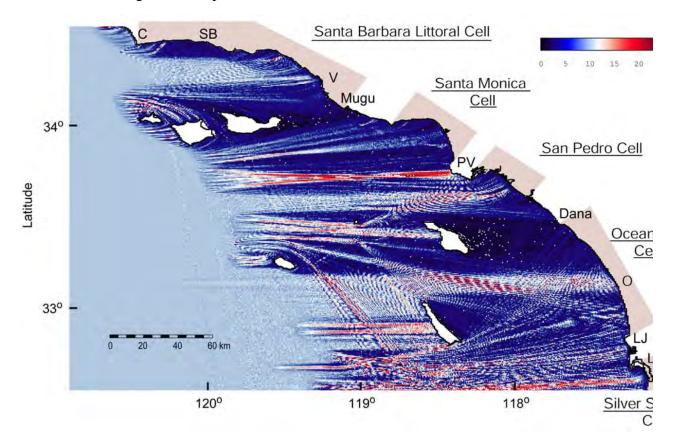


Figure 19. Back-refraction using *oceanrds.for* with waves measured by San Clemente CDIP station during the storm of 17 January 1988 with 10m high waves at 17 second period approaching the Southern California Bight from 270⁰

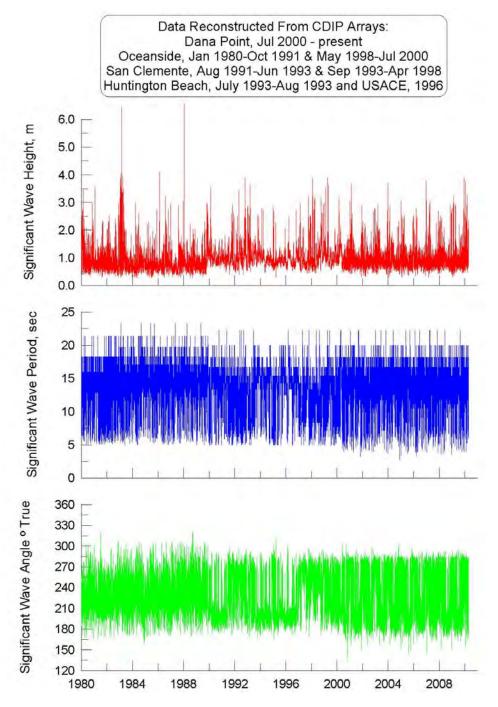


Figure 20. Deep water wave data for wave forcing in Torrey Pines Sub-Cell derived from back refraction of CDIP monitoring data, 1980-2012

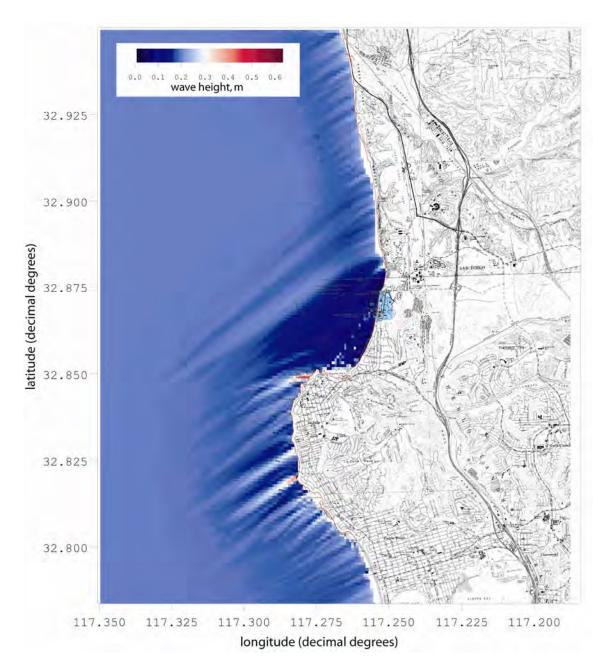


Figure 21. High resolution refraction/diffraction computation for extreme dry weather model scenario based on 0.2m deep water wave height from 210⁰ with 10 sec period.

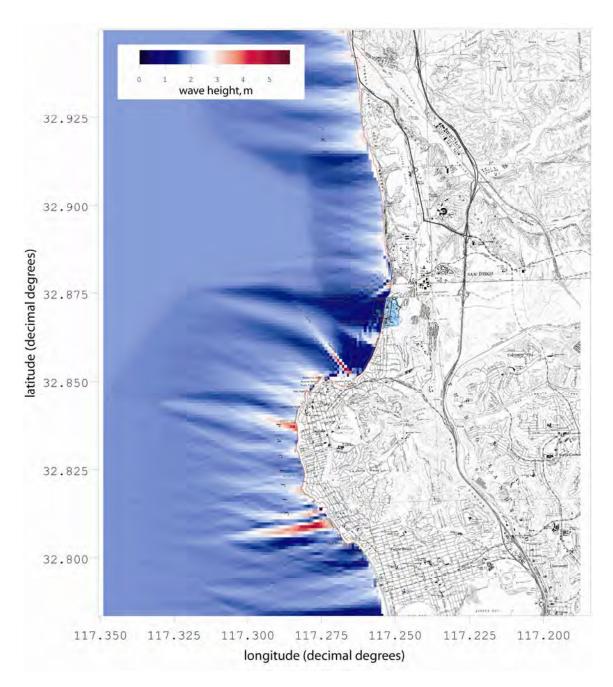


Figure 22. High resolution refraction/diffraction computation for worst-case wet weather model scenario based on 1.8 m deep water wave height from 283⁰ with 14 sec period, 21 November 2011.

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The directional dependence of the shoaling waves on La Jolla Shores that is shown in the refraction analysis of the examples in Figures 21 and 22 has been decomposed into a set of probability density functions in Figure 23, used to characterize the long-term wave mixing in the zone of initial dilution of storm drains SDL-062 and SDL-157. This figure reveals that the waves that influence dilution and dispersion of surf zone discharges of storm water at La Jolla Shores are *bi-modal* in character, meaning there are two primary directional modes: waves that approach the La Jolla Shores from the *northwest*, and waves that approach from the *southwest*. At lowest order, this bi-modality reflects seasonal wave climate cycles. The waves from the northwest are typically wet weather winter storm waves from Gulf of Alaska frontal-cyclones, and have typically higher significant wave heights 0(3m) and shorter wave periods 0(11 sec.). The southwest approaching waves are typically dry weather summer swells from distant Mexican tropical cyclones or southern hemisphere storms with lesser significant wave heights 0(1m - 2m) and longer wave periods 0(16 sec).

Southern Oscillation climate effects give rise to enhancements and protractions of the seasonal wave climate cycles, and their two extremes are referred to as El Niño (SOI negative) and La Niña (SOI positive). The wave climate in southern California changed, beginning with the El Niño years of 1978/79 and extending at least until the present. The average SOI for this period was -0.5, with the 1978/79 El Niño averaging -1.2, the 1982/83 El Niño averaging a record -1.7 and the 1993/94 El Niño recording a mean of -1.0. The prevailing northwesterly winter waves were replaced by high energy waves approaching from the west or southwest, and the previous southern hemisphere swell waves of summer have been replaced by shorter period tropical storm waves during late summer months from the more immediate waters off Central America. The net result appears to be a decrease in the southward component of the wave-induced longshore currents and the net littoral drift that had otherwise prevailed during the preceding thirty years (Jenkins and Wasyl, 2005, Inman and Jenkins, 1999). The wave statistics in Figure 23 seem to confirm this theory; whereby, despite the lesser intensity, there were 140,830 realizations of waves approaching from the southwest (El Niño dominant direction), as compared against only 61,508 realizations of waves approaching La Jolla Shores and the Torrey Pines Sub-cell from the northwest (La Niña dominant direction).

B) Currents: While waves dominate the initial dilution and dispersion of storm water and seawater discharges in the inshore domain, the tidal currents control dilution and dispersion in the offshore domain, particularly over most of the ASBS 29 footprint. A general southward net tidal drift is produced by the daily average of all the potential combinations of standing and progressive mixed tides. This net southward drift is an indication that the tidal transport in this region is *ebb dominated*. The strength of the net drift varies with the spring-neap cycle, with the strongest southward drift produced on the spring tides. In the La Jolla Bay portion of the Torrey Pines Sub-Cell, the southward drift is deflected by Pt La Jolla, producing a complex eddy structure in La Jolla Bay with a jet of converging flow immediately to the South of Pt La Jolla. The La Jolla Bay eddy is of particular interest because it often exerts sufficient entrainment near shore to cause ventilation of the ASBS by currents.

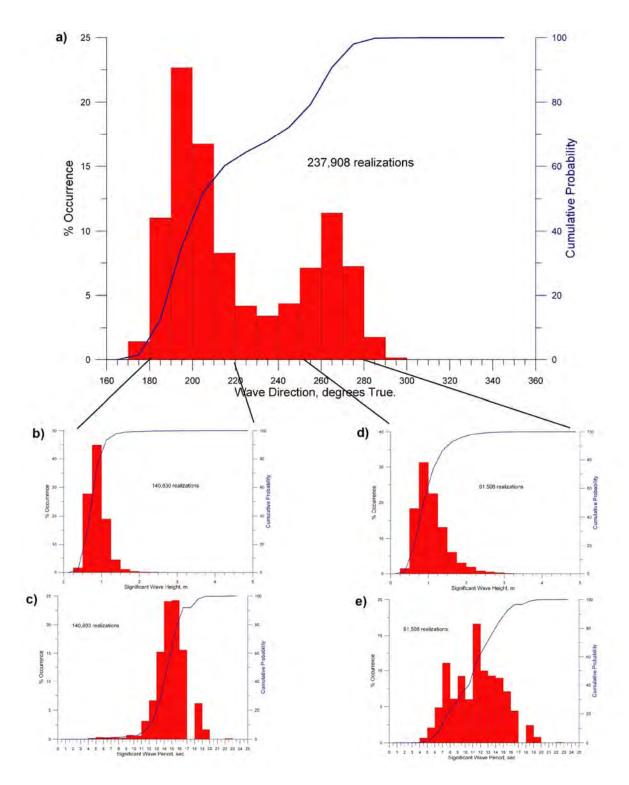


Figure 23. Statistics of composite wave record obtained by merging the CDIP archival data for 1980-2012 with the 2011-2012 ADCP wave burst measurements.

Figure 24 shows a progressive vector plot of the net tidal drift in La Jolla Bay during a neap tidal day, 22 August 2011. This current field is used to represent the minimal offshore mixing and advection conditions of the dry weather conditions. It can be seen from the vector field in Figure 24 that the La Jolla Bay eddy entrainment during neap tide does not extend close enough to shore to cause any appreciable ventilation of La Jolla Shores Beach or ASBS 29. The tidal drift ranges from nil to at most 5 cm/sec along the offshore boundary of the ASBS 29. The core of the La Jolla Bay eddy remains several kilometers offshore and its convergence with the broad field drift produces a 25 cm/sec jet flowing to the south near Marine Street in La Jolla. Given these features, the neap tidal drift used in the dry weather modeling scenario would appear to provide minimal dispersion of the beach discharges at La Jolla Shores Beach or the Devil's Slide area.

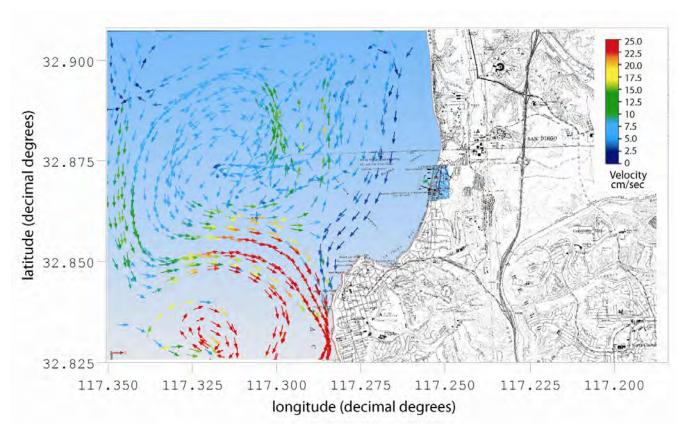


Figure 24. Progressive vector plot of current field in La Jolla Bay during a neap tidal day. Vector magnitude scaled by the color bar in the upper right corner.

By contrast, Figure 25 gives a progressive vector plot of the net tidal drift in La Jolla Bay during the spring tidal day concurrent with the wave conditions in Figure 22 during Wet Weather Event-1. This current field is used to represent the offshore mixing and advection conditions of the wet weather modeling scenario. In this case the La Jolla Bay eddy has moved closer to shore and has become paired with a system of counter rotating eddies off Pt La Jolla. The eddy entrainment currents off La Jolla Shores and through ASBS 29 are typically 9-15 cm/sec flowing toward the

south, following the shoreline contours along La Jolla Shores and subsequently feeding an eddy pair off Pt La Jolla. This eddy pair in turn discharges a jet flowing 45 cm/sec into deeper waters several kilometers west of Pt La Jolla. In total, the eddy system in the La Jolla Bay region during spring tides forms a very efficient conveyor for transporting near shore discharges at La Jolla Shores Beach and Devil's Slide into deep water off shore with significant intervening vorticity and eddy mixing to promote dilution.

Figures 26-29 confirms these model results with ADCP current measurements taken at the northern edge of ASBS 29 (Figure 1) where local water depth is -10 m MSL. Figure 26 gives near bottom currents from profile cell #1 (2.4 m above seabed) during the site monitoring period 11/14/11-11/24/12 for the east-west current velocity component (a); north-south velocity component (b); total velocity amplitude (c). Figure 27 decomposes the near-bottom total velocity amplitudes into probability densities (red bars) and cumulative probability (blue). Over this one-year current monitoring effort, we occasionally find rather large maximum near bottom currents on the order of 50 cm/sec (~1.0 kt) at the northern

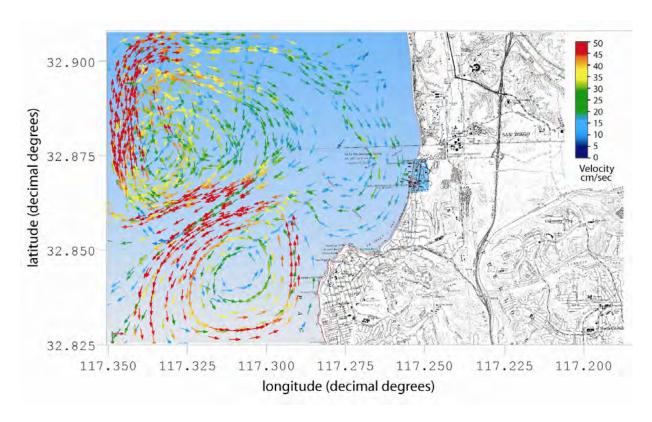


Figure 25. Progressive vector plot of current field in La Jolla Bay during a spring tidal day used in the wet weather modeling scenario. Vector magnitude scaled by the color bar in the upper right corner.

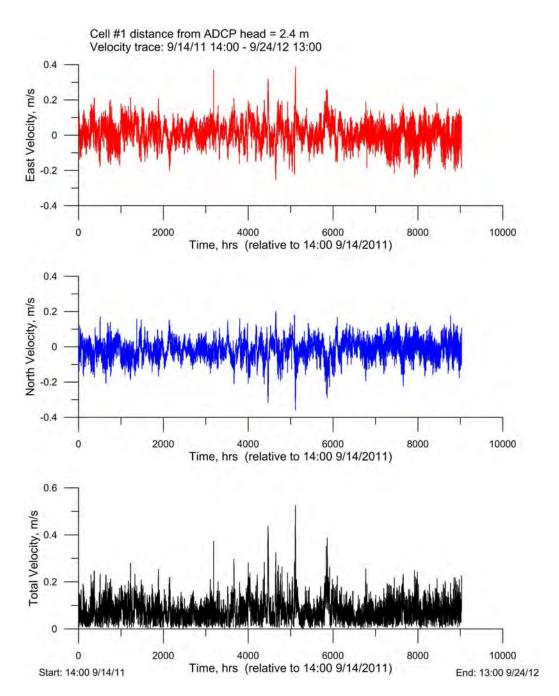


Figure 26. Near-bottom currents (2.4 m above seabed) at mooring location -10 m MSL at northern edge of ASBS 29 (cf. Figure 1). Measurements by Acoustic Doppler Current Profiler (ADCP), 11/14/11-11/24/12. East-west current velocity component (top); north-south velocity component (middle); total velocity amplitude (bottom).

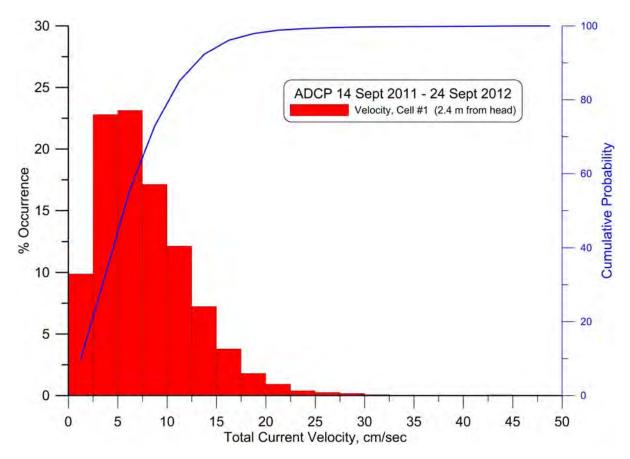


Figure 27. Histogram (probability density) and cumulative probability of nearbottom currents (2.4 m above seabed) at mooring location-10 m MSL at northern edge of ASBS 29, (cf. Figure 1); 11/14/11-11/24/12.

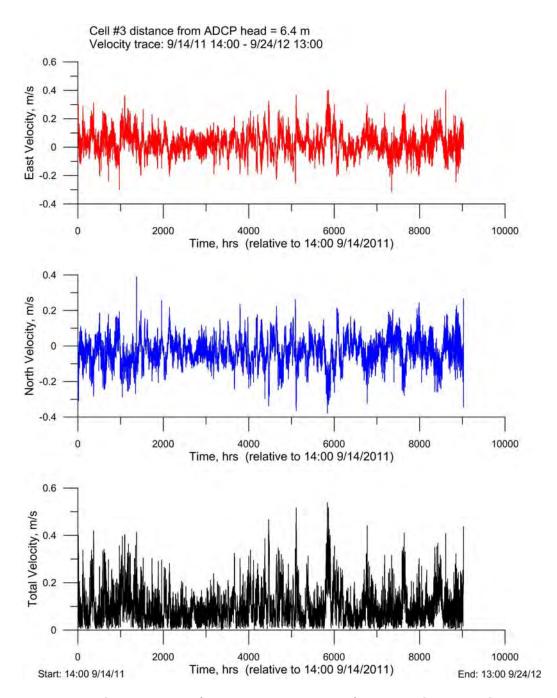


Figure 28. Interior currents (6.4 m above seabed) at mooring location -10 m MSL at northern edge of ASBS 29 (cf. Figure 1). Measurements by Acoustic Doppler Current Profiler (ADCP), 11/14/11-11/24/12. East-west current velocity component (top); north-south velocity component (middle); total velocity amplitude (bottom).

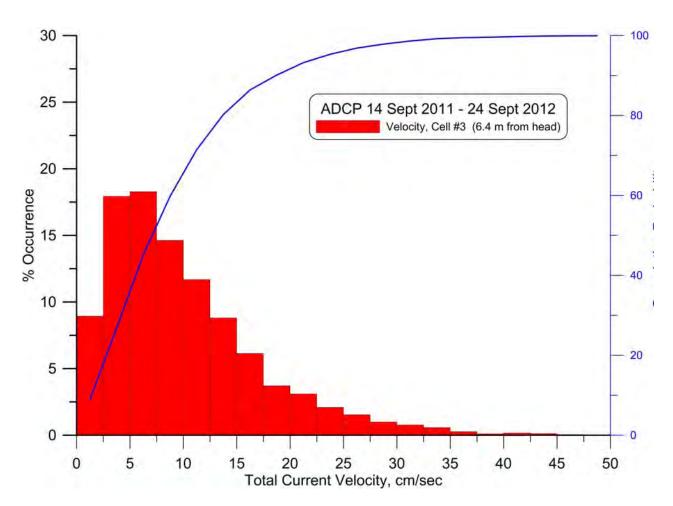


Figure 29. Histogram (probability density) and cumulative probability of interior currents (6.4 m above seabed) at mooring location-10 m MSL at northern edge of ASBS 29, (cf. Figure 1); 11/14/11-11/24/12.

edge of ASBS 29, while average near bottom currents are considerably less, on the order of only 5cm/sec. Further up into the water column, ADCP currents from profile cell #3 (6.4 m above seabed) in Figure 28 somewhat higher current amplitudes with more instances of maximum currents on the order of 50 cm/sec to 55 cm/sec during the site monitoring period 11/14/11-11/24/12. Accordingly, the probability densities (red bars) and cumulative probability (blue) of the interior currents in Figure 29 reveal higher average total current amplitudes, with average amplitudes of 8 cm/sec to 10 cm/sec, consistent with model results in Figures 24 & 25. These are favorable findings with respect to brine dilution rates of storm water in ASBS 29 where persistent currents are found at water column elevations close to the surface mixing layer of buoyant storm water.

C) Wind: Daily mean winds from the Scripps Pier Shore Station (SIO, 2001) and the Scripps Pier CDIP station (CDIP, 2002) were compiled over the 32 year period from January 1980 thru March 2012. This wind record is plotted in Panel-d of Figure 8. Because the lower Southern

California Bight is a "wind drought" region due to orographic blocking by the Peninsular Range, the 32 year mean wind speed is only 5.6 knots. However, El Niño storms and North Pacific cold fronts episodically increase daily wind speeds with the maximum sustained 24 hour mean wind speed reaching 19.6 knots during the 1997 El Niño storms. The minimum daily mean wind speed is 0 knots. In general the coastal winds in the nearshore of La Jolla Shores and Scripps Beach are benign, thereby limiting the degree of wind mixing of the surface water mass.

3.3 Model Scenarios

Based on discussions held in November 2004 with the San Diego Regional Water Quality Control Board, it was decided that the numerical modeling study of the beach discharges should be based on a wet weather worst-case scenario with a long-term probability assessment to bracket the envelope of the possible outcomes. The analysis of the multi-decadal rainfall variability of the San Diego found at the start if Section 3 provides justification for this approach.

A) Search Criteria for Wet Weather Worst Case Conditions: The 32 year long records of the boundary condition and variables in Figures 6-8 and the forcing function variables in Figure 20 were subjected to a joint probability analysis for the simultaneous recurrence of the combination of these variables for the wet weather worst case combination. Table 3 provides the search criteria that were applied to these records to establish the wet weather worst case combination of model inputs. The principle requirement of the wet weather worst-case is to maximize the combined discharges of storm water, while using ocean mixing variables that are characteristic, yet minimal for wet weather conditions. Because wet weather conditions occur during the passage of Pacific storms over the Southern California Bight, the worst case criteria of minimal ocean mixing tends to be mutually exclusive with the occurrence of storm water runoff. Wet weather worst case conditions for the receiving water involve storm minimums with respect to winds, waves and currents. To find such conditional minimums from among the 11,688 combinations found in Figures 6-8, and 20, the computer search criteria solved for the minimum of the largest 10% of the wind, waves and current combinations. Because the storm water discharge is predominately fresh water that is warmer than ocean water, the search criteria seeks salinity maximums and temperature minimums in the receiving water concurrent with the storm minimums of waves, winds and currents. In this way the maximum possible density contrast is achieved between the storm drain discharges and receiving water to retard the dilution rates of the discharge as much as possible under wet weather conditions.

Table 3.
Search Criteria for Wet Weather Worst-Case Combinations of Controlling Variables.

Variable	Search Criteria	Physical Significance
Discharge Rate	Maximize	Higher storm water and seawater discharge flow rates lead to higher source loading
Waves	Minimize Highest 10%	Smaller waves result in less mixing in surfzone and less inshore dilution
Currents	Minimize Highest 10%	Weaker currents result in less advection and less offshore dilution
Winds	Minimize Highest 10%	Weaker winds result in less surface mixing and less dilution in both the inshore and offshore
Ocean Water Level	Minimize	Lower water level results in less dilution volume in receiving water
Ocean Salinity	Maximize	Higher salinity leads to greater density contrast between discharge and receiving water
Ocean Temperature	Minimize	Higher temperature leads to density contrast between discharge and receiving water

B) Wet Weather Worst-Case Assignments: Among the three events monitored by AMEC (2012), Wet Weather Event-1 comes closest to matching the search conditions in Table-3 for the wet-weather worst-case proxy. The daily combination of receiving water variables that were recovered from the search criteria in Table 3 was represented best by the conditions on 21 November 2011. This day was post frontal with moderate, winds, waves, and currents. Ocean salinity was 33.11 ppt, depressed about 0.4 ppt by the run off from storms in the previous days while the ocean temperature was 14.5° C, about 4° C below the annual mean. Wave heights were 1.8 m, approaching La Jolla Bay from the northwest at 283⁰ with a 14 sec period. The refraction/diffraction pattern of these storm swells are shown in Figure 22. Winds were postfrontal northwest winds at 10-12 knots from at 290°. The maximum tidal currents in ASBS 29 were 9-15 cm/sec flowing toward the south, (Figure 25), with a weak inshore counter current immediately outside the surf zone following the shoreline contours northward along La Jolla Shores. The tidal current was due to a moderate ebbing spring tide with a minimum water level of -3.63 ft NGVD. With this combination of receiving water variables the model overlaid the storm drain discharges from Wet Weather Event-1 from the monitoring program. Discharges for storm drain outfalls SDL-062 and SDL-157 were initialized according to Figures 9-11. For the purposes of initializing SDL-186 for the worst-case wet weather proxy, we use these modeled discharges from AMEC (2012). For Wet Weather Event-1, flow volume for SDL-186 was taken as 28,237 cubic feet. TSS fluxes for SDL-186 were taken as 70 kg in a 15 minute interval, or 4.7 kg/min, or equivalent to peak TSS fluxes from the neighboring SDL-062 storm drain.

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C) Long-Term Simulations of Zone of Initial Dilution: The historic boundary conditions from Figures 6-8 and the forcing functions from Figure 20 were sequentially input into the model, producing daily solutions for the dilution field due to the peak flow event during the monitoring period (Wet Weather Event-1). This procedure inevitably produces some combinations of wet weather discharges with dry weather ocean mixing conditions, and thereby results in certain worst case dilution scenarios that are more extreme, with even less dilution, than the wet weather worst case proxy derived from the monitoring program (Wet-Weather Event-1). The input stream of seven controlling variables from Figures 6-8 & 20 produced 11,688 daily solutions for the dilution field in the zone of initial dilution (ZID) taken as the surfzone. A numerical scan of each of these daily solutions searched for the minimum dilution averaged between the wave break point and the shoreline along the inshore boundary of ASBS 29. Based on the ZID definition contained in the NPDES permit issued by the RWQCB, San Diego Region, for beach discharges at Encina Generating Station, Carlsbad, CA, the search for the minimum dilution was taken longshore to a distance of 1000 ft (305 meters) away from the discharge points in all directions, (although the minimum dilution was found without exception to be within 150m of the discharge points). The solution scans searched for minimum dilution in both the water column and along the sea floor. For each search, the minimum dilution found in any direction away from the outfall was entered into a histogram bin for ultimately assembling a probability density function and cumulative probability from the 11,688 outcomes. These results provided a statistical basis for assessing long term variability of dilution in the ZID. For comparison, a similar procedure of searching the solution space for minimum dilution was repeated at an offshore control point near the interior of ASBS 29, where local water depth was -10m MSL, see Figure-1.

3.4 Calibration

The coupled sets of wave, current and dilution/dispersion models were calibrated for end-to-end simulations of known dye dispersion events measured off Scripps Beach by Inman et al. (1971). Initializations for the model were derived from the measured forcing functions reported in that publication. Free parameters in the subroutines of the **SEDXPORT** for dilution/dispersion model were adjusted iteratively until a best fit was achieved between the measured and simulated dye concentrations and dilution factors.

The subroutines of **SEDXPORT**.for contain seven free parameters which are selected by a calibration data set specific to the coastal type for which the hindcast calibration simulation is run. These parameters are as follows according to subroutine:

BOTXPORTAf

*ak2 - stretching factor for vertical eddy diffusivit

*ak - adjusts mixing lengths for outfalls

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NULLPOINTAf

*ak7 - adjusts the asymmetry of the bedform distribution curve

SURXPORTAf

*aks - adjusts the surf zone suspended load efficiency, K_s

ak4 - stretching factor for the horizontal eddy diffusivity_x

RIVXPORTAf

*ak3_1 - adjusts the jetty mixing length and outfall mixing lengths

*ak3 - stretching factor for the horizontal eddy diffusivity of the river plume_H

The set of calibration values for these parameters was used without variation or modification for all model scenarios.

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RESULTS

4.0

The model scenarios defined in Section 3.3 are run in simulations of one tidal day using the numerical codes described in Section 2 and listed in Appendices A-F. The dilution fields are then depth averaged, and averaged over the simulation period. In the sections that follow, dilution fields are contoured in base -10 log according to the color bar scale in the lower right-hand corner of each plot, with a scale range that covers dilution factors between 10⁰ and 10⁷. Two separate perspectives of the worst-case results are given: 1) a composite dilution field when storm drains SDL-062, SDL-157 and SDL-186 are discharging simultaneously (Figure 30); and 2) the dilution field associated with each individual storm drain discharging in isolation under worst-case conditions (Figures 31-33). These dilution results are then applied to the peak TSS discharge fluxes to give TSS concentrations in the receiving water in mg/L in Figures 34-36.

4.1 Wet Weather Worst Case Dilution Results

Figure 30 shows the broad-scale view of the footprint of the dilution field for the wet weather worst case scenario (Wet-Weather Event-1) involving the simultaneous discharge from storm drains SDL-157, SDL-062 and SDL-186. In the broad-scale view, the dilution field of storm water spreads about a kilometer seaward of the shoreline due to vigorous cross shore mixing and advection from the storm winds and shoaling swells of the wet weather receiving water scenario. The dilution field of storm water from all three storm drains spreads about 2 kilometers along the shoreline, covering most of ASBS 29 and intruding into ASBS 31 under the advective influence of the longshore transport of rip current cells near the shoreline, while the tidal drift dominates the longshore spreading of the dilution field offshore, (cf. Figures 22 and 25). The dilution factors for the combined discharges from SDL-157, SDL-062 and SDL-186 in both ASBS 29 and ASBS 31 range between a minimum of 10² near shore to 10⁷ along the seaward boundaries during the wet weather worst case. Dilution factors of 10⁴ to 10⁷ characterize the outer one-half of ASBS 29, while dilution factors 10² to 10⁴ characterize the inner one-half. In the immediate neighborhood of SDL-157 and SDL-062, very near the shoreline of ASBS 29, dilution factors are as low as 40 to one.

Figures 31-33 allow us to examine the individual contributions to the combined storm water plume from each storm drain during the wet weather worst case from the monitoring program. It is clear from Figures 31 and 32 that the preponderance of the discharge plume is from SDL-157 and SDL-062, while SDL-186 at Devil's Slide (Figure 33) makes a very minor contribution. This is a fortuitous outcome in the sense that a significant amount of high-value, hard bottom marine habitat lives in the southern end of ASBS 29, and in the immediate neighborhood of SDL-186. Dilution factors of storm water discharged from SDL-186 are at most 10³ near the shore in the nearfield of SDL-186 (Figure 33) and more typically 10⁴ to 10⁵ elsewhere along the bluff-faced shoreline of the southern end of ASBS 29. The largest single contributing footprint to the combined storm water plume appears to be from SDL-062 at the end of Avendia De La Playa (Figure 32) that produces a large patch with 10² minimum dilution very near the shoreline that spreads south along the shore to the La Jolla Beach and Tennis Club. The region influenced by

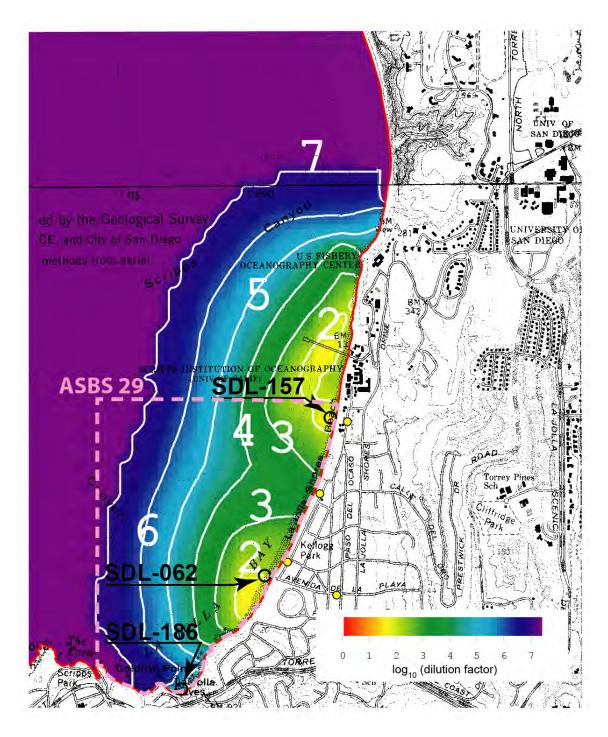


Figure 30. Depth averaged dilution for wet weather worst case due to simultaneous discharges of storm water from SDL-157, SDL-062 and SDL-186; 21 November 2011; waves: H = 1.8 m, T = 14 s, $\alpha = 283^{\circ}$; wind = 11 kts.

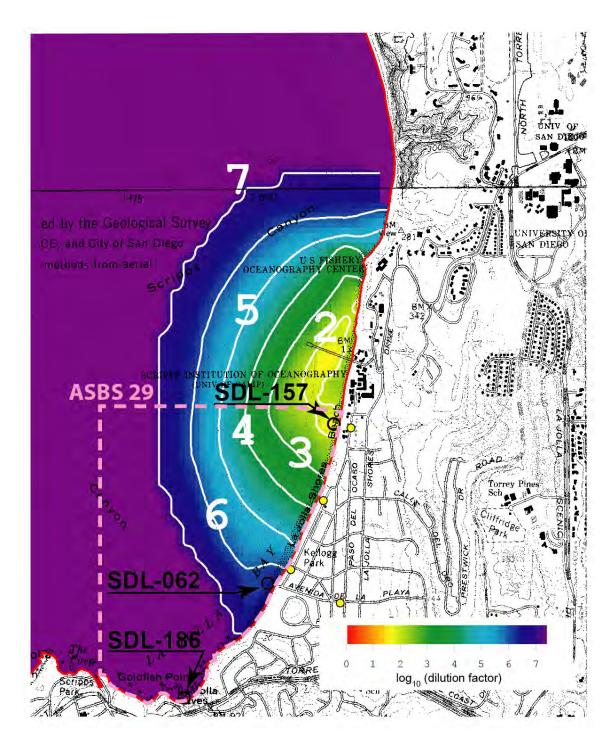


Figure 31. Depth averaged dilution for wet weather worst case due to isolated discharge of storm water from SDL-157; 12 November 2011; waves: H = 1.8 m, T = 14 s, $\alpha = 283^{\circ}$; wind = 11 kts.

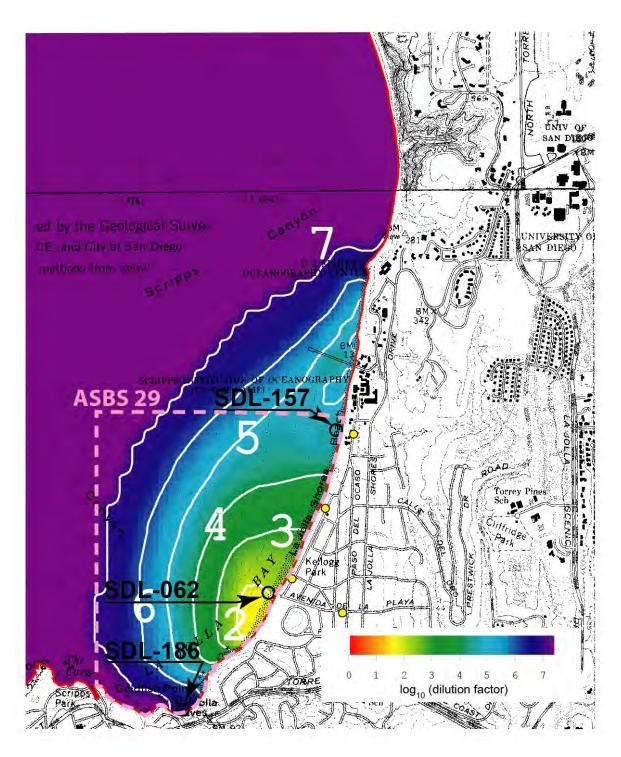


Figure 32. Depth averaged dilution for wet weather worst case due to isolated discharge of storm water from SDL-062; 21 November 2011; waves: H = 1.8 m, T = 14 s, $\alpha = 283^{\circ}$; wind = 11 kts.

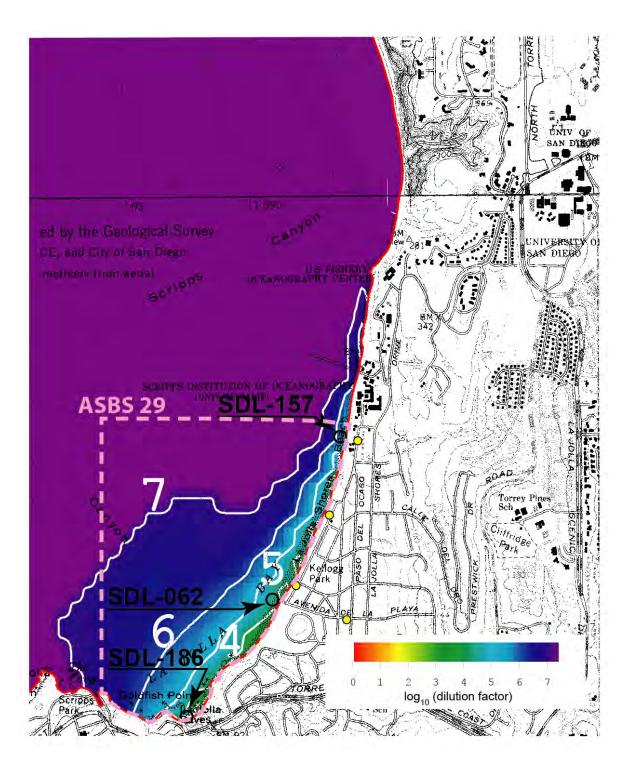


Figure 33. Depth averaged dilution for wet weather worst case due to isolated discharge of storm water from SDL-186; 21 November 2011; waves: H = 1.8 m, T = 14 s, $\alpha = 283^{\circ}$; wind = 11 kts.

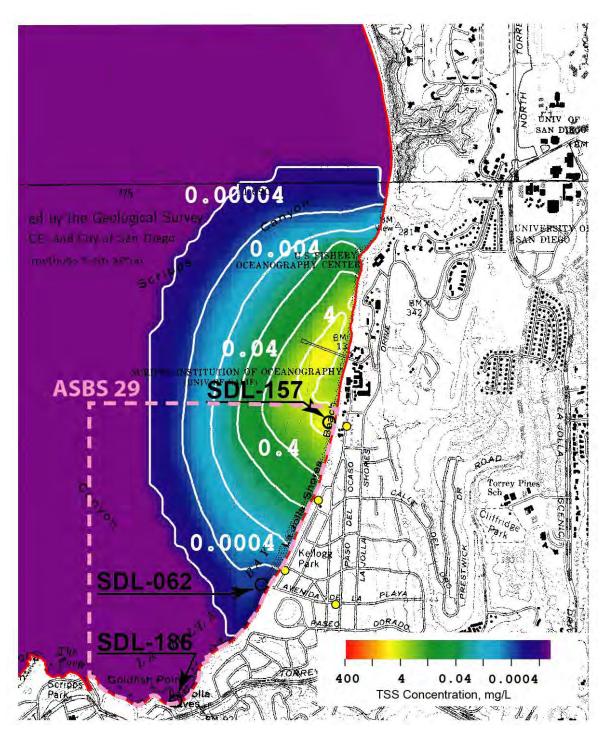


Figure 34. Depth averaged concentration of total suspended solids (TSS) for wet weather worst case due to discharge of storm water from SDL-157; 21 November 2011; waves: H = 1.8 m, T = 14 s, α = 283°; wind = 11 kts.

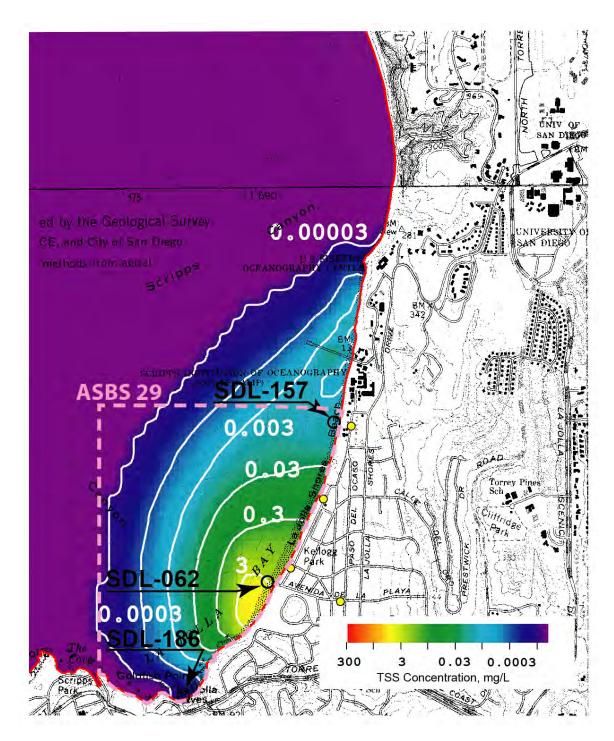


Figure 35. Depth averaged concentration of total suspended solids (TSS) for wet weather worst case due to discharge of storm water from SDL-062; 21 November 2011; waves: H = 1.8 m, T = 14 s, α = 283°; wind = 11 kts.

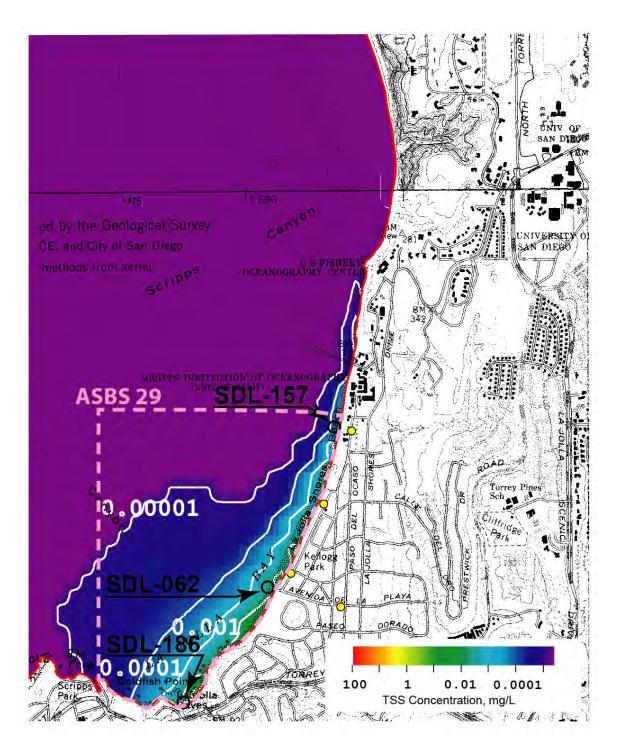


Figure 36. Depth averaged concentration of total suspended solids (TSS) for wet weather worst case due to discharge of storm water from SDL-186; 21 November 2011; waves: H = 1.8 m, T = 14 s, α = 283°; wind = 11 kts.

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this feature from SDL-062 is sandy, soft-bottom marine habitat. SDL-157 produces another patch with 10^2 minimum dilution, but this feature moves offshore and intrudes into the southern end of ASBS 31 (Figure-31), where again the marine habitat is a sandy soft-bottom type.

Figures 34-36 the corresponding concentrations of total suspended solids (TSS) in the storm water plumes discharged by the individual storm drains during the wet weather worst case from the monitoring program. Maximum TSS concentrations in the receiving water due to SDL-186 storm water discharges are at most 0.001 mg/L near the shore in the nearfield of SDL-186 (Figure 36) and more typically 0.0001 mg/L elsewhere along the bluff-faced shoreline of the southern end of ASBS 29, where significant high-value, hard bottom marine habitat resides. The highest TSS concentrations in the receiving water around SDL-062 (at the end of Avendia De La Playa, see Figure 35), were found to be 3 mg/L in a nearshore patch that that spreads south along the shore to the La Jolla Beach and Tennis Club. Most of the shoreline impacted by SDL-157 experiences maximum TSS concentrations from storm water in the range of 0.3 mg/L to 0.03 mg/L, all of which is sandy soft- bottom marine habitat. At the northern end of ASBS 29, and extending into ASBS 31, maximum TSS concentrations in the receiving from SDL-157 are in the range of 4 mg/L to 0.4 mg/L, the highest anywhere in La Jolla Bay. These relatively higher TSS values are probably a consequence of the steep land forms that comprise the watershed of SDL-157, with portions of both developed and undeveloped coastal bluffs. Regardless, the marine habitat subjected to these TSS loadings from SDL-157 is sandy, soft-bottom habitat type.

4.2 Long Term Minimum Dilution in the Surf Zone and Offshore

Because of the disparity in length scales between dilution in the offshore region versus the surfzone, a separate analysis was performed on a nested fine scale grid covering the surf zone for a long shore reach that extended 1000 ft (305 meters) either side of the beach outfalls. The size of this grid was based on precedent already set for the definition of a zone of initial dilution (ZID) by the Regional Water Quality Control Board, San Diego. All 11,688 combinations of receiving water variables from Figures 6 and 20 were input for daily simulations of dilution using the numerical codes described in Section 2 and listed in Appendices A-F. The ZID was searched for the minimum dilution after averaging across the width of the surf zone. Source loading for these simulations were based on the discharges measured during Wet-Weather Event-1 from outfalls SDI-157, SDL-062 and SDL-186. The surfzone dilution results are then compared to minimum dilution on the sea surface at an offshore control point in ASBS 29 where local water depth is -10 m MSL (Figure-1).

Figure 37 presents a probability density function (red histogram) with corresponding cumulative probability (blue line) of the minimum surfzone dilution within the ZID for storm drain SDL-157 for an ensemble of 11,688 daily outcomes (32 years). The median outcome is a minimum dilution factor of 32 to 1 based on the historical sequence of receiving water variables found in Figures 6-8 and 20. However the potential range of minimum dilution within the ZID goes as high as 88 to 1, and as low as 15 to 1. Low energy, dry weather ocean mixing produced the 15 to 1 minimum dilution outcome, which had a probability of occurrence of 0.13%. Altogether, 90% of the potential outcomes produce minimum dilutions in the ZID greater than 20 to 1.

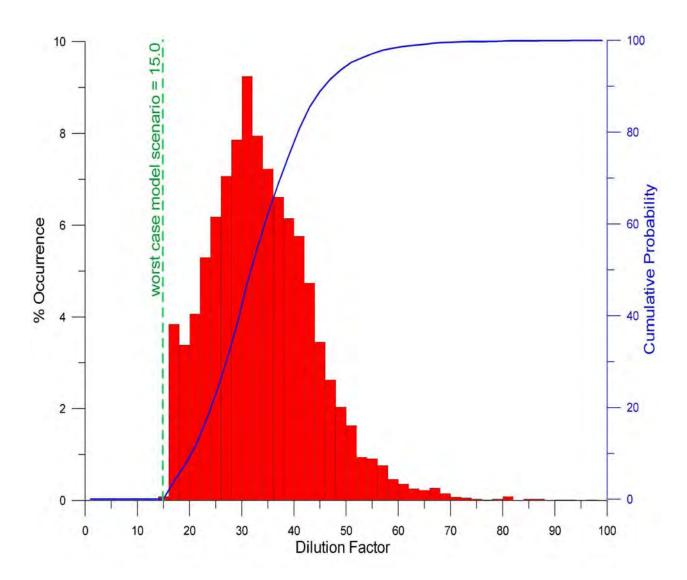


Figure 37. Probability density function (red) with corresponding cumulative probability (blue line) of the minimum surfzone dilution within the ZID around storm drain SDL-157. Derived from ocean boundary conditions and forcing functions, 1980-2012, 11,688 total numbers of realizations.

Minimum surfzone dilutions in the ZID off SDL-062 in Figure 38 are slightly less. This is due to the wave shadow found chronically in the refraction pattern of La Jolla Shores in the neighborhood of Avendia De La Playa (Figures 21 & 22). The minimum surfzone dilution of discharges from SDL-062 is found in Figure 38 to be 12.6 to 1, again a low energy, dry weather ocean mixing result. The median outcome of minimum surfzone dilution for SDL-062 is 22 to 1; while the potential range of minimum dilution goes as high as 64 to 1, based on the historical sequence of receiving water variables found in Figures 6-8 and 20. Ninety percent of the potential outcomes for SDL-062 produce minimum surfzone dilutions greater than 15 to 1.

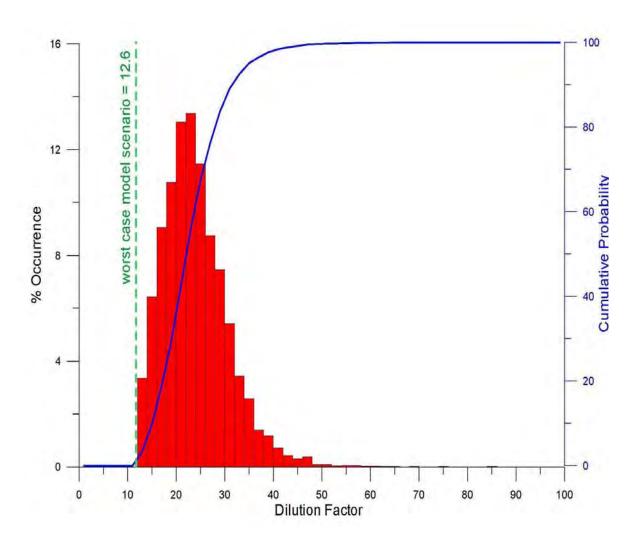


Figure 38. Probability density function (red) with corresponding cumulative probability (blue line) of the minimum surfzone dilution within the ZID around storm drain SDL-062. Derived from ocean boundary conditions and forcing functions, 1980-2012, 11,688 total numbers of realizations.

For comparison, probability statistics for minimum dilution are presented in Figure 39 at an offshore control point in ASBS 29 where local water depth is -10 m MSL (Figure-1). Here worst case minimum dilution of storm water is 13,200 to 1 while the median outcome from 11,688 model solutions is 52,000 to 1. The highest minimum dilutions of storm water at the offshore control point were found to be 4.2 x 10⁵. Consequently, storm water dilution is sufficiently high in the offshore regions of ASBS 29 that concentrations of storm water runoff constituents should be well below quantifiable detection limits.

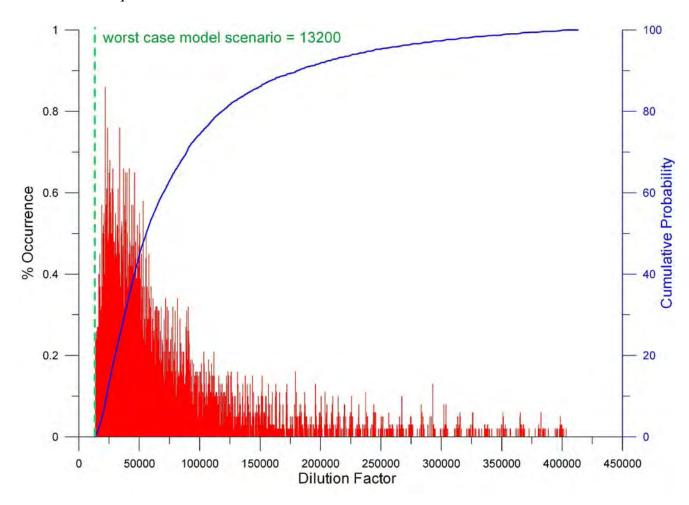


Figure 39. Probability density function (red) with corresponding cumulative probability (blue line) of the minimum dilution on the sea surface at offshore control point in ASBS 29. Derived from ocean boundary conditions and forcing functions, 1980-2012, 11,688 total numbers of realizations.

Revised: 13 March 2013

5.0 SUMMARY AND CONCLUSIONS:

During the 2011-2012 wet weather monitoring season (October 1, 2011 through April 30, 2012), storm water samples were collected from outfall discharge and receiving water mixing zone monitoring locations during three events (AMEC, 2012). These data were used to initialize the source loading inputs to a hydrodynamic model (*SEDXPORT*) used in the present receiving water analysis of storm water dilution and dispersion.

The *SEDXPORT* hydrodynamic modeling system was developed at Scripps Institution of Oceanography for the US Navy's *Coastal Water Clarity System* and *Littoral Remote Sensing Simulator*. This model has been peer reviewed multiple times and has been calibrated and validated in the Southern California Bight for six previous water quality and design projects.

Based on previous protocols established with the San Diego Regional Water Quality Control Board for storm drain dilution analysis in ASBS 31 (Jenkins and Wasyl, 2007), the numerical modeling study of the beach discharges is based on a wet weather worst-case scenario with a long-term probability assessment to bracket the envelope of the possible outcomes. Among the three wet-weather events monitored by AMEC (2012), Wet Weather Event-1 (20-21 November, 2011) comes closest to matching a computer search of marine environmental conditions for the wet-weather worst-case proxy. Analysis of the worst-case wet weather proxy was conducted over the entire coastal domain of La Jolla Bay and the Torrey Pines Littoral Sub-Cell.

The footprint of the dilution field for the wet weather worst case scenario, (involving the simultaneous discharge from storm drains SDL-157, SDL-062 and SDL-186), spreads about a kilometer seaward of the shoreline and about 2 kilometers along the shoreline, covering most of ASBS 29 and intruding into the southern portion of ASBS 31. The dilution factors of storm water in both ASBS 29 and ASBS 31 range between a minimum of 10² near shore to 10⁷ along the seaward boundaries during the wet weather worst case. Dilution factors of 10⁴ to 10⁷ characterize the outer one-half of ASBS 29, while dilution factors 10² to 10⁴ characterize the inner one-half. In the immediate neighborhood of SDL-157 and SDL-062, very near the shoreline of ASBS 29, dilution factors are as low as 40 to one.

The preponderance of the storm water discharge plume during the wet weather worst case scenario is from SDL-157 and SDL-062, while SDL-186 at Devil's Slide makes a very minor contribution. This is a fortuitous outcome in the sense that a significant amount of high-value, hard bottom marine habitat lives in the southern end of ASBS 29, and in the immediate neighborhood of SDL-186. Dilution factors of storm water discharged from SDL-186 are at most 10^3 near the shore in the nearfield of SDL-186, and more typically 10^4 to 10^5 elsewhere along the bluff-faced shoreline of the southern end of ASBS 29. The largest single contributing footprint to the combined storm water plume appears to be from SDL-062 at the end of Avendia De La Playa that produces a large patch with 10^2 minimum dilution very near the shoreline that spreads south along the shore to the La Jolla Beach and Tennis Club. The region influenced by this feature from SDL-062 is sandy, soft-bottom marine habitat. SDL-157 produces another patch with 10^2

minimum dilution, but this feature moves offshore and intrudes into the southern end of ASBS 31, where again the marine habitat is a sandy soft-bottom type.

Maximum concentrations of storm water born total suspended solids (TSS) in the receiving water due to SDL-186 storm water discharges are at most 0.001 mg/L near the shore in the nearfield of SDL-186, and more typically 0.0001 mg/L elsewhere along the bluff-faced shoreline of the southern end of ASBS 29, where significant high-value, hard bottom marine habitat resides. The highest TSS concentrations in the receiving water around SDL-062 (at the end of Avendia De La Playa), were found to be 3 mg/L in a nearshore patch that that spreads south along the shore to the La Jolla Beach and Tennis Club. Most of the shoreline impacted by SDL-157 experiences maximum TSS concentrations from storm water in the range of 0.3 mg/L to 0.03 mg/L, all of which is sandy soft-bottom marine habitat. At the northern end of ASBS 29, and extending into ASBS 31, maximum TSS concentrations in the receiving from SDL-157 are in the range of 4 mg/L to 0.4 mg/L, the highest anywhere in La Jolla Bay. These relatively higher TSS values are probably a consequence of the steep land forms that comprise the watershed of SDL-157, with portions of both developed and undeveloped coastal bluffs. Regardless, the marine habitat subjected to these TSS loadings from SDL-157 is a sandy, soft-bottom type.

Because of the disparity in length scales between dilution in the offshore region versus the surfzone, a separate analysis was performed on a nested fine scale grid covering the surf zone for a long shore reach that extended 1000 ft (305 meters) either side of the beach outfalls. The size of this grid was based on precedent already set for the definition of a zone of initial dilution (ZID) by the Regional Water Quality Control Board, San Diego. Thirty two years of receiving water variables (involving 11,688 distinct combinations) were input for daily simulations of dilution. The ZID was searched for the minimum dilution after averaging across the width of the surf zone. Source loading for these simulations were based on the discharges measured during Wet-Weather Event-1 from outfalls SDI-157, SDL-062 and SDL-186. The surfzone dilution results are then compared to minimum dilution on the sea surface at an offshore control point in ASBS 29 where local water depth is -10 m MSL.

This probability analysis procedure (based 11,688 historic combination of environmental variables) inevitably produces some combinations of wet weather discharges with dry weather ocean mixing conditions, and thereby results in certain worst case dilution scenarios that are more extreme, with even less dilution, than the wet weather worst case proxy derived from the monitoring program (Wet-Weather Event-1). For storm drain SDL-157, the median outcome for minimum dilution factor within the surfzone ZID is 32 to 1; however the potential range of minimum dilution goes as high as 88 to 1, and as low as 15 to 1. Low energy, dry weather ocean mixing produced the 15 to 1 minimum surfzone dilution outcome, which had a probability of occurrence of 0.13%. Altogether, 90% of the potential outcomes produce minimum dilutions in the surfzone ZID greater than 20 to 1.

Minimum surfzone dilutions in the ZID off SDL-062 are slightly less. This is due to the wave shadow found chronically in the refraction pattern of La Jolla Shores in the neighborhood of Avendia De La Playa. The minimum surfzone dilution of discharges from SDL-062 is found to be 12.6 to 1, again a low energy, dry weather ocean mixing result. The median outcome of minimum surfzone dilution for SDL-062 is 22 to 1; while the potential range of minimum dilution goes as high as 64 to 1, based on the 32-year historical sequence of receiving water variables. Ninety percent of the potential outcomes for SDL-062 produce minimum surfzone dilutions greater than 15 to 1.

For comparison, probability statistics for minimum dilution at an offshore control point in ASBS 29 (where local water depth is -10 m MSL) discovered a worst case minimum dilution of storm water of 13,200 to 1; while the median outcome is 52,000 to 1. The highest minimum dilutions of storm water at the offshore control point were found to be 4.2 x 10⁵. Consequently, storm water dilution is sufficiently high in the offshore regions of ASBS 29 that concentrations of storm water runoff constituents should be well below quantifiable detection limits.

6.0 REFERENCES AND BIBLIOGRAPHY

- Armi, L. A. (1979). Effects of variations in eddy diffusivity on property distributions in the oceans, Jour. of Mar. Res., v. 37, n. 3, p. 515-530.
- Berkoff, J. C. W. (1972). *Computation of combined refraction-diffraction*, Proc. 13th Coastal Eng. Conf., p. 471-490.
- Boas, M. L. (1966). *Mathematical Methods in the Physical Sciences*, John Wiley & Sons, Inc., New York, 778 pp.
- Coastal data information program (CDIP) (2001). University of California, San Diego (UCSD) Scripps Institution of Oceanography (SIO) Reference Series, 01-20 and http://cdip.ucsd.edu.
- Connor, J. J. and Wang, J.D. (1973). Finite element modeling of two-dimensional hydrodynamic circulation, MIT Tech Rpt., #MITSG 74-4, p. 1-57.
- Conte, S. D. and DeBoor, C. (1972). *Elementary Numerical Analysis*, Second Edition, McGraw-Hill Book Co., New York.
- Cox, R. A., and N. D. Smith (1959). The specific heat of sea water, Proc. Roy. Soc., A, 252, 51-62.
- Dalrymple, R. A., Kirby, J.T. and Hwang, P.A. (1984). *Wave diffraction due to areas of energy dissipation*, Jour. Waterway Port, Coast, and Ocean Engineering, v. 110, p. 67-79.
- DeGroot, S. R. and Mazur, P. (1984). *Non-Equilibrium Thermodynamics*, Dover Pub., Inc., New York, 510 pp.
- Durst, C. S. (1924). *The relationship between current and wind*, Quart. J. R. Met. Soc., v. 50, p. 113 (London).
- Finlayson, B. A. (1972). The Method of Weighted Residuals and Variational Principles, Academic Press.
- Flick, R.E. and Sterrett, E.H. (1994). *The San Diego Shoreline, Shoreline Erosion Assessment and Atlas of the San Diego Region*, Vol. 1, California Department of Boating and Waterways, 135 pp.
- Fujita, H. (1962). Mathematical Theory of Sedimentation Analysis. York: Academic, 315pp.
- Gallagher, R. H. (1981). Finite Elements in Fluids, John Wiley & Sons, New York, 290 pp.
- Goddard, L. and Graham, N.E. (1997). *El Niño in the 1990's*, Jour. Geophysical Res., v. 102, n. C5, p. 10,423-36.

- Inman, D. L., Nordstorm, C. E. and Flick, R. E. (1976). *Currents in submarine canyons: an air-sea-land interaction*, p. 275-310 in M. Van Dyke, et al (eds.), Annual Review of Fluid Mechanics, v. 8, 418 pp.
- Inman, D. L., Nordstorm, C. E. and Tait, R. J. (1971). *Mixing in the surf zone* Jour. Geophys. Res., v.76, no. 15, pp 3493-3514.
- Inman, D. L., Flick, R. E. and Guza, R. T. (1981). *Elevation and velocity measurements of laboratory shoaling waves*, J. Geophys. Res., 86(C5), 4149-4160.
- Inman, D. L. and Masters, P. M. (1991a). Coastal sediment transport concepts and mechanisms, Chapter 5 (43 pp.) in State of the Coast Report, San Diego Region, Coast of California Storm and Tidal waves Study, U. S. Army Corps of Engineers, Los Angeles District Chapters 1-10, Appen. A-I, 2 v.
- Inman, D. L. and Masters, P. M., (1991b). *Budget of sediment and prediction of the future state of the coast*, Chapter 9 (105 pp.) in *State of the Coast Report*, *San Diego Region*, *Coast of California Storm and Tidal Waves Study*, U. S. Army Corps of Engineers, Los Angeles District, Chapters 1-10, Appen. A-I; 2 v.
- Inman D. L. and Jenkins, S. A. (2006). *Thermodynamic solutions for equilibrium beach profiles*, Jour. Geophys. Res., v.3, C02003, doi:10.1029, 21pp.
- Inman, D. L. and Jenkins, S. A. (1999). *Climate change and the episodicity of sediment flux of small California rivers*, Jour. Geology, v. 107, p. 251–270. http://repositories.cdlib.org/sio/cmg/2/
- Inman, D. L. and Jenkins, S. A. (1997). *Changing wave climate and littoral drift along the California coast*, p. 538-49 in O. T. Magoon et al., eds., California and the World Ocean '97, American Society of Civil Engineers (ASCE), Reston, VA, 1756 pp.
- Inman, D. L. and Jenkins, S. A. (1996). A chronology of ground mine studies and scour modeling in the vicinity of La Jolla, UCSD SIO Reference Series 96-13, 26 pp.
- Inman, D. L. and Jenkins, S. A. (1985). *Erosion and accretion waves from Oceanside Harbor*, p. 591-3 in Oceans 85: Ocean Engineering and the Environment, Marine Technological Society & IEEE, v. 1, 674 pp.
- Inman D. L. and Jenkins, S. A. (1985). On a submerged sphere in a viscous fluid excited by small amplitude periodic motion, Jour. Fluid Mech., v. 157, p. 199-124.
- Inman, D. L., Jenkins, S. A. and Elwany, M. H. S. (1996). *Wave climate cycles and coastal engineering practice*, Coastal Engineering, 1996, Proc. 25th Int. Conf., (Orlando), ASCE, New York, v. 1, Ch 25, p. 314-27.

- Inman, D. L., Jenkins, S. A. and Elwany, M. H. S. (1993). *Shorerise and bar-berm profiles on ocean beaches*, Jour. Geophys. Res., 98(C10), p. 18, 181-199.
- Inman, D. L., Jenkins, S. A. and Van Dorn, W. G. (1981). *Evaluation of sediment management procedures*, UCSD SIO Reference Series No. 81-22, 212 pp.
- Inman D. L., Jenkins, S. A., Wasyl, J., Richardson, M. D., Wever, T.F. (2007). *Scour and burial mechanics of objects in the nearshore*, IEEE Jour.Oc.Eng, vol. 32, no. 1, pp 78-90.
- Inman D. L. and Komar, P. D. (1970). *Longshore sand transport of beaches*, Jour. Geophys. Res., v. 75, n. 30, p. 5914-5927.
- Inman, D. L., Pawka, S. S., Lowe, R. L., and Holmes, L. (1976). *Wave climate at Torrey Pines Beach, California*, U. S. Army Corps of Engineers, Coastal Engineering Research Center, Tech Paper 76-5, 372 pp.
- Jenkins, S. A., Paduan, J., Roberts, P., Schlenk, D., and Weis, J. (2012). *Management of Brine Discharges to Coastal Waters; Recommendations of a Science Advisory Panel*, submitted at the request of the California Water Resources Control Board, 56 pp. + App.
- Jenkins, S. A. and Skelly, D. W. (1989). *An evaluation of the coastal data base pertaining to seawater diversions at Encina Power Plant*, UCSD SIO Reference Series, #89-4, 52 pp.
- Jenkins, S. A. and Wasyl, J. (2007). *Hydrodynamic Modeling of Shoreline Discharges of Laboratory Seawater and Storm Water at Scripps Beach, CA*, submitted to UCSD Facilities Design and Construction, 266 pp.
- Jenkins, S. A. and Wasyl, J. (2005). *Coastal evolution model*, UCSD SIO Tech. Rpt. No. 58, 179 pp + appendices. http://repositories.cdlib.org/sio/techreport/58/
- Jenkins, S. A. and Wasyl, J. (1990). *Resuspension of estuarial sediments by tethered wings*, Jour. of Coastal Res., v. 6, n. 4, p. 961-980.
- Jenkins, S. A., Wasyl, J. and Aijaz, S. (1992). *Transport of fine sediment by hydrostatic jets*, Coastal and Estuarine Studies, American Geophysical Union, v. 42, p. 331-47.
- Jenkins, S. A., Wasyl, J. and Jenkins, S. A. (1989). *Dispersion and momentum flux study of the cooling water outfall at Agua Hedionda*, UCSD SIO Reference Series, #89-17, 36 pp.
- Jenkins, S. A., Wasyl, J., Cleveland, J. S., Talcott, J.C., Health, A.L., Goosby, S.G., Schmitt, K.F., and Leven, L.A. (1995). *Coastal water clarity modeling*, Science Applications International Corporation (SAIC), Tech. Rpt. 01-1349-03-4841-000, 491 pp.
- Jerlov, N.G. (1976). Marine Optics, Elsevier, Amsterdam, 231 pp.

- Kirby, J. T. (1986a). *Higher-order approximations in the parabolic equation method for water waves*, Jour. Geophys. Res., v. 91, C1, p. 933-952.
- Kirby, J. T. (1986b). *Rational approximations in the parabolic equation method for water waves*, Coastal Engineering, 10, p. 355-378.
- Kirby, J. T. (1986c). *Open boundary condition in the parabolic equation method*, Jour. Waterway, Port, Coastal, and Ocean Eng., 112(3), p. 460-465.
- Krone, R.B. (1962). Flume Studies of the Transport of Sediment in Estuarial Shoaling Processes, University of California, Berkeley Press, 69 pp.
- Lazara, B. J. and Lasheras, J. C. (1992a), *Particle dispersion in a developing free shear layer, Part 1, Unforced flow,* Jour. Fluid Mech. 235, p. 143-178.
- Lazara, B. J. and Lasheras, J. C. (1992b). *Particle dispersion in a developing free shear layer, Part 2, Forced Flow,* Jour. Fluid Mech., 235, p. 179-221.
- LePage, S. and Ware, R. (2001). Assessment of biological impacts of the Agua Hedionda jetty restoration project, Tech. Rpt. Submitted to Cabrillo Power 1LLC.
- List, E. J., Gartrell, G. and Winant, C. D. (1990). *Diffusion and dispersion in coastal waters*, Jour. Hydraulic Eng., v. 116, n. 10, p. 1158-79.
- Longuet-Higgins, M. S. (1970). *Longshore currents generated by obliquely incident waves*, Jour. Geophys. Res., v. 75, n. 33, p. 6778-6789.
- MACTEC. (2011a). La Jolla Shores Area of Special Biological Significance (ASBS) Compliance Monitoring Work Plan. October 2011. Prepared for the City of San Diego
- MACTEC. (2011b). La Jolla Shores Area of Special Biological Significance (ASBS) Compliance Monitoring Work Plan Addendum. November 2011. Prepared for the City of San Diego
- Martin, J. E. and Meiberg, E. (1994). The accumulation and dispersion of heavy particles in forced two-dimensional mixing layers, 1: The fundamental and subharmonic cases, Phys. Fluids, A-6, p. 1116-1132.
- National Oceanic and Atmospheric Administration (NOAA) (2000). *Verified/Historical Water Level Data*, http://www.opsd.nos.noaa.gov/data_res.html
- Neumann, G. (1952). *Ober die komplexe Natur des Seeganges, Teil 1 and 2*, Deut. Hydrogr. Zeit., v. 5, n. 2/3, p. 95-110, n. 5/6, p. 252-277.
- Neumann, G., Pierson, Jr., W. J. (1966). *Principles of Physical Oceanography*, Prentice-Hall, Inc., Englewood Cliffs, NJ, 545 pp.

- Nielsen, P. (1979). *Some basic concepts of wave sediment transport*, Series Paper No. 20, Institute of Hydrodyn. and Hydro. Eng., Tech. Univ. of Denmark.
- Oden, J. T. and Oliveira, E. R. A. (1973). *Lectures on Finite Element Methods in Continuum Mechanics*, The University of Alabama Press.
- Pawka, S. S. (1982). Wave directional characteristics on a partially sheltered coast, PhD dissertation, UCSD, 246 pp.
- PBS&J (2005). UCSD/SIO seawater separation and discharge" Appendix C of Jacobs and Associates design submission to UCSD Facilities, Design and Construction May 20, 2005, 10 pp + figs.
- Radder, A. C. (1979). *On the parabolic equation method for water-wave propagation*, J. Fluid Mech., 95, part 1, p. 159-176.
- Schmidt, W. (1917). Wirkungen der ungeordneten Bewegungen im Wasser der Meere und Seen, Ann. D. Hydr. u. Marit. Meteorol., vol. 45, p. 367-381.
- Schoonmaker, J. S., Hammond, R. R., Heath, A. L., and Cleveland, J. S. (1994). *A numerical model for prediction of sub-littoral optical visibility*, SPIE Ocean Optics XII, 18 pp.
- Scripps Institution of Oceanography (SIO) (2001). SIO shore station, Scripps Pier, http://www-mlrg.ucsd.edu/shoresta/mnSIOMain/siomain.htm
- Stommel, H. (1949). *Horizontal diffusion due to oceanic turbulence*, Journal of Marine Research, v. VIII, n. 3, p. 199-225.
- Thorade, H. (1914). *Die Geschwindigkeit von Triftstormungen und die Ekman'sche Theorie*, Ann. D Hydr. u. Marit. Meteorol., v. 42, p. 379.
- University of California, San Diego (UCSD) (2005a). *Quarterly (January March, 2005) Monitoring Report Attachment C*, Discharge Order No. R9-2005-0008, NPDES Permit No. CA0107239. UCSD SIO. June 1, 2005.
- UCSD (2005b). *Quarterly (April June, 2005) Monitoring Report -. Attachment C*, Discharge Order No. R9-2005-0008, NPDES Permit No. CA0107239. UCSD SIO. Volume I. September 1, 2005.
- UCSD, (2005c). *Quarterly (January March, 2005) Monitoring Report Attachment B*, Discharge Order No. R9-2005-0008, NPDES Permit No. CA0107239. UCSD SIO. June 1, 2005.

Revised: 13 March 2013

- UCSD, (2005d). *Quarterly (April June, 2005) Monitoring Report Attachment B*, Discharge Order No. R9-2005-0008, NPDES Permit No. CA0107239. UCSD SIO. Volume I. September 1, 2005.
- UCSD, (2005e). Semi-Annual (January June, 2005) Monitoring Report, Discharge Order No. R9-2005-0008, NPDES Permit No. CA0107239. UCSD SIO. Volume II. September 1, 2005.
- Weston Solutions, Inc. (2011). La Jolla Shores Area of Special Biological Significance Regional Compliance Monitoring, 2010-2011, Final Report. June, 2011. Prepared for the City of San Diego.
- Weston Solutions, Inc. (2010). La Jolla Shores Area of Special Biological Significance Regional Compliance Monitoring, 2009-2010, Final Report. June, 2010. Prepared for the City of San Diego.
- Weston Solutions, Inc. (2009). La Jolla Shores ASBS Regional Compliance Monitoring, 2008-2009, Final Report. June, 2009. Prepared for the City of San Diego.
- Weston Solutions, Inc. (2007). La Jolla Shores Watershed Urban Runoff Characterization and Watershed Characterization Study, Final Report. July, 2007. Prepared for the City of San Diego.
- Wang, H. P. (1975). Modeling an ocean pond: a two-dimensional, finite element hydrodynamic model of Ninigret Pond, Charleston, Rhode Island, Univ. of Rhode Island, Marine Tech. Rpt., #40, p. 1-58.
- Weiyan, T. (1992). Shallow Water Hydrodynamics, Water & Power Press, Hong Kong, 434 pp.
- White, W. B. and Cayan, D.R. (1998). *Quasi-periodicity and global symmetries in interdecadal upper ocean temperature variability*, Jour. Geophysical Res., v. 103, n. C10, p. 21,335-54.
- Winant, C. D. (1974). *Internal surges in coastal waters*, Jour. Geophys. Res., v. 79, n. 30, p. 4523-26.

APPENDIX A

GOVERNING EQUATIONS AND CODE FOR THE TIDE_FEM CURRENT MODEL

Appendix A: Governing Equations and Code for the TIDE_FEM Current Model

A finite element approach was adapted in preference to more common finite difference shallow water tidal models, e.g., Leendertse (1970), Abbott et al (1973), etc. Finite difference models employ rectangular grids which would be difficult to adapt to the complex geometry of the La Jolla Bay. It is believed that large errors would accumulate from attempting to approximate the irregular boundaries of the La Jolla Bay system with orthogonal segments. On the other hand, finite element methods allow the computational problem to be contained within a domain bounded by a continuous contour surface, such as the $S_{\rm f}$ contours stored within the *bathy* bathymetry file.

A finite element tidal hydraulics model, **TIDE_FEM**, [Inman and Jenkins, 1996] was employed to evaluate the tidal hydraulics of the La Jolla Bay (containing ASBS 31). **TIDE_FEM** was built from some well-studied and proven computational methods and numerical architecture that have done well in predicting shallow water tidal propagation in Massachusetts Bay [Connor and Wang, 1974] and estuaries in Rhode Island, [Wang, 1975], and have been reviewed in basic text books [Weiyan, 1992] and symposia on the subject, e.g., Gallagher (1981).

TIDE_FEM employs a variant of the vertically integrated equations for shallow water tidal propagation after Connor and Wang (1975). These are based upon the Boussinesq approximations with Chezy friction and Manning's roughness. The finite element discretization is based upon the commonly used *Galerkin weighted residual method* to specify integral functionals that are minimized in each finite element domain using a variational scheme, see Gallagher (1981). Time integration is based upon the simple *trapezoidal rule* [Gallagher, 1981]. The computational architecture of TIDE_FEM is adapted from Wang (1975), whereby a transformation from a global coordinate system to a natural coordinate system based on the unit triangle (see Figure A-1) is used to reduce the weighted residuals to a set of order-one ordinary differential equations with constant coefficients. These coefficients (*influence coefficients*) are posed in terms of a *shape function* derived from the natural coordinates of each nodal point. The resulting systems of equations are assembled and coded as banded matrices and subsequently solved by *Cholesky's method*, see Oden and Oliveira (1973 and Boas (1966).

Specifying the Shape Function <N> for any 3-Node Triangular Element

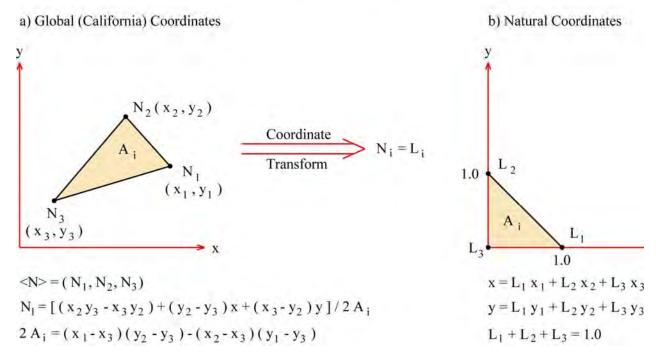


Figure A-1. Shape function polynomial and transform to natural coordinates for a generalized 3-node triangular element in the cross-shore plane

We adapt the California coordinates as our **global** coordinate system (x, y) to which the nodes in the computational mesh are referenced, with \mathbf{x} (easting) and y (northing). The vertical coordinate z is fixed at 0.0 ft NGVD and is positive upward. The local depth relative to 0.0 ft NGVD is \mathbf{h} and the mean surface elevation about 0.0 ft NGVD is $\mathbf{\eta}$. The total depth of water at any node is $\mathbf{H} = \mathbf{h} + \mathbf{\eta}$. The vertically averaged xy-components of velocity are (\mathbf{u}, \mathbf{v}) . The continuity and momentum equations may be written from Connor and Wang, (1974), as:

$$\frac{\partial}{\partial t} \rho H + \frac{\partial}{\partial x} q_x + \frac{\partial}{\partial y} q_y = 0$$

$$\frac{\partial}{\partial t} q_x + \frac{\partial}{\partial x} \overline{u} q_x + \frac{\partial}{\partial y} \overline{u} q_y = B_x + \frac{\partial}{\partial x} (F_{xx} - F_p) + \frac{\partial}{\partial x} F_{yx}$$

$$\frac{\partial}{\partial t} q_y + \frac{\partial}{\partial x} \overline{v} q_x + \frac{\partial}{\partial y} \overline{v} q_y = B_y + \frac{\partial}{\partial y} (F_{yy} - F_p) + \frac{\partial}{\partial x} F_{xy}$$
(A2)

Here $q_x,\,q_y$ are mass flux components

$$q_{x} = \rho \int_{-h}^{\eta} \overline{u} dz \tag{A3}$$

$$q_{y} = \rho \int_{-h}^{\eta} \overline{v} dz \tag{A4}$$

and \mathbf{q}_I is the mass flux through the ocean inlet due to water surface elevation changes in the estuary:

$$q_I = \rho \frac{\partial}{\partial t} \left(\frac{\partial s}{\partial \eta} \right) \tag{A5}$$

 \mathbf{F}_{p} is the pressure force resultant and \mathbf{F}_{xx} , \mathbf{F}_{xy} , \mathbf{F}_{yy} are "equivalent" internal stress resultants due to turbulent and dispersive momentum fluxes

$$F_{p} = \int_{-h}^{\eta} p dz = \frac{\rho g H^{2}}{2}$$

$$F_{xx} = 2\varepsilon \frac{\partial}{\partial x} q_{x}$$

$$F_{yy} = 2\varepsilon \frac{\partial}{\partial y} q_{y}$$

$$F_{yx} = F_{xy} = \varepsilon \left(\frac{\partial}{\partial y} q_{y} + \frac{\partial}{\partial x} q_{x}\right)$$
(A6)

and ε is the eddy viscosity. \mathbf{B}_x and \mathbf{B}_y are the bottom stress components

$$B_{x} = \tau_{x} + \rho g H \frac{\partial h}{\partial x}$$

$$B_{y} = \tau_{y} + \rho g H \frac{\partial h}{\partial y}$$
(A7)

In Equation (A7), τ_x and τ_y are the bottom shear stress components that are quasi-linearized by Chezy-based friction using Manning's roughness factor, \mathbf{n}_0 :

$$\tau_{x} = -\frac{g}{\rho H^{2} C_{z}^{2}} q_{x} (q_{x}^{2} + q_{y}^{2})^{1/2}$$

$$\tau_{y} = -\frac{g}{\rho H^{2} C_{z}^{2}} q_{y} (q_{x}^{2} + q_{y}^{2})^{1/2}$$
(A8)

where C_z is the Chezy coefficient calculated as:

$$C_z = \frac{1.49}{n_0} H^{1/6} \tag{A9}$$

Boundary conditions are imposed at the locus of possible land/water boundaries, S_f in the *bathym* file and at the shoreline, S_o . Flux quantities normal to these contours are denoted with "n" subscripts and tangential fluxes are given "s" subscripts. At any point along a boundary contour, the normal and tangential mass fluxes are:

$$q_{n} = \int_{-h}^{\eta} \rho u_{n} dz = \alpha_{nx} q_{x} + \alpha_{ny} q_{y}$$

$$q_{s} = \int_{-h}^{\eta} \rho u_{s} dz = -\alpha_{nx} q_{x} + \alpha_{ny} q_{y}$$

$$\alpha_{nx} = \cos(n, x)$$

$$\alpha_{ny} = \cos(n, y)$$
(A10)

Components of momentum fluxes across a boundary are equivalent to internal force resultants according to:

$$F_{nx} = \alpha_{nx}(F_{xx} - F_p) + \alpha_{ny}F_{yx}$$

$$F_{ny} = \alpha_{ny}(F_{yy} - F_p) + \alpha_{nx}F_{xy}$$
(A11)

On land boundary contours, the flux components are prescribed

$$q_n = q_s = 0 on land (A12)$$

On the deep water ocean boundary, the normal boundary forces (due to sea surface elevation) are continuous with ocean values, and the mass exchange is limited by the storage capacity of the estuary. Hence

$$F_{nm} = \overline{F}_{nm}$$
 and $q_{nm} = q_I$ ocean boundary (A13)

In the problem at hand \bar{F}_{nn} is prescribed on the shoreline by the ocean tidal elevation, η_0 , and the local depth, $\mathbf{h_0}$ according to

$$\overline{F}_{nm} = \frac{\rho g}{2} (\eta_0 + h_0)^2$$
 on S0 (A14)

Ocean *tidal forcing functions* η_0 were developed in Section 3.1. The ocean boundary condition as specified by Equation (A13) places a dynamic boundary condition on the momentum equations and a kinematic boundary condition on the continuity equation that is constrained by the storage rating curve. Solutions are possible by specifying only the dynamic boundary condition, but then mass exchanges are controlled by the wetting and drying of individual grid cells with associated discretization and interpolation errors which threaten mass conservation. The technique of over specifying the ocean boundary condition with both a dynamic and kinematic condition is discussed in the book by Weiyan (1992).

The governing equations (A2-A9) and the boundary conditions (A10-A13) are cast as a set of integral functionals in a variational scheme, [Boas, 1966]. Within the domain of each element of the mesh, A_i the unknown solution to the governing equations is simulated by a set of *trial functions* (\hat{H} , \hat{q}) having adjustable coefficients. The trial functions are substituted into the governing equations to form *residuals*, (R_H , R_q). The residuals are modified by *weighting functions*, (ΔH , Δq). The coefficients of the trial functions are adjusted until the weighted residuals vanish. The solution condition on the weighted residuals then becomes:

$$\iint_{A_i} R_H \Delta H dA = 0$$

$$\iint_{A_i} R_q \Delta q dA = 0$$

By the Galerkin method of weighted residuals, [Finlaysen, 1972], the weighting functions are set equal to nodal *shape functions*, <**N**>, or:

$$\Delta H \sim N_i$$
$$\Delta q \sim N_j$$

The shape function, $\langle N \rangle$, is a polynomial of degree which must be at least equivalent to the order of the highest derivative in the governing equations. The shape function also provides the mechanism to discretize the governing equations. Figure A-1 gives the shape function polynomial in terms of *global (California)* coordinates for the first nodal point, N_1 of a generalized 3-node triangular element of area A_i . Wang (1975) obtained significant numerical efficiency in computing the weighted residuals when the shape functions of each nodal point, N_i , are transformed to a system of *natural* coordinates based upon the unit triangle, giving $N_i \rightarrow L_i$ as detailed in Figure A-1. The shape functions also permit semi-discretization of the governing equations when the trial functions are posed in the form:

$$\hat{H}(x, y, t) = \sum_{i} H_{i}(t) N_{i}(x, y)$$

$$\hat{q}(x, y, t) = \sum_{j} q_{j}(t) N_{j}(x, y)$$
(A15)

Discretization using the weighting and trial functions expressed in terms of the nodal shape functions allows the *distribution* of dependent variables over each element to be obtained from the values of the independent variables at discrete nodal points. However, the shape function at any given nodal point, say N_1 , is a function of the independent variables of the two other nodal points which make up that particular 3-node triangular element, see Figure A-1. Consequently, the computations of the weighted residuals lead to a series of influence coefficient matrices defined by

$$a_{ij} = \frac{1}{A_i} \iint N_i N_j dA$$

$$s_{ij} = \frac{1}{A_i} \iint N_i \frac{\partial N_j}{\partial x} dA$$

$$t_{ij} = \frac{1}{A_i} \iint N_i \frac{\partial N_j}{\partial y} dA$$

$$g_{ijk} = \frac{1}{A_i} \iint N_i N_j \frac{\partial N_k}{\partial x} dA$$

$$h_{ijk} = \frac{1}{A_i} \iint N_i N_j \frac{\partial N_k}{\partial y} dA$$
(A16)

The influence coefficient matrices given by equation (A16) are evaluated in both global and natural coordinates. Once the influence coefficients have been calculated for each 3-node element, the weighted residuals reduce to a set of order-one ordinary differential with constant coefficients. The continuity equation becomes:

$$\sum \left(a_{ij} \frac{dH_{i}}{dt} \right) = -\sum_{i} \sum_{k} \left[g_{ijk} \left(H_{i} q_{xk} + H_{k} q_{xi} \right) + h_{ijk} \left(H_{i} q_{yk} + H_{k} q_{yi} \right) \right]
\sum \left(a_{ij} \frac{dq_{xj}}{dt} \right) = -\sum_{j} \sum_{k} \left[g_{ijk} \left(q_{xk} q_{xj} \right) + h_{ijk} \left(q_{yj} q_{xk} \right) \right] + N_{i} \sum_{j} N_{j} S_{fj} + g \sum_{i} s_{ij} H_{i}
\sum \left(a_{ij} \frac{dq_{yj}}{dt} \right) = -\sum_{j} \sum_{k} \left[g_{ijk} \left(q_{xj} q_{yk} \right) + h_{ijk} \left(q_{yj} q_{xk} \right) \right] + N_{i} \sum_{j} N_{j} S_{fj} + g \sum_{i} t_{ij} H_{i}$$
(A17)

Equations (A17) are essentially simple oscillator equations forced by the collection of algebraic terms appearing on the right hand side; and are therefore easily integrated over time. The time integration scheme used over each time step of the tidal forcing function is based upon the *trapezoidal rule*, see Gallagher 1981) or Conte and deBoor (1972). This scheme was chosen because it is known to be unconditionally stable, and in tidal propagation problems has not been known to introduce spurious phase differences or damping. It replaces time derivatives between two successive times, $\Delta t = t_{n+1} - t_n$, with a truncated Taylor series. For the water depth it would take on the form:

$$\frac{dH}{dt} = \eta(t)$$

$$H_{n+1} - H_n = \frac{\Delta t}{2} (\eta_{n+1} + \eta_n) + E\Delta t$$

$$E = \frac{1}{12} (\Delta t)^2 \left| \frac{d^2 \eta}{dt^2} \right|$$
(A18)

To solve equation (A18), iteration is required involving successive forward and backward substitutions.

Of particular interest are the *hydraulic friction slope coefficients*, S_{fj} , appearing as damping terms on the right hand side of equation (A17). Two separate formulations are used. One is given for the 3-node triangular elements situated in the interior of La Jolla Bay which do not experience successive wetting and drying during each tide cycle. The other formulation is used for the hydraulic friction slope coefficients for the elements situated along the wet and dry boundaries of the shoreline. These have been formulated as 3-node triangular elements with one curved side based upon the cubic-spline matrices developed by Weiyan (1992). The wet-dry boundary coordinates of the curved side, (x=, y=), are linearly interpolated for any given water elevation from the contours stored in the *bathym* file.

The influence and friction slope coefficient matrices together with the trapezoidal rule reduce equations (A16) and (A17) to a system of algebraic equations [Grotkop, 1973] which are solved by Cholesky's method per a numerical coding scheme by Wang (1975). For more details, refer to the TIDE_FEM code below, and Gallagher (1981) or Oden and Oliveira (1973).

Bathymetry errors are the most common cause of modeling errors. Other sources of errors include:

ELEMENT INTERPOLATION ERROR: Due to the degree of the polynomial used to specify shape function, N_i.

DISCRETIZATION ERRORS: Due to mesh coarseness and approximating the curved wet/dry boundary side of an element with a quadratic spline.

QUADRATURE ERRORS: Due to reducing the weighted residual integral with the influence coefficient matrices.

ITERATION ERRORS: Due to solving the system of algebraic equations reduced from the Galerkin Equations.

ROUNDOFF ERRORS: Due to time integration by the trapezoidal rule.

SEA LEVEL ANOMALIES: Due to discrepancies between the astronomic tides and the actual observed water levels in the ocean.

INSUFFICIENT CALIBRATION DATA: Due to limitations in the period of record

The following is a listing of the TIDE_FEM code:

```
C..tide_fem.f
c..Finite element tidal hydraulics model using the Galerkin weighted
c..residual process to implement the finite element scheme
c..with Chezy based friction terms.
c.. Adapted from Wang (1975) by Scott Jenkins and Joseph Wasyl.
c..7/16/04
c
     dimension X(500), Y(500), H(500), B1(1000,100), B2(500,50),
  & U(500), V(500), ETA(500), UVP(500), EP(500), MEXT(500),
  & MINT(500), R1(1000), R2(500), A(20,1000), Q(1000), MB(500),
  & CMAN(1000), CHEZ(1000), AREA(1000), N1(1000), N2(1000), N3(1000),
  & MH(10), MLE(200), ANG(200), tide(500), time(500)
   character*12 tidein, bathym, nodes, datout, temp
c
   open(20,file='tide_fem.inp',status='old')
   read(20,'(a12)')tidein
   read(20,'(a12)')bathym
   read(20,'(a12)')nodes
   read(20,'(a12)')temp
   read(20,'(a12)')datout
C..READ COMPUTATIONAL PARAMETERS
   READ(20,280) NN,NM,NW
280 FORMAT(3I10)
   READ(20,281) T,TLIM,DT,WW
281 FORMAT(4F10.0)
   READ(20,282) ATTD,HA,WX,WY
282 FORMAT(4F10.1)
   READ(20,283) STEP0,PUNCH
283 FORMAT(2F10.0)
   READ(20,284) UNIT, UNAR
284 FORMAT(2F10.0)
   READ(20,285) SILL
285 FORMAT(F10.3)
C
   open(21,file=bathym,status='old')
   open(22,file=nodes,status='old')
   open(23,file=tidein,status='old')
   open(24,file=datout,status='unknown')
   open(16,file=temp,status='unknown')
```

```
C..READ GLOBAL COORDINATE OF EACH NODE
  WRITE(16,298)
298 FORMAT(IH,' NODE NUMBER',5X,'X',9X,'Y',9X,'DEPTH',/)
  DO 205 I=1,NN
  READ(21,300)MEXT(I),MB(I),X(I),Y(I),H(I)
300 FORMAT(I4,I1,2F10.5,F3.1)
205 MINT(MEXT(I)=I
  WRITE(16,301)MEXT(I),I,MB(I),X(I),Y(I),H(I),I=1,NN
301 FORMAT(2(3I5,3F10.3))
C..READ ELEMENT DATA
  READ(22,302)N1(I),N2(I),N3(I),I=1,NM)
302 FORMAT(3I3)
  WRITE(16,303)
303 FORMAT(1H, 'THE ELEMENT CONNECTIONS',/)
  WRITE(16,304) (I,N1(I),N2(I),N3(I),I=1,NM)
304 FORMAT(5(I7,2X,3I4)
C
C..READ OPEN AND LAND BOUNDARY
  READ(22,306)MHNO,(MH(I),I=1,MHNO)
  READ(22,306)MLNO,(MLE(I),I=1,MLNO)
306 FORMAT(16I5)
  WRITE(16,307)MHNO
307 FORMAT(1H, 'THERE ARE', 15, 'NODES ON THE OPEN BOUNDARY')
  WRITE(16,309) (MH(I),I=1,MHNO)
  WRITE(16,308)MLNO
308 FORMAT(1H, THERE ARE', 15, 'NODES ON THE OPEN BOUNDARY')
  WRITE(16,309) (MLE(I),I=1,MLNO)
309 FORMAT(3X,15I5)
  READ(22,310)(ANG(I),I=1,MLNO)
310 FORMAT(8F10.4)
  WRITE(16,311)
311 FORMAT(1H, 'THE OUTWARD DIRECTIONS',/)
  WRITE(16,312)(MLE(I),ANG(I),I=1,MLNO)
312 FORMAT(5(I8,F10.4))
  STEBC=1.e10
  NBAND=0
  CRHO=0.00114
  G=32.174
  CDRAG=0.0025
```

```
F=3.141592/21600.0*SIN(ATTD/180.0*3.141592)
  NWN=2*NW-1
  NN1=NN*2
  NW1 = NW*2 + 1
  CCX=CDRAG*CRHO*ABS(WX)*WX
  CCY=CDRAG*CRHO*ABS(WY)*WY
  C1=1.0/3.0
  C2=1.0/3.0
  C3=1.0/3.0
  C4=1.0/12.0
  C5=1.0/12.0
  C6=1.0/12.0
  C7=1.0/6.0
  C8=1.0/6.0
  C9=1.0/6.0
  DO 201 I=1,NN
  X(I)=X(I)*UNIT
201 Y(I)=Y(I)*UNIT
  DO 202 I=1,MLNO
202 ANG(I)=ANG(I)*DATAN(6.1001)/45.0
  DO 204 I=1,NN1
  DO 204 J=1,NWN1
204 B1(I,J)=0.0
  DO 203 I=1,NN
  DO 203 J=1,NWN
203 B2(I,J)=0.0
C
C
  DO 10 I=1,NM
  N1(I)=MINT(N1(I))
  N2(I)=MINT(N2(I))
  N3(I)=MINT(N3(I))
  L1=IABS(N1(I)-N2(I))+1
  L2=IABS(N2(I)-N3(I))+1
  L3 = IABS(N3(I)-N1(I))+1
  IF(NBAND.LT.L1) NBAND=L1
  IF(NBAND.LT.L2) NBAND=L2
  IF(NBAND.LT.L3) NBAND=L3
C..TRANSFER TO LOCAL COORDINATE
C
  XP1=(2.0*X(N1(I))-X(N2(I))-X(N3(I)))/3.0
  XP2=(2.0*X(N2(I))-X(N1(I))-X(N3(I)))/3.0
```

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```

```
XP3=(2.0*X(N3(I))-X(N1(I))-X(N2(I)))/3.0
  YP1=(2.0*Y(N1(I))-Y(N2(I))-Y(N3(I)))/3.0
  YP2=(2.0*Y(N2(I))-Y(N1(I))-Y(N3(I)))/3.0
   YP3=(2.0*Y(N3(I))-Y(N1(I))-Y(N2(I)))/3.0
C
C..CALCULATE ELEMENT AREA
C
   AREA=0.5*(XP2*YP3+XP1*YP2+XP3*YP1-XP2*YP1-XP3*YP2-XP1*YP3)
\mathbf{C}
C..CALCULATE MANNING FACTOR
C
  CMAN(I)=0.03
\mathbf{C}
C..CALCULATE COEFFIECIENTS OF SHAPE FUNCTION
   A(1,I)=(XP2*YP3-XP3*YP2)/2.0*AREA(I))
  A(2,I)=(YP2-YP3)/2.0*AREA(I))
  A(3,I)=(XP3-XP2)/2.0*AREA(I))
   A(4,I)=(XP3*YP1-XP1*YP3)/2.0*AREA(I))
  A(5,I)=(YP3-YP1)/2.0*AREA(I))
  A(6,I)=(XP1-XP3)/2.0*AREA(I))
  A(7,I)=(XP1*YP2-XP2*YP1)/2.0*AREA(I))
  A(8,I)=(YP1-YP2)/2.0*AREA(I))
  A(9,I)=(XP2-XP1)/2.0*AREA(I))
C
C
  AREA(I)=AREA(I)*UNAR
  DIS1=SQRT((X(N1(I))-X(N2(I)))**2+(Y(N1(I))-Y(N2(I)))**2)
  DIS2=SQRT((X(N2(I))-X(N3(I)))**2+(Y(N2(I))-Y(N3(I)))**2)
  DIS3=SQRT((X(N3(I))-X(N1(I)))**2+(Y(N3(I))-Y(N1(I)))**2)
  DH1A=DIS1/SQRT(G*H(N1(I)))
  DH1B=DIS1/SQRT(G*H(N2(I)))
  DH2A=DIS2/SQRT(G*H(N2(I)))
  DH2B=DIS2/SQRT(G*H(N3(I)))
  DH3A=DIS3/SQRT(G*H(N3(I)))
  DH3B=DIS3/SQRT(G*H(N1(I)))
  IF(STEBC.GT.DH1A) STEBC=DH1A
  IF(STEBC.GT.DH1B) STEBC=DH1B
  IF(STEBC.GT.DH2A) STEBC=DH2A
  IF(STEBC.GT.DH2B) STEBC=DH2B
  IF(STEBC.GT.DH3A) STEBC=DH3A
  IF(STEBC.GT.DH3B) STEBC=DH3B
\mathbf{C}
```

```
C..FORM THE GLOBAL MATRIX
\mathbf{C}
  II=N1(I)
  JJ=N2(I)
  KK=N3(I)
  II1=NW
  JJ1=JJ-(II-NW)
  KK1=KK-(II-NW)
  B2(II,II1)=B2(II,II1)+C7*AREA(I)
  B2(II,JJ1)=B2(II,JJ1)+C4*AREA(I)
  B2(II,KK1)=B2(II,KK1)+C6*AREA(I)
  II1=II-(JJ-NW)
  JJ1=NW
  KK1=KK-(JJ-NW)
  B2(JJ,II1)=B2(JJ,II1)+C4*AREA(I)
  B2(JJ,JJ1)=B2(JJ,JJ1)+C8*AREA(I)
  B2(JJ,KK1)=B2(JJ,KK1)+C5*AREA(I)
  II1=II-(KK-NW)
  JJ1=JJ-(KK-NW
  KK1=KK-NW
  B2(KK,II1)=B2(KK,II1)+C6*AREA(I)
  B2(KK,JJ1)=B2(KK,JJ1)+C5*AREA(I)
  B2(KK,KK1)=B2(KK,KK1)+C9*AREA(I)
10 CONTINUE
   WRITE(16,320) NBAND, STEBC, UNAR
320 FORMAT(1H0,' THE BANDWIDTH = ',I5,'THE SMALLEST L/SQRT(GH) ='
  & ,F10.,//,1H,' ELEMENT AREA (UNIT',F10.2,' SQRT FEET)')
  WRITE(16,321)(I,AREA(I),I=1,MN)
321 FORMAT(5(I6,F12.4))
  DO 12 I=1.NN
  DO 12 J=1,NWN
  B1(2*I-1,2*J)=B2(I,J)
12 B1(2*I,2*J)=B1(2*I-1,2*J)
\mathbf{C}
C..ROTATE COORDINATE ON THE BOUNDARY
  LB1=NWN-1
  LB2=(NN1-NW1+2)/2
  DO 13 NI=1,MLNO
  I=MINT(MLE(NI))
  NMB=MB(I)
  GO TO (14,13,14,13),NMB
14 CALL ROTB1(B1,ANG(NI),NN1,NW1,NWN1,NW,I,LB1,LB2)
13 CONTINUE
```

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```
DO 23 NI=1,MLNO
  I=MINT(MLE(NI))
  NMB=MB(I)
  GO TO (15,23,15,21),NMB
15 DO 16 J=1,NWN1
16 B1(2*I-1,J)=0.0
  B1(2*I-1,NW1)=1.0
  GO TO 23
21 DO 22 J=1,NWN1
  B1(2*I-1,J)=0.0
22 B1(2*I,J)=0.0
  B1(2*I,NW1)=1.0
  B1(2*I-1,NW1)=1.0
23 CONTINUE
\mathbf{C}
C..L U DECOMPOSE MATRIX
  CALL MATRIX(B1,NN1,NW1,NWN1)
  DO 17 NI=1,MHNO
  I=MINT(MH(NI))
  DO 18 J=1,NWN
18 B2(I,J)=0.0
  B2(I,NW)=1.0
17 CONTINUE
  CALL MATRIX(B2,NN,NW,NWN)
\mathbf{C}
C
C..SET INITIAL VALUES
  READ(23,318)(I,TIDE(I),U(I),V(I),I=1,NN)
  DO 9 I=1,NN
  if(TIDE(I).GT.sill)THEN
  ETA(I)=TIDE(I)
  ELSE
  ETA(I)=SILL+0.00001
  ENDIF
  Q(2*I-1)=U(I)*(ETA(I)+H(I))
9 Q(2*I)=V(I)*ETA(I)+H(I)
C
  KL=0
  ML=0
  MLL=1
\mathbf{C}
```

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```
C..START SEMI-IMPLICIT METHOD
\mathbf{C}
  IF(KL.EQ.ML*STEP0) GO TO 27
  GO TO 28
27 \quad ML=ML+1
  TIM = T/60.0
  WRITE(16,315)TIM
315 FORMAT(/,' TIME='F10.2)
   WRITE(16,316)
316 FORMAT(2X, 'ELEMENT TIDE U VELOCITY V VELOCITY')
   WRITE(16,317)(I,MEXT(I),ETA(I),U(I),V(I),I=1,NN)
317 FORMAT(3(2I4,3F10.3))
28 IF(KL.EO.MLL*PUNCH) GO TO 34
  GO TO 36
34 MLL=MLL+1
   WRITE(24,318)(I,ETA(I),U(I),V(I),I=1,NN)
318 FORMAT(I5,3F1.4,I5,3F11.4)
36 CALL WH(T,HH,WW,HA)
  DO 38 NI=1,MHNO
  I=MINT(MH(NI))
  ETA(I)=HH
38 CONTINUE
39 DO 40 I=1,NN
  R1(2*I-1)=0.0
40 R1(2*i)=0.0
  DO 42 I=1.NM
  UU=Q(2*N1(I)-1)**2+Q(N*N2(I)-1)**2+Q(2*N3(I)-1)**2
   VV = Q(2*N1(I))**2 + Q(N*N2(I))**2 + Q(2*N3(I))**2
  AVEG = ((UU+VV)/3.0)**0.5
  HE1=H(N1(I))+ETA(N1(I))
  HE2=H(N2(I))+ETA(N2(I))
  HE3=H(N3(I))+ETA(N3(I))
   AVEG=((HE1+HE2+HE3)/3.0)**2
  CHEZ(I)=1.49/CMAN(I)*((HE1+HE2+HE3)/3.0)**(1.0/6.0)
  C746E=C7*ETA(N1(I))+C4*H(N2(I))+C6*ETA(N3(I))
  C485E=C4*ETA(N1(I))+C8*H(N2(I))+C5*ETA(N3(I))
  C659E=C6*ETA(N1(I))+C5*H(N2(I))+C9*ETA(N3(I))
  C746H=C7*H(N1(I))+C4*H(N2(I))+C6*H(N3(I))
  C485H=C4*H(N1(I))+C8*H(N2(I))+C5*H(N3(I))
  C659H=C6*H(N1(I))+C5*H(N2(I))+C9*H(N3(I))
  A258E=A(2,I)*ETA(N1(I))+A(5,1)*ETA(N2(I))+A(8,1)*ETA(N3(I))
   A369E=A(3,I)*ETA(N1(I))+A(6,1)*ETA(N2(I))+A(9,1)*ETA(N3(I))
```

PREX1=G*(C746H+C746E)*A258E

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PREX2=G*(C485H+C485E)*A258E

PREX3=G*(C659H+C659E)*A258E

PREY1=G*(C746H+C746E)*A369E

PREY2=G*(C485H+C485E)*A369E

PREY3=G*(C659H+C659E)*A369E

C746U=C7*Q(2*N1(I)-1)+C4*Q(2*N2(I)-1)+C6*Q(2*N3(I)-1)

C485U=C4*Q(2*N1(I)-1)+C8*Q(2*N2(I)-1)+C5*Q(2*N3(I)-1)

C659U=C6*Q(2*N1(I)-1)+C5*Q(2*N2(I)-1)+C9*Q(2*N3(I)-1)

C746V=C7*Q(2*N1(I))+C4*Q(2*N2(I))+C6*Q(2*N3(I))

C485V=C4*Q(2*N1(I))+C8*Q(2*N2(I))+C5*Q(2*N3(I))

C659V = C6*Q(2*N1(I)) + C5*Q(2*N2(I)) + C9*Q(2*N3(I))

QXX1=Q(2*N1(I)-1)**2/HE1

QXX2=Q(2*N2(I)-1)**2/HE2

QXX3=Q(2*N3(I)-1)**2/HE3

QYY1=Q(2*N1(I))**2/HE1

QYY2=Q(2*N2(I))**2/HE2

QYY3=Q(2*N3(I))**2/HE3

QXY1=Q(2*N1(I)-1)*Q(2*N1(I))/HE1

QXY2=Q(2*N2(I)-1)*Q(2*N2(I))/HE2

QXY3=Q(2*N3(I)-1)*Q(2*N3(I))/HE3

CONU = (A(2,I)*QXX1+A(5,I)*QXX2+A(8,1)*QXX3+

&A(3,I)*QXY1+A(6,I)*QXY2+A(9,I)*QXY3)/3.0

CONV = (A(2,I)*QXY1 + A(5,I)*QXY2 + A(8,1)*QXY3 +

&A(3,I)*QYY1+A(6,I)*QYY2+A(9,I)*QYY3)/3.0

GACA=G*AVEG/(CHEZ(I)**2)/AVEGH

FRU1=GACA*C746U

FRV1=GACA*C746V

FRU2=GACA*C485U

FRV2=GACA*C485V

FRU3=GACA*C659U

FRV3=GACA*C659V

UK1 = (CONU + PREX1 - F*C746V - CCX + FRU1)*AREA(I)

UK2 = (CONU + PREX2 - F*C485V - CCX + FRU2)*AREA(I)

UK3 = (CONU+PREX3-F*C659V-CCX+FRU3)*AREA(I)

VK1 = (CONV + PREY1 - F*C746V - CCX + FRU1)*AREA(I)

VK2= (CONV+PREY2-F*C485V-CCX+FRU2)*AREA(I)

VK3 = (CONV + PREY3 - F*C659V - CCX + FRU3)*AREA(I)

II=N1(I)

JJ=N2(I)

KK=N3(I)

R1(II*2-1)=R1(II*2-1)-UK1

R1(JJ*2-1)=R1(JJ*2-1)-UK2

R1(KK*2-1)=R1(KK*2-1)-UK3

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```
R1(II*2)=R1(II*2)-VK1
   R1(JJ*2)=R1(JJ*2)-VK2
42 R1(KK*2)=R1(KK*2)-VK3
\mathbf{C}
   T=T+DT/2.0
   DO 50 NI=1,MLNO
   I=MINT(MLE(NI))
   NMB=MB(I)
   GO TO (49,50,49,52),NMB
52 R1(2*I-1)=0.0
   R1(2*I)=0.0
   GO TO 50
49 CALL ROTUV(R1(2*I-1),R1(2*I),ANG(NI))
   R1(2*I-1)=0.0
50 CONTINUE
   CALL SOLVE(B1,R1,UVP,NN1,NW1,NWN1)
   DO 53 NI=1,MLNO
   I=MINT(MLE(NI))
   NMB=MB(I)
   GO TO (54,53,54,53),NMB
54 CALL ROTUV(UVP(2*I-1)+DT*UVP(2*I),-ANG(NI))
53 CONTINUE
   DO 56 I=1,NN
   Q(2*I-1)=Q(2*I-1)+DT*UVP(2*I-1)
56 Q(2*I)=Q(2*I)+DT*UVP(2*I)
   DO 62 I=1,NN
   HE=H(I)+ETA(I)
   U(I)=Q(2*I-1)/HE
   V(I)=Q(2*I)/HE
62 R2(I)=0.0
   DO 64 I=1,NM
   EK = (A(2,1)*Q(2*(N1(I))-1)+A(5,1)*Q(2*(N2(I))-1)+
  & A(8,1)*Q(2*(N3(I))-1)+A(3,1)*Q(2*(N1(I))-1)
  & A(6,1)*Q(2*(N2(I)))+A(9,1)*Q(2*(N3(I))))/3.0*AREA(I)
   II=N1(I)
   JJ=N2(I)
   KK=N3(I)
   R2(II)=R2(II)-EK
   R2(JJ)=R2(JJ)-EK
64 R2(KK)=R2(KK)-EK
   T=T+DT/2.0
   CALL WHP(T,HP,WW,HA)
   DO 66 NI=1,NHNO
```

```
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         I=MINT(MH(NI))
         EP(I)=HP
         R2(I)=EP(I)
      66 CONTINUE
         CALL SOLVE(B2,R2,EP,NN,NW,NWN)
         DO 68 I=1,NN
      68 ETA(I)=ETA(I)+DT*EP(I)
         KL=KL+1
         IF(T-TLIM) 25,25,70
      70 STOP
         END
      \mathbf{C}
         SUBROUTINE WH(T,HH,WW,HA)
         HH=COS(2.0*3.141592*T/WW)*HA
         RETURN
         END
      \mathbf{C}
         SUBROUTINE WHP(T,HP,WW,HA)
         HP=-2.0*3.141592/WW*SIN(2.0*3.141592*(T/WW))*HA
         RETURN
         END
      \mathbf{C}
      C..LU DECOMPOSITION
         SUBROUTINE MATRIX(A,N,NW,NWN)
         DIMENSION A(N,NWN)
         M=N-1
         DO 30 K=1,M
         I1=K+1
         NW1=NW+K-1
         IF(NW1.le.N) GO TO 20
         NW1=N
      20 DO 30 I=I1,NW1
         NI=NW-1+I1
         Y=A(I,NI-1)/A(K,NW)
         A(I,NI-1)=Y
         NW11=NI+NW-2
      45 DO 30 J=NI,NW11
         A(I,J)=A(I,J)-Y*A(K,J+I-K)
      30 CONTINUE
         RETURN
         END
```

 \mathbf{C}

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```
SUBROUTINE SOLVE(A,B,X,N,NW,NWN)
  DIMENSION A(N,NWN),B(N),X(N)
  X(1)=B(1)
  K1=NW-1
  DO 60 K=2,N
  SUM=0.0
  NW2=NW-K+1
  IF(K.le.NW) GO TO 54
  NW2=1
54 DO 55 J=NW2,K1
55 SUM=SUM+A(K,J)*X(J-NW+K)
60 \quad X(K)=B(K)-SUM
  X(N)=X(N)/A(N,NW)
  K=N
  NW4=NW+1
62 SUM=0.0
  K=K-1
  NW3=NW+N-K
  IF(NW3.LT.NWN) GO TO 64
  NW3=NWN
64 DO 65 J=NW4,NW3
65 SUM=SUM+A(K,J)*X(J-NW+K)
  X(K)=(X(K)-SUM)/A(K,NW)
  IF(K.EQ.1) GO TO 80
  GO TO 62
80 RETURN
  END
\mathbf{C}
C..COORDINATE POSITION
\mathbf{C}
  SUBROUTINE ROTB1(B1,ANGLE,NN1,NWN1,NWN1,LB1,LB2)
  DIMENSION B1(NN1,NWN1)
  NWM=NW-1
  IF(I.LE.NW) GO TO 20
  LB=LB1
9 DO 10 J=NW,LB
  II=I-J+NW-1
  CALL ROTSM(B1(2*II-1,2*J+2),B1(2*II-1,2*J+3),B1(2*II,2*J+1),
  &B1(2*II,2*J+2),ANGLE)
  NWJ=4*NW-2*J
  B1(2*I,NWJ-2)=B1(2*II-1,2*J+2)
  B1(2*I,NWJ-3)=B1(2*II-1,2*J+3)
  B1(2*I-1,NWJ-1)=B1(2*II,2*J+1)
```

```
B1(2*I-1,NWJ-2)=B1(2*II,2*J+2)
10 CONTINUE
  GO TO 30
20 IF(I.EQ.1)GO TO 30
  LB=I+NW-2
  GO TO 9
30 IF(I.GT.LB2 GO TO 60
  LB=1
50 DO 70 J=LB,NWM
  II=I-J+NW
  CALL ROTSM(B1(2*II-1,2*J),B1(2*II-1,2*J+1),B1(2*II,2*J-1),
  &B1(2*II,2*J),ANGLE)
  NWJ=4*NW-2*J
  B1(2*I,NWJ)=B1(2*II-1,2*J)
  B1(2*I,NWJ-1)=B1(2*II-1,2*J)
  B1(2*I-1,NWJ+1)=B1(2*II-1,2*J-1)
  B1(2*I-1,NWJ)=B1(2*II,2*J)
70 CONTINUE
  GO TO 90
60 IF(I.EQ.NN1/2)GO TO 90
  LB=1+(I-LB2)
90 RETURN
  END
\mathbf{C}
  SUBROUTINE ROTSM(SM1,SM2,SM3,SM4,ANGLE)
  SM1P=SM1*COS(ANGLE)+SM2*SIN(ANGLE)
  SM2=SM1*SIN(ANGLE)+SM2*COS(ANGLE)
  SM3P=SM3*COS(ANGLE)+SM4*SIN(ANGLE)
  SM4=SM3*SIN(ANGLE)+SM4*COS(ANGLE)
  SM1=SM1P
  SM3=SM3P
  RETURN
  END
\mathbf{C}
  SUBROUTINE ROTUV(A,B,ANGLE)
  AP=A*COS(ANGLE)+B*SIN(ANGLE)
  B=A*SIN(ANGLE)+B*COS(ANGLE)
  A=AP
  RETURN
  END
\mathbf{C}
```

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APPENDIX B CODE FOR THE TID_DAYS TIDAL FORCING MODEL

COMMON /AC2/ STORX,

Appendix B: Code for the TID_DAYS Tidal Forcing Model

```
c: Tidal Forcing Model TID_DAYS.FOR
        Sep 23, 2004
c..only does 1 month at user selected interval TIMER
c..modified STORX and u2 arrays to 50000
c..outputs in feet NGVD
c....6 minute
C This program outputs the predicted tidal amplitudes at 1 hr
C intervals for a 24 hr period beginning at local time 00:00 of
C January of the year selected in the input file. The input file
C must contain the amplitude and phase of the local tidal constituents
C (up to 37 constituents).
C Uses Long's code for the prediction of tides using harmonic
C equations from U.S. Dept of Commerse SP #98 1988
C by Scott A. Jenkins & Joseph Wasyl
character nameref*60,ifile*60
   LOGICAL
                   MKTABLE
   INTEGER
                STARTDATE(3), BEGIN_DAY(12), END_DAY(12)
   INTEGER
                   MO(12)
   DIMENSION
                      A(37),
                                AMP(37), PHASE(37)
                     SPD(37),
                                 AMPA(37), EPOCH(37)
   DIMENSION
                   STORX(50000)
   DIMENSION
   DIMENSION
                    YODE(37),
                                  YVPU(37)
   DIMENSION
                   JDAYF(12),
                                  JDAYL(12), JWKDA(12)
\mathbf{C}
C Tidal constituant speeds in degrees per hour
\mathbf{C}
c Tidal constituents speed, amplitude, phase angle read from data
c file tideconsts.dat (degrees/hr, feet, degrees)
c
c
   DATA
             MO/ 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12/
\mathbf{C}
   DATA BEGIN_DAY/ 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1/
\mathbf{C}
   DATA END_DAY/ 31, 28, 31, 30, 31, 30, 31, 30, 31, 30, 31, 30, 31 /
C
   COMMON /AC1/ SPD,
                            EPOCH,
                                         AMPA
```

MO, BEGIN_DAY, END_DAY,

```
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```

```
&
           NUMCON, MKTABLE,
                                       JP.
                                             LPYR,
  &
           TIMER,
                    NOHRS
\mathbf{C}
C PRELIMINARY CONSTANTS
\mathbf{C}
   MKTABLE=.TRUE.
                                !MAKE A TABLE
   CON=2048./90.
                           !UNITS/DEG, DISCRETE COS LOOP
   NUMCON=37
                             !# OF TIDAL CONSTITUANTS
                      TIMER=1. !# OF PREDICTIONS/HOUR
c* read from input file
                                !# OF MONTHS TO RUN
   NUM MONTHS=12
c
c
   open(25,file='tides1mo.inp',status='old')
c
  read(25,*)IYR
   read(25,*)IMO
   read(25,*)TIMER
   read(25,'(a)')nameref
   close(25)
c
   JP=IMO
c
    write(ifile,1000)nameref(1:LSTGDCHR(nameref))
c
1000 format(a)
c....tidal constituents from file name in input file 'tide_1yr.inp'
c
   open(26,file=ifile,status='old')
c
   do 49 ic=1,numcon
   read(26,*)A(ic),AMP(ic),PHASE(ic)
49 continue
c
   read(26,*)c1
   close(26)
c
   M2=AMP(1)
c
\mathbf{C}
C COMPUTE INDEX OF THE WEEKDAY OF 1 JAN OF COMPUTATION YEAR
C (1=SUNDAY, 2=MONDAY, ..., 7=SATURDAY)
\mathbf{C}
```

```
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   IDAY11=1+MOD((IYR-1+INT(FLOAT(IYR-1)/4.)),7)
\mathbf{C}
C CHECK FOR AND DEAL WITH LEAP YEAR
  LPYR=0
   NDAYS=365
   IF(MOD(IYR,4).EQ.0) THEN
    LPYR=1
    END_DAY(2)=29
    NDAYS=366
  END IF
C
C SET STARTING DATE
  STARTDATE(1)=IYR
                                !YEAR
   STARTDATE(2)=1
                              !MONTH
   STARTDATE(3)=1
                              !DAY
\mathbf{C}
C SET EQUILIBRIUM ARGUMENTS (?)
\mathbf{C}
   CALL EQU(NUMCON, STARTDATE, NDAYS, YODE, YVPU)
\mathbf{C}
C OUTPUT CONTENTS OF YODE AND YVPU (COMMENT OUT IF NOT NEEDED)
C
\mathbf{C}
    OPEN(11,FILE='EQUOUT.DAT',STATUS='UNKNOWN',FORM='FORMATTED')
\mathbf{C}
   WRITE(11,'(1X," YODE(J)",7X," YVPU(J)",/)')
C
   DO 10 J=1,NUMCON
C
     WRITE(11,'(1X,F8.3," ----- ",F8.3)') YODE(J),YVPU(J)
C 10 CONTINUE
C
   CLOSE(11)
\mathbf{C}
   WRITE(*,'(1X,/,1X,''Calculating tide predictions.'',1X,/)')
C CREATE OPERATING ARRAYS: AMPA, EPOCH, SPD
\mathbf{C}
  DO 20 J=1,NUMCON
    AMPA(J)=AMP(J)*YODE(J)
    TEMX=YVPU(J)-PHASE(J)
    IF(TEMX .LT. 0.) TEMX=TEMX+360.
    EPOCH(J)=TEMX*CON
    SPD(J)=A(J)*CON/TIMER
 20 CONTINUE
\mathbf{C}
```

```
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C COMPUTE JULIAN START AND END DAYS AND WEEK DAY OF 1ST OF
C EACH MONTH BASED ON END DAY ARRAY AND IDAY11
\mathbf{C}
   JDAYF(1)=1
  JDAYL(1)=END_DAY(1)
   JWKDA(1)=IDAY11
  DO 25 N=2,12
    JDAYF(N)=JDAYL(N-1)+1
    JDAYL(N)=JDAYL(N-1)+END_DAY(N)
    JWKDA(N)=1+MOD((JDAYF(N)+IDAY11-2),7)
 25 CONTINUE
\mathbf{C}
C CALL ROUTINES TO DO TIDAL PREDICTION CALCULATIONS
\mathbf{C}
C********Only calculate month from input file******
      DO 30 JP=1,NUM_MONTHS
    CALL CTIDE
    CALL TIDEOUT(M2,c1,IYR,IMO,IDAY,JDAYF(JP),JDAYL(JP),JWKDA(JP))
c..... 30 CONTINUE
C
   WRITE(*,'(1X,/,1X,"Tide calculations complete.")')
\mathbf{C}
  END
C
\mathbf{C}
C
  SUBROUTINE CTIDE
\mathbf{C}
C COMPUTES TIMER PREDICTIONS PER HOUR OF THE TIDES AND
C STORES THE RESULT IN ARRAY STORX.
C NUMCON = MAX NUMBER OF TIDAL CONSTITUENTS
C MKTABLE = LOGICAL FLAG; WHEN .TRUE. IT MAKES A TABLE
C
       OF 8193 COSINES (SAVED IN ARRAY XCOS) THAT
C
       REPRESENT 360 DEGREES FOR A TABLE LOOK-UP OF
\mathbf{C}
       THE TIDAL COSINE FUNCTION
C TABHR = HOURS TO THE BEGINNING OF EACH MONTH, OFFSET
\mathbf{C}
       BY 24. FIRST 12 ELEMENTS FOR A NORMAL YEAR;
```

LOGICAL MKTABLE
INTEGER BEGIN_DAY(12), END_DAY(12), MO(12)
DIMENSION XCOS(8193), ARG(37)

SECOND 12 FOR LEAP YEAR.

C

```
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                  SPD(37), EPOCH(37), AMPA(37)
  DIMENSION
                 TABHR(24), STORX(50000)
  DIMENSION
  DOUBLE PRECISION
                        Η
\mathbf{C}
  COMMON /AC1/
                     SPD.
                             EPOCH,
                                       AMPA
  COMMON /AC2/
                    STORX,
                                MO, BEGIN DAY,
  &
            END DAY,
                         NUMCON,
                                    MKTABLE,
  &
                              TIMER,
               JP,
                     LPYR,
  &
             NOHRS
\mathbf{C}
  DATA TABHR/ -24., 720., 1392., 2136., 2856., 3600.,
  &
         4320., 5064., 5808., 6528., 7272., 7992.,
  &
         -24., 720., 1416., 2160., 2880., 3624.,
  &
         4344., 5088., 5832., 6552., 7296., 8016./
\mathbf{C}
  IF(MKTABLE) THEN
    H=8.D0*DATAN(1.D0)/8192.D0
    DO 10 I=1.8193
     XCOS(I)=SNGL(DCOS(DFLOAT(I-1)*H))
10
    CONTINUE
    MKTABLE=.FALSE.
  END IF
C
C SET DATES AND TIMES
  NOD=END DAY(JP)-BEGIN DAY(JP)+1
                                        !# OF DAYS
  NOHRS=(NOD*24)*NINT(TIMER)+1
                                      !# OF SAMPLES
  NOHRS=NOHRS+24*NINT(TIMER)
                                      !ADD 12 HRS AT ENDS
c......NOHRS is the total number of tidal realizations
\mathbf{C}
C FIRST = # OF ESTIMATION POINTS (HOURS*TIMER), COUNTING
C FROM 0000 1 JAN OF PREDICTION YEAR, OF FIRST PREDICTION
\mathbf{C}
  FIRST=(TABHR(MO(JP)+12*LPYR)+24.*BEGIN DAY(JP))*TIMER
  FIRST=FIRST-12.*TIMER
                                !START @ 1200, DAY 0
C START MAIN COMPUTATION LOOP
\mathbf{C}
  DO 30 K=1,NOHRS
```

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C SET NUMCON VALUES OF ARG(J)

```
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\mathbf{C}
    IF(K.EQ.1) THEN
     DO 40 J=1,NUMCON
       ARGU=SPD(J)*FIRST+EPOCH(J)
       ARG(J)=AMOD(ARGU,8192.)
       IF(ARG(J).LT.0.) ARG(J)=ARG(J)+8192.
      CONTINUE
 40
    ELSE
     DO 50 J=1,NUMCON
       ARG(J)=ARG(J)+SPD(J)
       IF(ARG(J).GE.8192.) ARG(J)=ARG(J)-8192.
 50
      CONTINUE
    END IF
C SUM NUMCON CONSTITUENT CONTRIBUTIONS FOR ONE TIDAL ELEVATION
\mathbf{C}
    TIDE=0.
    DO 60 J=1, NUMCON
     NP=INT(ARG(J)+1.5)
     TIDE=TIDE+AMPA(J)*XCOS(NP)
 60
    CONTINUE
\mathbf{C}
C SET FINAL OUTPUT
\mathbf{C}
    STORX(K)=(TIDE+0.26)c..convert from feet MSL to feet NGVD C
 30 CONTINUE
\mathbf{C}
C END OF MAIN COMPUTATION LOOP
\mathbf{C}
   RETURN
   END
\mathbf{C}
\mathbf{C}
\mathbf{C}
\mathbf{C}
   SUBROUTINE TIDEOUT(M2,c1,IYR,IMO,IDAY,JDAYB,JDAYE,JDAYW)
\mathbf{C}
C WRITES TIDAL PREDICTION ARRAY AND COMPUTED TIME TO AN
C OUTPUT FILE. JDAYB AND JDAYE ARE BEGINNING AND ENDING
C JULIAN DAYS OF THE JP'TH MONTH. JDAYW IS THE WEEKDAY OF
C THE 1ST OF THE MONTH (1 = SUNDAY, 2 = MONDAY, ...,
```

```
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C 7 = SATURDAY
\mathbf{C}
   LOGICAL
                   MKTABLE
   CHARACTER*2
                         CYR
   CHARACTER*3
                     MONAME(12)
   CHARACTER*10
                        FNAME
                BEGIN_DAY(12), END_DAY(12),
                                                  MO(12)
   INTEGER
   DIMENSION
                   STORX(50000), u0(50000)
\mathbf{C}
   COMMON /AC2/
                        STORX,
                                      MO, BEGIN DAY,
  &
               END DAY,
                             NUMCON, MKTABLE,
  &
                 JP.
                         LPYR,
                                  TIMER,
  &
                NOHRS
C
   DATA MONAME/ 'Jan', 'Feb', 'Mar', 'Apr', 'May', 'Jun',
  &
           'Jul','Aug','Sep','Oct','Nov','Dec'/
\mathbf{C}
C CORRECTION MINUTES. HOURS AND DAYS FOR OFFSET START TIME
   IMNOFF=0
   IHROFF=0
   IDAOFF=0
C
C NAME OUTPUT FILE
   WRITE(CYR,'(I2)') MOD(IYR,100)
   FNAME=MONAME(JP)//CYR//'.DAT'
   WRITE(*,*) ' Writing to ', FNAME
C
   OPEN(10,FILE=FNAME,STATUS='UNKNOWN',FORM='FORMATTED')
\mathbf{C}
c******comment out the bullshit headers*****
    WRITE(10,'(2X,"Tides for ",A3,", ",I4)') MONAME(JP), IYR
    WRITE(10,'(2X,"Julian days ",I3," to ",I3)') JDAYB, JDAYE
    WRITE(10, (2X, I1, "= weekday of ", A3, "1st", /)) JDAYW,
c
   & MONAME(JP)
    WRITE(10,'(2X," date/time",3X,"m (NGVD)",/)')
c
\mathbf{C}
   PLUSTIME=60./TIMER
   IMN=-PLUSTIME + IMNOFF
                                     !START @ 0 MIN + FIX
                               !START @ NOON + FIX
   IHR=12 + IHROFF
   IDA=BEGIN DAY(JP) - 1 + IDAOFF !START @ DAY 0 + FIX
\mathbf{C}
```

```
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c*******only write out 00:00 day1 - 23:00 day 30,31,28,29)******
  TIMER12=TIMER*12
  NOHRS12T=NOHRS-12*NINT(TIMER)-1
  DO 10 I=1,NOHRS
    IMN=IMN+PLUSTIME
    IF(IMN.GE.60) THEN
     IHR=IHR+1
     IMN=IMN-60
    END IF
    IF(IHR.GE.24) THEN
     IDA=IDA+1
     IHR=IHR-24
    END IF
    ITIME=100*IHR+IMN
    if(I.LE.TIMER12)go to 1111
    if(I.GT.NOHRS12T)go to 1111
c
    Im1=I-1
eldif=(STORX(I)-STORX(Im1))
   u0(I)=(2.04*9.8*ABS(eldif)*M2/c1)**0.5
   if(eldif.LT.0)dir=0
   if(eldif.GE.0)dir=180
chrs=1.0*(I-TIMER12-1)/TIMER
                                 WRITE(10,'(F10.2,F10.4)')
  &
      chrs,STORX(I)
1111 continue
 10 CONTINUE
\mathbf{C}
  CLOSE(10)
\mathbf{C}
  RETURN
  END
\mathbf{C}
\mathbf{C}
C
  SUBROUTINE EQU(NSPED,IDT,LENGTH,FFF,VAU)
C CALCULATE TIDAL EQUILIBRIUM ARGUMENTS (VAU) AND NODE
C FACTORS (FFF). DEVELOPERS: E.E. LONG, B.B. PARKER,
```

```
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C L. HICKMAN AND G. FRENCH. NOTES: VAU IS CALCULATED
C FOR THE BEGINNING OF THE SERIES; FFF IS ADJUSTED TO
C THE MIDPOINT OF THE YEAR BEING CALCULATED
\mathbf{C}
                      LNAME, LABEL(37)
   CHARACTER*10
                 NODAYS(12), FFF(37), VAU(37), IDT(3)
   DIMENSION
   DIMENSION
                   CXX(30),
                              OEX(5)
   DOUBLE PRECISION SPEED, SPD(37)
\mathbf{C}
   COMMON /LOCAT/ TM, GONL
   COMMON/COSTX/ CXX, OEX
   COMMON /FAD/ IPICK
   COMMON /VEE/ TML, CON, U, Q, UI
   COMMON /BOXA/ S, XL, PM, PL, SL, PS,
            PLM, SKYN, VI, V, XI, VPP
  &
   COMMON /BOXB/ VP,
                           P, AUL, AUM, CRA, CQA
   COMMON /BOXS/ AW, AI, AE, AE1, ASP
\mathbf{C}
  DATA NODAYS/31, 28, 31, 30, 31, 30, 31, 30, 31, 30, 31, 30, 31 /
\mathbf{C}
  DATA LABEL/
             ', 'S(2)
  & 'M(2)
                      ', 'N(2)
                               ', K(1)
                      ', 'M(6)
                                ', 'MK(3)
             ', 'O(1)
  & 'M(4)
            ', 'MN(4)
                      ', 'Nu(2) ', 'S(6)
  & 'S(4)
             ', '2N(2)
                       ', 'OO(1) ', 'Lambda(2)',
  & 'Mu(2)
            ', 'M(1)
                      ', 'J(1)
                               ', 'Mm
  & 'S(1)
  & 'Ssa
            ', 'Sa
                     ', 'Msf
                              ', 'Mf
                                ', 'R(2)
  & 'Rho(1) ', 'Q(1)
                       ', 'T(2)
  & '2Q(1)
                      ', '2SM(2)
            ', 'P(1)
                                 ', M(3)
            ', '2MK(3) ', 'K(2)
  & 'L(2)
                                 ', M(8)
  & 'MS(4)
\mathbf{C}
C SET SOME VARIABLES:
   NSPED = NUMBER OF CONSTITUENTS TO BE CALCULATED
C
   MONTH = MONTH OF FIRST DATA POINT
    IDAY = DAY OF FIRST DATA POINT
\mathbf{C}
\mathbf{C}
    IYER = YEAR OF FIRST DATA POINT
C
  LENGTH = LENGTH (IN DAYS) OF SERIES TO BE GENERATED
\mathbf{C}
   IYER=IDT(1)
   DAYB=0.0
   GRBS=0.0
```

 \mathbf{C}

```
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C DEAL WITH LEAP YEAR FEBRUARY, DAYS/YEAR AND MIDDLE DAY/TIME
\mathbf{C}
   IF(MOD(IYER,4) .EQ. 0) THEN
    NODAYS(2)=29
    NDAY=366
    DAYM=184.0
    GRMS=0.0
   ELSE
    NODAYS(2)=28
    NDAY=365
    DAYM=183.0
    GRMS=12.0
   END IF
\mathbf{C}
   XYER=FLOAT(IYER)
C
C CALL ROUTINE TO COMPUTE BASIC ASTRONOMICAL CONSTANTS
   CALL ASTRO(XYER, DAYB, DAYM, GRBS, GRMS)
C
C MORE CONSTANTS FOR COMMON BLOCKS
   TM = 0.0
   GONL=0.0
   TML=0.0
  JUDAY=IDT(3)
\mathbf{C}
C LOOK UP CONSTITUENT PARAMETERS BY NAME MATCHING
  DO 10 J=1,NSPED
    LNAME=LABEL(J)
    SPEED=0.D0
    CALL NAME(SPEED,LNAME,ISUB,INUM,2)
    SPD(J)=SPEED
    CALL VANDUF(SPEED,E,F,1)
    FFF(J)=F
    VAU(J)=E
 10 CONTINUE
C ROUND NODE TO 3 DECIMAL PLACES, VO+U TO 1 DECIMAL PLACE
\mathbf{C}
```

DO 30 J=1.NSPED

FFF(J)=ANINT(FFF(J)*1000.)*0.001

```
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    VAU(J)=ANINT(VAU(J)*10.)*0.1
 30 CONTINUE
\mathbf{C}
   RETURN
  END
C
\mathbf{C}
\mathbf{C}
  SUBROUTINE ASTRO(XYER, DAYB, DAYM, GRBS, GRMS)
\mathbf{C}
C COMPUTES ASTRONOMICAL CONSTANTS FOR THE YEAR BEGINNING
C DAY (DAYB) AND HOUR (GRBS) AND FOR THE YEAR MIDDLE DAY
C (DAYM) AND HOUR (GRMS)
C
  DIMENSION CXX(30), OEX(5)
\mathbf{C}
   COMMON /LOCAT/ TM, GONL
   COMMON/COSTX/CXX, OEX
   COMMON /VEE/ TML, CON, U, Q, UI
   COMMON /BOXA/ S, XL, PM, PL, SL, PS, PLM,
           SKYN, VI, V, XI, VPP
   COMMON /BOXB/ VP, P, AUL, AUM, CRA, CQA
   COMMON /BOXS/ AW, AI, AE, AE1, ASP
  COMMON/BOXXS/ VIB, VB, XIB, VPB, VPPB, CXSB, CXPB,
  &
           CXHB, CXP1B
\mathbf{C}
  PINV = 57.29578
                          !degrees/radian
\mathbf{C}
C ORBIT SETS LUNAR AND SOLAR MEAN LATITUDES FOR THE BEGINNING OF
C THE CENTURY CONTAINING THE TIDE YEAR XYER (FROM TABLE 1 IN
C S.P. 98)
\mathbf{C}
   CALL ORBIT(XCEN,XSX,XPX,XHX,XP1X,XNX,OEX,T,XYER,5)
\mathbf{C}
C FROM TABLE 1 IN S.P. 98:
C
C XW = OBLIQUITY OF THE ECLIPTIC = MAX DECLINATION OF THE SUN
C
     (w) (DEGREES)
C XI = INCLINATION OF THE MOON'S ORBIT TO PLANE OF THE ECLIPTIC
     (i) (DEGREES)
C AW = XW IN RADIANS
C AI = XI IN RADIANS
```

```
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C AE = ECCENTRICITY OF MOON'S ORBIT (e)
C AE1 = ECCENTRICITY OF EARTH'S ORBIT (e1)
C ASP = SOLAR FACTOR (S')
   XW = 23.4522944 + T*(-0.0130125 + T*(-0.00000164 + T*(0.000000503)))
   XI = 5.14537628
   AW = 0.0174533*XW
                                          !w
   AI = 0.0174533*XI
                                       !I
   AE = 0.0548997
   AE1 = 0.01675104 + T^*(-0.0000418 + T^*(-0.000000126)) !e1
   ASP = 0.46022931
                                       !S'
\mathbf{C}
   DO 30 NOE=1,30
    CXX(NOE) = 0.0
 30 CONTINUE
C
C This next section must have something to do with the 1/4 day per
C year difference between the Common Year and the Julian Year
\mathbf{C}
                                         !reduce beginning day by 1
   IF(DAYB.GT.0.0) DAYB = DAYB-1.
                                          !reduce middle day by 1
   IF(DAYM.GT.0.0) DAYM = DAYM-1.
   AMIT = 0.0
                             !initialize correction
   AMI = XYER-XCEN
                                  !years into current century
   CPLEX = XCEN/400.+0.0001
                                     !century/400 + epsilon
                                       !remainder after century/400
   DICF = CPLEX-AINT(CPLEX)
   IF(AMI .EQ. 0.) GO TO 40
                                  !skip if exact century year
   XCET = XCEN+1.
                                !century year + 1
   CDIF = XYER-XCET
                                  !years into century - 1
                                   !(yrs in cent - 1)/4 + eps
   DOBY = CDIF/4.+0.0001
                                   !truncate (yrs in cent -1)/4
   AMIT = AINT(DOBY)
   IF(DICF .LT. 0.001) AMIT = AMIT+1.0 !add 1 if 1600's or 2000's
 40 CONTINUE
\mathbf{C}
   DBMT = DAYB + AMIT
                                    !fix for 1/4 day/(year in cent)
   DMMT = DAYM + AMIT
                                           ditto
C FROM TABLE 1, S.P. 98, BOX 4, CXX(1-8) ARE RATES OF CHANGE OF
C MEAN LATITUDE OF:
\mathbf{C}
C CXX(1) = MOON TO BEGINNING DAY AND BEGINNING HOUR
C CXX(2) = LUNAR PERIGEE TO BEGINNING DAY AND HOUR
C CXX(3) = SUN TO BEGINNING DAY AND HOUR
C CXX(4) = SOLAR PERIGEE TO BEGINNING DAY AND HOUR
```

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```
CXX(5) = LUNAR PERIGEE TO MIDDLE DAY AND HOUR
C CXX(6) = MOON'S NODE TO MIDDLE DAY AND HOUR
C CXX(7) = LUNAR PERIGEE TO MIDDLE DAY AND BEGINNING HOUR
C CXX(8) = MOON'S NODE TO BEGINNING DAY AND HOUR
C
   CXX(1)=XSX+ 129.384820*AMI+ 13.1763968*DBMT+0.549016532*GRBS
   CXX(2)=XPX+ 40.6624658*AMI+ 0.111404016*DBMT+0.004641834*GRBS
   CXX(3)=XHX- 0.238724988*AMI+ 0.985647329*DBMT+0.041068639*GRBS
   CXX(4)=XP1X+ 0.01717836*AMI+ 0.000047064*DBMT+0.000001961*GRBS
   CXX(5)=XPX+ 40.6624658*AMI+ 0.111404016*DMMT+0.004641834*GRMS
   CXX(6)=XNX- 19.3281858*AMI- 0.052953934*DMMT-0.002206414*GRMS
   CXX(7)=XPX+ 40.6624658*AMI+ 0.111404016*DMMT+0.004641834*GRBS
   CXX(8)=XNX-19.328185764*AMI-0.0529539336*DBMT-0.002206414*GRBS
C
C Round CXX(1-8) to nearest 0.01 deg and, if negative, increment
C by 360 deg (essentially modulo 360 arithmetic)
\mathbf{C}
   DO 50 J=1.8
    ZAT = CXX(J)/360.
                                   !# of loops + fraction
    IF(ZAT .LT. 0.) THEN
                                    !if negative angle
                                            !360 + fraction*360
      CXX(J) = ((ZAT-AINT(ZAT))+1.)*360.
    ELSE
                              !if positive angle
      CXX(J) = (ZAT-AINT(ZAT))*360.
                                          !fraction*360
    END IF
    CXX(J) = FLOAT(IFIX(CXX(J)*100.+0.5))*0.01!nearest 0.01 deg
 50 CONTINUE
C
C Use latitude of moon's node (N) at beginning day and hour
   ANG = CXX(8)
C
C Set astronomical constants at beginning day and hour
   CALL TABLE6(VIB, VB, XIB, VPB, VPPB, XX, XX, XX, XX, XX, ANG, ANB, ATB)
\mathbf{C}
   CXX(26) = VIB
                    !I at beginning day and hour
   CXX(27) = VB
                    !nu at
   CXX(28) = XIB
                    !zi at
   CXX(29) = VPB
                    !nu' at
   CXX(30) = VPPB
                     !2*nu" at
\mathbf{C}
   CXSB = CXX(1)
                     !s at beginning day and hour
   CXPB = CXX(2)
                     !p
```

```
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   CXHB = CXX(3)
                         !h
   CXP1B = CXX(4)
                         !p1
C
C Use latitude of moon's node (N) at middle day and hour
C
   ANG = CXX(6)
C
C Set astronomical constants at middle day and hour
   CALL TABLE6(VI,V,XI,VP,VPP,CIG,CVX,CEX,PVC,PVCP,ANG,AN,AT)
C
   CXX(9) = VI
                      !I at middle day and hour
   CXX(10) = V
                       !nu at
                                 "
   CXX(11) = XI
                       !zi at
   CXX(12) = VP
                       !nu' at
   CXX(13) = VPP
                        !2*nu" at
\mathbf{C}
C Round CXX(9-13) to nearest 0.01 deg
C
   DO 60 J=9,13
     CXX(J) = FLOAT(IFIX(CXX(J)*100.+0.5))*0.01
 60 CONTINUE
C
C Define P = p - zi following Eq. 191, para. 122.
   PGX = FLOAT(IFIX((CXX(5)-CXX(11))*100.+0.5))*0.01! define P to 0.01 deg
   ZAT = PGX/360.
                                          !modulo 360 math
   IF(ZAT .LT. 0.) THEN
     PGX = ((ZAT-AINT(ZAT))+1.)*360.
                                                    !360+fraction*360
   ELSE
                                                  !fraction*360
     PGX = (ZAT-AINT(ZAT))*360.
   END IF
                                              !P in radians
   XPG = PGX*0.0174533
   CXX(14) = PGX
\mathbf{C}
C For argument of constituent L2, compute R of Eq. 214, para. 129 (which
C info. is also tabulated in Table 8 of S.P. 98):
\mathbf{C}
C
           RAXE = sin(2P)
C
           RAXN = -cos(2P) + 1/6 [cot(I/2)]**2
\mathbf{C}
             R = \arctan(RAXE/RAXN)
\mathbf{C}
   RAXE = SIN(2.*XPG)
```

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```
RAXN = -\cos(2.*XPG)+1./(6.*(TAN(0.5*AT)**2))
   RXX = 0.
   IF(RAXE .EQ. 0. .OR. RAXN .EQ. 0.) GO TO 70
   RAX = RAXE/RAXN
                                           !tan(R)
   IF(RAX .GT. 3450.) GO TO 70
   RXX = ATAN(RAX)*PINV
                                              !R in degrees
   CXX(22) = RXX
 70 CONTINUE
\mathbf{C}
C For amplitude of constituent L2, compute 1/Ra with Eq. 213 of
C para. 129 [for which Table 7 lists log(Ra)]:
C
C 1/Ra = Sqrt[1 - 12 tan^2(I/2) cos(2P) + 36 tan^4(I/2)]
C
   CRA = SQRT(1.
  &
           -12.*(TAN(0.5*AT)**2)*COS(2.*XPG)
  &
           +36. * ( TAN(0.5*AT)**4 ) )
C Find constant terms in cosine argument of Eq. 212, para. 129.
C
                                        !2(zi-nu)
   UM2 = 2.*(CXX(11)-CXX(10))
   CXX(21) = UM2
                                  !2(zi-nu)
                                 !1/Ra
   CXX(24) = CRA
   U12
         = UM2-RXX
                                  !2(zi-nu)-R
   U12
        = U12+180.
                                 !2(zi-nu)-R+180
   CXX(15) = U12
C
C Compute Q of Eq. 202, para. 123:
C
C
          Q = \arctan[(5 \cos I - 1) * \tan P / (7 \cos I + 1)]
\mathbf{C}
   ZES
          = (5.*COS(AW)-1.) * SIN(XPG)
   ZEC
          = (7.*COS(AW)+1.) *COS(XPG)
   CALL FITAN(ZES,ZEC,QXX,SPXX,2)
   CXX(23) = QXX
                                  !Q in proper quadrant
C
C For constant terms in cosine argument of Eq. 201, para. 123.
C Note: sign on 90 is opposite to that given in S.P. 98
C
   CRAV = 0.5*UM2+QXX+90.
                                        !zi-nu+Q+90
   CXX(16) = CRAV
\mathbf{C}
C For 1/Qa of Eq. 195, para. 122 for M1 tide:
```

```
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C Note: changed AW to AT to be consistent with S.P. 98!!!
C
C \rightarrow OLD TERM: CQA = SQRT(0.25)
            +1.50 * COS(2.*XPG) * COS(AW) / (COS(0.5*AW)**2)
C &
C &
            +2.25*(COS(AW)**2)/(COS(0.5*AW)**4)
\mathbf{C}
   CQA = SQRT(0.25)
           +1.50 * COS(2.*XPG) * COS(AT) / (COS(0.5*AT)**2)
  &
  &
           +2.25*(COS(AT)**2)/(COS(0.5*AT)**4)
\mathbf{C}
   CXX(25) = CQA
C
C Round CXX(14-23) to nearest 0.01 deg
C
   DO 80 J=14,23
    CXX(J) = FLOAT(IFIX(CXX(J)*100.+0.5))*0.01
 80 CONTINUE
C Give names to some of the array elements
\mathbf{C}
   PM = CXX(1)
                       !s (beginning day)
   PL = CXX(2)
                      !p
   SL = CXX(3)
                      !h
   PS = CXX(4)
                      !p1
   PLM = CXX(5)
                       !p (middle day)
                        !N
   SKYN = CXX(6)
   VI = CXX(9)
                      !I
   V = CXX(10)
                      !nu
   XI = CXX(11)
                      !zi
   VP = CXX(12)
                       !nu'
   VPP = CXX(13)
                       !2*nu" "
   P = CXX(14)
   AUL = CXX(15)
                        !2zi-2nu-R+180 (middle)
   AUM = CXX(16)
                        !zi-nu+Q+90
   CRA = CXX(24)
                        !1/Ra
   CQA = CXX(25)
                        !1/Qa
C
   U = V*0.0174533 !nu in radians
   Q = P*0.0174533 !P in radians
   UI = VI*0.0174533 !I in radians
```

RETURN END

```
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C
C
\mathbf{C}
   SUBROUTINE FITAN(AUS, AUC, RTA, SPDX, JMAP)
C***********************
C THIS APPEARS TO BE A FORM OF THE FUNCTION ATAN2, BUT *
C WITH SOME ADDITIONAL STUFF.
IF(AUC .EQ. 0.) THEN
    IF(AUS .LT. 0.) THEN
      RTA = 270.
    ELSE IF(AUS .EQ. 0.) THEN
      RTA = 0.
    ELSE
      RTA = 90.
    END IF
   ELSE
    RTA = 57.2957795*ATAN(AUS/AUC)
    IF(JMAP .EQ. 2) THEN
      IF(AUS .LE. 0.) THEN
       IF(AUC .LT. 0.) THEN
         RTA = RTA + 180.
       ELSE IF(AUC .EQ. 0.) THEN
         RTA = 0.
       ELSE
         RTA = RTA + 360.
       END IF
      ELSE
       IF(AUC .LT. 0.) THEN
         RTA = RTA + 180.
       ELSE IF(AUC .EQ. 0.) THEN
         RTA = 90.
       END IF
     END IF
    END IF
   END IF
\mathbf{C}
   SPDX = 0.
\mathbf{C}
   RETURN
   END
\mathbf{C}
```

```
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C
C
\mathbf{C}
   SUBROUTINE ORBIT(XCEN,XSX,XPX,XHX,XP1X,XNX,OEX,T,
   & XYER,NNN)
\mathbf{C}
C Computes fundamental astronomical variables from Table 1 in
C S.P. 98 (1988 ed.)
\mathbf{C}
C S = rate of change of mean longitude of the moon per solar day
C P = rate of change of mean longitude of lunar perigee/solar day
C XH = rate of change of mean longitude of the sun per solar day
C P1 = rate of change of mean longitude of solar perigee/solar day
C XN = rate of change of mean longitude of moon's node/solar day
C T = number of Julian centuries (36525 days) reckoned from
C
      Greenwich mean noon, 31 December 1899
C YR = number of days and half days to correct astronomical
\mathbf{C}
      constants to 0000 hrs, 1 January of proper century, starting
\mathbf{C}
      with the year 1600 (correction is 0.5 days for 20th century,
\mathbf{C}
      i.e., noon on 31 December 1899 to 0000 hrs, 1 January 1900)
C GAT = the century (XCEN) in which the year XYER resides
C OEX = array of (NNN =) 5 elements, redundantly returned with the
C
      mean latitudes listed below as of 0000 hrs, 1 January of
\mathbf{C}
      the proper century:
\mathbf{C}
C
       XSX = OEX(1) = mean longitude of moon (s)
\mathbf{C}
       XPX = OEX(2) = mean longitude of lunar perigee (p)
\mathbf{C}
       XHX = OEX(3) = mean longitude of sun (h)
\mathbf{C}
      XP1X = OEX(4) = mean longitude of solar perigee (p1)
\mathbf{C}
       XNX = OEX(5) = mean longitude of moon's node (N)
\mathbf{C}
   DIMENSION OEX(NNN)
\mathbf{C}
   S = 13.1763968
   P = 0.1114040
   XH = 0.9856473
   P1 = 0.0000471
   XN = -0.0529539
   XCAN = XYER*0.01+0.001
   XCEN = AINT(XCAN)*100.
   T = -3.0
   YR = 2.5
   GAT = 1600.
```

```
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\mathbf{C}
   DO 10 JK=1,30
    GP = GAT/400.+0.00001
    COL = GP-AINT(GP)
    IF(COL .LT. 0.01) THEN
      IF(GAT .EQ. XCEN) GO TO 12
    ELSE
      IF(GAT .EQ. XCEN) GO TO 12
      YR = YR-1.
    END IF
    GAT = GAT + 100.
 10 CONTINUE
 12 CONTINUE
C
   T
       = (GAT-1900.)*0.01
\mathbf{C}
   OEX(1) = 270.437422 + T*(307.892 + T*(0.002525)
                     + T*(0.00000189))) + YR*S
   OEX(2) = 334.328019 + T*(109.032206 + T*(-0.01034444))
                     + T*(-0.0000125))) + YR*P
  &
   OEX(3) = 279.696678 + T*(0.768925 + T*(0.0003205))
                                + YR*XH
   OEX(4) = 281.220833 + T*(1.719175 + T*(0.0004528)
  &
                     + T*(0.00000333))) + YR*P1
   OEX(5) = 259.182533 + T*(-134.142397 + T*(0.00210556)
                     + T*(0.00000222))) + YR*XN
  &
\mathbf{C}
   DO 100 I=1,5
   ZAT = OEX(I)/360.
   IF(ZAT .LT. 0.) THEN
    OEX(I) = ((ZAT-AINT(ZAT))+1.)*360.
   ELSE
    OEX(I) = (ZAT-AINT(ZAT))*360.
   END IF
   OEX(I) = FLOAT(IFIX(OEX(I)*100.+0.5))*0.01
 100 CONTINUE
\mathbf{C}
   XSX = OEX(1)
   XPX = OEX(2)
   XHX = OEX(3)
   XP1X = OEX(4)
   XNX = OEX(5)
```

 \mathbf{C}

```
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    RETURN
    END
\mathbf{C}
\mathbf{C}
\mathbf{C}
\mathbf{C}
    SUBROUTINE VANDUF(SPEED,E,F,ITYPE)
\mathbf{C}
C Computes phases and node factors for tidal constituents.
\mathbf{C}
C References:
C S.P. 98 = US Dept of Commerce, Coast and Geodetic Survey,
C
          "Manual of Harmonic Analysis and Prediction
         of Tides," 1988 Ed.
\mathbf{C}
C A.M. = British Admiralty, "Admiralty Manual of Tides,"
\mathbf{C}
         1941 Ed.
\mathbf{C}
C Common block variables have the following meanings (as
C best I can tell):
\mathbf{C}
C /LOCAT/ TM and GONL are 0.0 in main program, not defined,
C
        but may be related to meridian differences of
C
        local time and Greenwich time per Eq. 318, para.
\mathbf{C}
        223 in S.P. 98
C
C /FAD/ IPICK is defined in this routine as constituent
        index based on matching SPEED with array of SPD
\mathbf{C}
\mathbf{C}
        in NAMES common block
C
C /VEE/ TML = not defined (may relate to TM in /LOCAT/)
        CON = defined in this routine
\mathbf{C}
C
         U = nu for middle day in radians
C
          O = P
\mathbf{C}
         UI = I
\mathbf{C}
C /BOXA/ S = not defined
\mathbf{C}
         XL = not defined
C
         PM = s for beginning day in degrees
\mathbf{C}
         PL = p
\mathbf{C}
         SL = h
C
         PS = p1
\mathbf{C}
        PLM = p for middle day in degrees
```

```
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\mathbf{C}
       SKYN = N
C
        VI = I
\mathbf{C}
        V = nu
C
        XI = zi
\mathbf{C}
       VPP = nu"
C
C /BOXB/ VP = nu' for middle day in degrees
\mathbf{C}
\mathbf{C}
       AUL = 2*zi - 2*nu - R + 180, middle day, deg
C
       AUM = zi - nu + Q + 90, middle day, deg
C
       CRA = 1/Ra for L(2) constituent
C
       CQA = 1/Qa for M(1) constituent
\mathbf{C}
C /BOXS/ AW = w obliquity of ecliptic (max. declination of Sun)
\mathbf{C}
        AI = i inclination of Moon's orbit to ecliptic plane
\mathbf{C}
        AE = e eccentricity of Moon's orbit
\mathbf{C}
       AE1 = e1 eccentricity of Earth's orbit
\mathbf{C}
       ASP = S' Solar factor
C
C Order of constituents is same as in NAMES common.
\mathbf{C}
C NOTE: Constituents marked with * have phases shifted
C by 180 deg from what is given in S.P. 98. These are coded
C with the option of adding 180 \text{ deg by setting ISHIFT} = 1.
C Original code is retained by setting ISHIFT = 0.
\mathbf{C}
   DOUBLE PRECISION
                             SPEED, SPD(37)
   DIMENSION
                       MS(37)
C
   COMMON /LOCAT/ TM, GONL
   COMMON /FAD/ IPICK
   COMMON /VEE/ TML, CON, U, Q, UI
   COMMON /BOXA/ S, XL, PM, PL, SL, PS, PLM,
               SKYN, VI, V, XI, VPP
   &
   COMMON /BOXB/ VP, P, AUL, AUM, CRA, CQA
   COMMON /BOXS/ AW, AI, AE, AE1, ASP
   COMMON /SPEEDS/ SPD
   COMMON /MMSS/
                          MS
C
                     ! if 1, flips *'d constituents by 180 deg
   ISHIFT=0
C
   CON = SL + TML
                          ! h, beginning day + TML
   C5AW = COS(0.5*AW) ! cos(w/2)
```

```
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   C5AI = COS(0.5*AI) ! cos(i/2)
   C5UI = COS(0.5*UI) \quad ! \cos(I/2)
   SAW = SIN(AW) ! sin w
   SAI = SIN(AI) ! sin i
   SUI = SIN(UI) ! sin I
\mathbf{C}
   DO 600 J=1,37
    IPICK = J
    IF(SPEED .EQ. SPD(J)) GO TO 610
 600 CONTINUE
   WRITE(*,'(2X,"SPEED MATCH NOT FOUND IN VANDUF -> QUIT")')
   WRITE(*,'(2X,"SPEED = ",F12.7)') SPEED
   STOP
 610 CONTINUE
\mathbf{C}
   GO TO (1, 2, 3, 4, 5, 6, 7, 8, 9, 10,
        11, 12, 13, 14, 15, 16, 17, 18, 19, 20,
  &
        21, 22, 23, 24, 25, 26, 27, 28, 29, 30,
  &
        31, 32, 33, 34, 35, 36, 37), IPICK
C CONSTITUENT CALCULATION CODES
C
C M(2): term A39, p. 22 and Eq. 70, 78, S.P. 98
  1 E = 2.*(CON-PM+XI-V)
   F = (C5AW^{**}4)^*(C5AI^{**}4)/(C5UI^{**}4)
   GO TO 800
\mathbf{C}
C N(2): term A40, p. 22 and Eq. 70, 78, S.P. 98
\mathbf{C}
  2 E = 2.*(CON+XI-V)-3.*PM+PL
   F = (C5AW**4)*(C5AI**4)/(C5UI**4)
   GO TO 800
\mathbf{C}
C S(2): p. 39, S.P. 98
  3 E = 2.*TML
   F = 1.
   GO TO 800
C
C *O(1): term A14, p. 21 and Eq. 67, 75, S.P. 98
```

```
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  4 E = CON-V-2.*(PM-XI)-90.+ISHIFT*180.
   F = SAW*(C5AW**2)*(C5AI**4)/(SUI*(C5UI**2))
   GO TO 800
\mathbf{C}
C *K(1): Eq. 222 and 227, p. 45, S.P. 98
  5 E = CON-VP+90.+ISHIFT*180.
   F = SQRT(0.8965*(SIN(2.*UI)**2)+0.6001*SIN(2.*UI)*COS(U)
   &
                                    +0.1006)
   F = 1./F
   GO TO 800
C
C K(2): Eq. 230 and 235, p. 46, S.P. 98
  6 E = 2.*CON-VPP
   F = SQRT(19.0444*(SUI**4)+2.7702*(SUI**2)*COS(2.*U)+0.0981)
   F = 1./F
   GO TO 800
C
C L(2): Term A41, p. 22, Eq. 212, p. 44, and Eq. 70, 78, and 215,
\mathbf{C}
      S.P. 98
C
  7 E = 2.*CON-PM-PL+AUL
   F = (C5AW^{**}4)^*(C5AI^{**}4)/(CRA^*(C5UI^{**}4))
   GO TO 800
\mathbf{C}
C 2N(2): Term A42, p. 22, and Eq. 70, 78, S.P. 98
  8 E = 2.*(CON+XI-V+PL)-4.*PM
   F = (C5AW^{**}4)^*(C5AI^{**}4)/(C5UI^{**}4)
   GO TO 800
\mathbf{C}
C R(2): Term B47, p. 39, and para. 118, p. 40, S.P. 98
  9 E = SL-PS+180.+2.*TML
   F = 1.
   GO TO 800
\mathbf{C}
C T(2): Term B40, p. 39, and para. 118, p. 40, S.P. 98
  10 E = 2.*TML-(SL-PS)
   F = 1.
```

GO TO 800

```
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C
C Lambda(2): Term A44, p. 22, and Eq. 70, 78, S.P. 98
  11 E = 2.*(CON+XI-V-SL)-PM+PL+180.
   F = (C5AW**4)*(C5AI**4)/(C5UI**4)
   GO TO 800
C
C Mu(2): Term A45, p. 22, and Eq. 70, 78, S.P. 98
\mathbf{C}
 12 E = 2.*(CON+XI-V+SL)-4.*PM
   F = (C5AW^{**}4)^*(C5AI^{**}4)/(C5UI^{**}4)
   GO TO 800
\mathbf{C}
C Nu(2): Term A43, p. 22, and Eq. 70, 78, S.P. 98
 13 E = 2.*(CON+XI-V+SL)-3.*PM-PL
   F = (C5AW**4)*(C5AI**4)/(C5UI**4)
   GO TO 800
C
C *J(1): Term A24, p. 22, and Eq. 68, 76, S.P. 98
 14 E = CON + PM - PL - V + 90 + ISHIFT * 180.
   F = SIN(2.*AW)*(1.-1.5*(SAI**2))/SIN(2.*UI)
   GO TO 800
\mathbf{C}
C *M(1): Eq. 201, p. 42, and Eq. 206, p. 43, S.P. 98
\mathbf{C}
  15 E = CON-PM+AUM+ISHIFT*180.
   F = SAW*(C5AW**2)*(C5AI**4)/(CQA*SUI*(C5UI**2))
   GO TO 800
\mathbf{C}
C *OO(1): Term A31, p. 22, and Eq. 69, 77, S.P. 98
  16 E = CON-V+2.*(PM-XI)+90.+ISHIFT*180.
   F = SAW*(SIN(0.5*AW)**2)*(C5AI**4)/(SUI*(SIN(0.5*UI)**2))
   GO TO 800
C
C P(1): Term B14, p. 39, and para. 118, p. 40, S.P. 98
 17 E = TML + 270.-SL
   F = 1.
   GO TO 800
C
```

```
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C *Q(1): Term A15, p. 21, and Eq. 67, 75, S.P. 98
C
  18 E = CON-V-3.*PM+2.*XI+PL-90.+ISHIFT*180.
   F = SAW*(C5AW**2)*(C5AI**4)/(SUI*(C5UI**2))
   GO TO 800
\mathbf{C}
C *2Q(1): Term A17, p. 21, and Eq. 67, 75, S.P. 98
  19 E = CON-V-4.*PM+2.*XI+2.*PL-90.+ISHIFT*180.
   F = SAW*(C5AW**2)*(C5AI**4)/(SUI*(C5UI**2))
   GO TO 800
C
C *Rho(1): Term A18, p. 21, and Eq. 67, 75, S.P. 98
 20 E = CON-V-3.*PM+2.*XI-PL+2.*SL-90.+ISHIFT*180.
   F = SAW*(C5AW**2)*(C5AI**4)/(SUI*(C5UI**2))
   GO TO 800
C
C M(4): para. 139, p. 47, S.P. 98
\mathbf{C}
 21 E = 4.*(CON-PM+XI-V)
   F = ((C5AW**4)*(C5AI**4)/(C5UI**4))**2
   GO TO 800
\mathbf{C}
C M(6): para. 139, p. 47, S.P. 98
 22 E = 6.*(CON-PM+XI-V)
   F = ((C5AW^{**4})^*(C5AI^{**4})/(C5UI^{**4}))^{**3}
   GO TO 800
\mathbf{C}
C M(8): para. 139, p. 47, S.P. 98
 23 E = 8.*(CON-PM+XI-V)
   F = ((C5AW**4)*(C5AI**4)/(C5UI**4))**4
   GO TO 800
C
C S(4): para. 139, p. 47, S.P. 98
 24 E = 4.*TML
   F = 1.
   GO TO 800
\mathbf{C}
```

```
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C S(6): para. 139, p. 47, S.P. 98
\mathbf{C}
  25 E = 6.*TML
   F = 1.
   GO TO 800
\mathbf{C}
C *M(3): Term A82, p. 35, and para. 106, p. 35, S.P. 98
 26 E = 3.*(CON-PM+XI-V)+180.+ISHIFT*180.
   F = (C5AW**6)*(C5AI**6)/(C5UI**6)
   GO TO 800
C
C *S(1): para. 119, p. 40, S.P. 98
 27 E = TML + 180. + ISHIFT*180.
   F = 1.
   GO TO 800
\mathbf{C}
C *MK(3): p. 68, A.M., M(2)+K(1) interaction
\mathbf{C}
 28 E = 2.*(CON-PM+XI-V)+(CON-VP+90.)+ISHIFT*180.
   F = ((C5AW**4)*(C5AI**4)/(C5UI**4))
   F = F/SQRT(0.8965*(SIN(2.*UI)**2)+0.6001*SIN(2.*UI)*COS(U)
   &
                                    +0.1006)
   GO TO 800
\mathbf{C}
C *2MK(3): p. 68, A.M., assume M(4)-K(1) interaction, sign is unsure
 29 E = 4.*(CON-PM+XI-V)-(CON-VP+90.)+ISHIFT*180.
   F = ((C5AW^{**}4)^*(C5AI^{**}4)/(C5UI^{**}4))^{**}2
   F = F/SQRT(0.8965*(SIN(2.*UI)**2)+0.6001*SIN(2.*UI)*COS(U)
   &
                                    +0.1006)
   GO TO 800
\mathbf{C}
C MN(4): p. 68, A.M., M(2)+N(2) interaction
 30 E = 4.*(CON+XI-V)+PL-5.*PM
   F = ((C5AW**4)*(C5AI**4)/(C5UI**4))**2
   GO TO 800
\mathbf{C}
C MS(4): p. 67, A.M., M(2)+S(2) interaction
\mathbf{C}
  31 E = 2.*(CON-PM+XI-V)+2.*TML
```

```
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   F = (C5AW**4)*(C5AI**4)/(C5UI**4)
   GO TO 800
C
C 2SM(2): p. 68, A.M., 2M(2)-S(2) interaction
 32 E = 4.*TML-2.*(CON-PM+XI-V)
   F = (C5AW**4)*(C5AI**4)/(C5UI**4)
   GO TO 800
\mathbf{C}
C Mf: Term A6, p. 21, and Eq. 66, 74, S.P. 98
 33 E = 2.*(PM-XI)
   F = (SAW^{**}2)*(C5AI^{**}4)/(SUI^{**}2)
   GO TO 800
\mathbf{C}
C MSf: Term A5, p. 21, and Eq. 65, 73, S.P. 98; p. 67, A.M.,
     S(2)-M(2) interaction
C
     Note: F was redefined to be consistent with S.P. 98,
C
         old F is commented out...
 34 E = 2.*TML-2.*(CON-PM+XI-V)
C F = (C5AW^{**}4)^*(C5AI^{**}4)/(C5UI^{**}4)
   F = ((2./3.-(SAW**2))*(1.-1.5*(SAI**2)))/(2./3.-(SUI**2))
   GO TO 800
C Mm: Term A2, p. 21, and Eq. 65, 73, S.P. 98
\mathbf{C}
 35 E = PM-PL
   F = ((2./3.-(SAW**2))*(1.-1.5*(SAI**2)))/(2./3.-(SUI**2))
   GO TO 800
\mathbf{C}
C Sa: para. 119, p. 40, S.P. 98
 36 E = SL
   F = 1.
   GO TO 800
C
C Ssa: Term B6, p. 39, S.P. 98
 37 E = 2.*SL
   F = 1.
   GO TO 800
```

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```
C END OF CONSTITUENT CALCULATION CODES
\mathbf{C}
 800 CONTINUE
\mathbf{C}
   IF(ITYPE .EQ. 2) THEN
     E = E + FLOAT(MS(IPICK))*GONL-SPD(IPICK)*TM/15.
   ELSE IF(ITYPE .EQ. 1) THEN
     ZAT = E/360.
                                !# OF CIRCLES
     IF(ZAT.LT.0.) THEN
      E = (ZAT-AINT(ZAT)+1.)*360.
                                        !360 + MOD(E,360)
     ELSE
      E = (ZAT-AINT(ZAT))*360.
                                       !MOD(E,360)
     END IF
   END IF
\mathbf{C}
   F = 1./F
\mathbf{C}
   RETURN
   END
C
\mathbf{C}
C
   SUBROUTINE TABLE6(VI,V,XI,VP,VPP,CIG,CVX,CEX,PVC,
  &
                          PVCP,ANG,AN,AT)
\mathbf{C}
C Using as input the longitude of the moon's node N (here
C called ANG) and the astronomical constants w, i, e, e1
C and S' (here called AW, AI, AE, AE1 and ASP, respectively,
C and input through the common block BOXS) from the beginning
C of subroutine ASTRO, computes the 5 astronomical entities
C listed in Table 6 of S.P. 98 and used for tidal computations,
C specifically:
C
\mathbf{C}
   VI = inclination of moon's orbit to celestial equator (I)
\mathbf{C}
      in degrees to nearest 0.01 deg
\mathbf{C}
    V = right ascension or longitude in celestial equator of
\mathbf{C}
      moon's orbit (nu) in degrees to nearest 0.01 deg
\mathbf{C}
   XI = longitude in moon's orbit of the lunar intersection
C
      (zi) to nearest 0.01 deg
  VP = nu' of Eq. 224 in S.P. 98 in degrees
C VPP = 2*nu" of Eq. 232 in S.P. 98 in degrees
```

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C
C Also returned are:
C
C CIG = VI(I) in radians
C CVX = V (nu) in radians
C CEX = XI(zi) in radians
C PVC = VP (nu') in radians
C PVCP = VPP (2*nu") in radians
C \quad AN = ANG(N) in radians
C AT = CIG identically, i.e., redundancy
\mathbf{C}
   COMMON /BOXS/ AW, AI, AE, AE1, ASP
C
   PI = 4.*ATAN(1.)
                             !YOU KNOW, PI
   DTR = 180./PI
                            !DEG./RAD., 57.295780
   RTD = PI/180.
                            !RAD./DEG., 0.0174533
\mathbf{C}
C Initialize returned values
   V = 0.0
                !nu, right ascension
   XI = 0.0
                !zi, long. in moon's orbit of intersection
   VP = 0.0
                !nu' of Eq. 224
                 !2*nu" of Eq. 232
   VPP = 0.0
   AN = ANG*RTD !N in radians
C
   AX = ANG
                                     !redundant replacement
   EYE = COS(AI)*COS(AW)-SIN(AI)*SIN(AW)*COS(AN) !cos(I)
   C9 = DTR*ACOS(EYE)
                                           !I in deg
   VI = FLOAT(IFIX(C9*100.+0.5))*0.01
                                                !I to nearest 0.01 deg
   CIG = RTD*VI
                                      !VI in radians
   AT = CIG
                                   !redundant CIG
C
C Special condition checks
   IF( CIG .EQ. 0. ) GO TO 230
   IF( AX .EQ. 0.) GO TO 230
   IF( AX .EQ. 180. ) GO TO 230
\mathbf{C}
C Spherical trigonometry to get nu (Fig. 1, Pg. 6, S.P. 98)
   VXXE = SIN(AI)*SIN(AN)
   VXXN = COS(AI)*SIN(AW)+SIN(AI)*COS(AW)*COS(AN)
   IF( VXXE .EQ. 0. ) GO TO 201
```

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   IF( VXXN .EQ. 0. ) GO TO 201
   VXX = VXXE/VXXN
                                         !tan(nu)
   C10 = DTR*ATAN(VXX)
                                           !nu in deg.
   V = FLOAT(IFIX(C10*100.+0.5))*0.01
                                              !nu to 0.01 deg
   IF( V .GT. 0. .AND. AX .GT. 180. ) V = -V !- for 180 < N < 360
\mathbf{C}
 201 CONTINUE
C
                                     !nu in radians
   CVX = RTD*V
C
C Spherical trigonometry to get zi (Fig. 1, Pg. 6, S.P. 98)
   TERM = SIN(AI)*COS(AW)/SIN(AW)
   EXX = TERM*SIN(AN)/COS(AN) + (COS(AI)-1.)*SIN(AN)
   EZZ = TERM + COS(AI) * COS(AN) + (SIN(AN) * * 2)/COS(AN)
   IF( EXX .EQ. 0. ) GO TO 202
   IF( EZZ .EQ. 0. ) GO TO 202
   EXEZ = EXX/EZZ
                                       !tan(zi)
   IF( EXEZ .GT. 3450. ) GO TO 202
   C11 = DTR*ATAN(EXEZ)
                                           !zi in deg.
   XI = FLOAT(IFIX(C11*100.+0.5))*0.01
                                              !zi to 0.01 deg
   IF( XI .GT. 0. .AND. AX .GT. 180.) XI = -XI !- if 180 < N < 360
\mathbf{C}
 202 CONTINUE
                                     !zi in radians
   CEX = RTD*XI
C
C From paragraph 133 of S.P. 98 to find nu':
   A22 = (0.5+0.75*(AE**2))*SIN(2.*CIG)
                                               !lunar coeff. A of Eq. 216
   B22 = (0.5+0.75*(AE1**2))*ASP*SIN(2.*AW)
                                                  !solar coeff. B of Eq. 217
   VPXE = A22*SIN(CVX)
                                          !numerator in Eq. 224
   VPXN = A22*COS(CVX)+B22
                                             !denominator in Eq. 224
   IF( VPXE .EQ. 0. ) GO TO 203
   IF( VPXN .EQ. 0. ) GO TO 203
   VPX = VPXE/VPXN
                                        !tan(nu')
   IF( VPX .GT. 3450. ) GO TO 203
   VP = DTR*ATAN(VPX)
                                          !nu' in degrees
   IF( VP .GT. 0. .AND. AX .GT. 180.) VP = -VP !- if 180 < N < 360
\mathbf{C}
 203 CONTINUE
\mathbf{C}
   PVC = RTD*VP
                                      !nu' in radians
```

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C
C From paragraph 135 in S.P. 98 for 2*nu":
   A47 = (0.5+0.75*(AE**2))*(SIN(CIG)**2)
                                           !lunar coeff. A of Eq. 218
   B47 = (0.5+0.75*(AE1**2))*ASP*(SIN(AW)**2)!solar coeff. B of Eq. 219
   VPYE = A47*SIN(2.*CVX)
                                      !numerator in Eq. 232
   VPYN = A47*COS(2.*CVX)+B47
                                          !denominator in Eq. 232
   IF( VPYE .EQ. 0. ) GO TO 204
   IF( VPYN .EQ. 0. ) GO TO 204
   VPY = VPYE/VPYN
                                    !tan(2*nu")
   IF( VPY .GT. 3450. ) GO TO 204
   VPP = DTR*ATAN(VPY)
                                      !2*nu" in degrees
  IF( VPP .GT. 0. .AND. AX .GT. 180.) VPP=-VPP !- if 180 < N < 360
C
 204 CONTINUE
C
   PVCP = RTD*VPP
                                   !2*nu" in radians
C
C Special condition jump address
\mathbf{C}
 230 CONTINUE
C
   RETURN
  END
\mathbf{C}
C
C
\mathbf{C}
   SUBROUTINE NAME(SPDD,ITAG,ISUB,INUM,ICODE)
C ROUTINE IDENTIFIES A CONSTITUENT BY ITS SPEED
C (ICODE=1), ITS NAME LABEL (ICODE=2), OR ITS CONSTITU- *
C ENT NUMBER (ICODE=3), AND MAKES IT AVAILABLE FOR
C LABELING. IT ALSO DETERMINES THE SUBSCRIPT OF THE
C CONSTITUENT. THE ORDER OF THE CONSTITUENT SPEEDS IS: *
\mathbf{C}
C M(2) N(2) S(2) O(1)
                            K(1)
                                  K(2)
C L(2) 2N(2) R(2) T(2) LAMBDA(2) MU(2)
C NU(2) J(1) M(1) OO(1)
                             P(1)
                                   Q(1)
C 2Q(1) RHO(1) M(4) M(6)
                               M(8)
                                     S(4)
C S(6) M(3) S(1) MK(3)
                            2MK(3)
                                    MN(4)
C MS(4) 2SM(2) MF
                      MSF
                               MM
                                      SA
                                *
C SSA
```

```
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LABLE(37), ITAG
   CHARACTER*10
   DIMENSION
                   IP(37)
   DOUBLE PRECISION SPD(37), SPDD
\mathbf{C}
   COMMON /MMSS/ IP
   COMMON /SPEEDS/ SPD
   COMMON /NAMES/ LABLE
\mathbf{C}
  1 FORMAT(10X, 'Speed constituent ',F12.7,' not in list')
  2 FORMAT(10X,'Constituent named ',A10,' not in list')
  3 FORMAT(10X,'Constituent number ',I4,' not in list')
\mathbf{C}
                              !SEARCH BY SPEED
   IF(ICODE.EQ.1) THEN
    DO 100 J=1,37
      IF(SPDD.NE.SPD(J)) THEN
       GO TO 100
      ELSE
       ITAG=LABLE(J)
       ISUB=IP(J)
       INUM=J
       GO TO 400
     END IF
 100 CONTINUE
    WRITE(*,1) SPDD
C
                                 !SEARCH BY NAME
  ELSE IF(ICODE.EQ.2) THEN
    DO 200 I=1,37
      IF(ITAG.NE.LABLE(I)) THEN
       GO TO 200
      ELSE
       SPDD=SPD(I)
       ISUB=IP(I)
       INUM=I
       GO TO 400
      END IF
 200 CONTINUE
    WRITE(*,2) ITAG
\mathbf{C}
  ELSE IF(ICODE.EQ.3) THEN
                                 !SEARCH BY NUMBER
    K=INUM
    IF(K.GT.0 .AND. K.LE.37) THEN
      ITAG=LABLE(K)
```

```
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     SPDD=SPD(K)
     ISUB=IP(K)
     GO TO 400
    END IF
    WRITE(*,3) INUM
  END IF
C
  WRITE(*,'("****EXECUTION STOPPED IN SUBROUTINE NAME****")')
  STOP
C
400 CONTINUE
\mathbf{C}
  RETURN
  END
\mathbf{C}
C
C
  BLOCK DATA CONSTS
C 37 CONSTITUENT ELEMENTS IS NOAA STANDARD AS OF PEARL *
C HARBOR DAY 1987
CHARACTER*10 LABLE(37)
                MS(37)
  INTEGER
  DOUBLE PRECISION SPD(37)
C
  COMMON /SPEEDS/ SPD
  COMMON /NAMES/ LABLE
  COMMON /MMSS/ MS
\mathbf{C}
C Tidal constituent speeds in degrees per hour
  DATA
            SPD/
  & 28.9841042D0, 28.4397295D0, 30.0000000D0, 13.9430356D0,
  & 15.0410686D0, 30.0821373D0, 29.5284789D0, 27.8953548D0,
  & 30.0410667D0, 29.9589333D0, 29.4556253D0, 27.9682084D0,
  & 28.5125831D0, 15.5854433D0, 14.4966939D0, 16.1391017D0,
  & 14.9589314D0, 13.3986609D0, 12.8542862D0, 13.4715145D0,
  & 57.9682084D0, 86.9523127D0, 115.9364169D0, 60.0000000D0,
    90.000000D0, 43.4761563D0, 15.000000D0, 44.0251729D0,
  & 42.9271398D0, 57.4238337D0, 58.9841042D0, 31.0158958D0,
     1.0980331D0, 1.0158958D0, 0.5443747D0, 0.0410686D0,
```

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  &
       0.0821373D0/
C
C Constituent subscripts
              MS/
   DATA
            2,
                     2,
  &
                             2,
                                      1,
                             2,
                                      2,
  &
            1,
                     2,
  &
            2,
                     2,
                             2,
                                      2,
            2,
  &
                     1.
                             1,
                                      1,
  &
            1,
                             1,
                                      1,
                     1,
  &
            4,
                     6,
                             8,
                                      4,
  &
            6,
                     3,
                             1,
                                      3,
  &
            3,
                     4,
                             4,
                                      2,
  &
            0,
                     0,
                             0,
                                      0,
  &
            0/
C
C Constituent names
   DATA
            LABLE/
  & 'M(2)
              ', 'N(2)
                                 ', 'O(1)
                       ', 'S(2)
                       ', 'L(2)
                                ', '2N(2)
  & 'K(1)
             ', 'K(2)
             ', T(2)
                      ', 'Lambda(2)', 'Mu(2)
  & 'R(2)
                       ', 'M(1)
  & 'Nu(2)
             ', 'J(1)
                                 ', 'OO(1)
             ', 'Q(1)
  & 'P(1)
                      ', '2Q(1)
                                ', 'Rho(1)
  & 'M(4)
             ', 'M(6)
                       ', 'M(8)
                                  ', 'S(4)
             ', 'M(3)
                       ', 'S(1)
  & 'S(6)
                                ', 'MK(3)
  & '2MK(3) ', 'MN(4) ', 'MS(4) ', '2SM(2)
             ', 'Msf
                                 ' , 'Sa
  & 'Mf
                      ', 'Mm
  & 'Ssa
             '/
\mathbf{C}
   END
INTEGER FUNCTION LSTGDCHR(NAMESUB)
c
   CHARACTER*(*)
                      NAMESUB
   INTEGER
             II,CLEN,INDEX
c
   CLEN = LEN(NAMESUB)
   DO 100 \text{ II} = \text{CLEN}, 1, -1
100 IF( ICHAR(NAMESUB(II:II)) .GT. 60 .AND.
      ICHAR(NAMESUB(II:II)) .LT. 127 )GOTO 101
101 INDEX=II
```

c

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 $\begin{aligned} & LSTGDCHR = INDEX \\ & RETURN \end{aligned}$

c

END

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APPENDIX C

CODES FOR THE OCEANRDS REFRACTION/DIFFRACTION MODEL

Appendix C: Codes for the OCEANRDS Refraction/Diffraction Model

There is a family of OCEANRDS codes, with each specific to particular grid domains. For the back refraction problems and other far field applications the **oceanrds_socal.for** version is used on a 2,405 x 4,644 raster formatted grid that encompasses the entire Southern California Bight. The input parameters output files which are required by **oceanrds_socal.for** are

graham_m.grd **	(name.grd) bathymetry input file
-1.0*	(gis) if data is parsed GIS data gis= -1.0, if NOS data gis=1.0
1*	wave exposure 1=west, 2=north, 3=east, 4=south (icoast)
0.0*	sea level adjustment MSL meters (sealev) (+ = deeper water)
77.5*	outer grid dimensions in meters perpendicular to coast (sx)
92.6*	outer grid dimensions in meters parallel to coast (sy)
4644*	number of grid cells in from deep water perpendicular to coast raster
	(nx)
2405*	number of grid cells along coast from top edge (ny)
17.0*	wave period in seconds (persw)
270.0*	wave direction degress clockwise from true north (asw)
10.0*	wave height meters (hsw)

```
the Neighborhood of ASBS 29 in La Jolla, California
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c
    Ocean refraction - diffraction module oceanrds_socal.for
c
C
  programmed to run on full socal elevation data (graham_m.grd)
c
  extracted from GIS grid by read graham bathy full.for
c
              10 oct, 2002
c
c
This program is the first program in a 2 program series to treat
 the distribution of wave heights, angles, wave numbers from the
 combined effect of a wave field containing two distinct periods
 and/or directions. The second program (windwave.f) must also be run
c in order for the ultimate output file to be properly name for use
c by the other modules even if there is 0 energy in the second band.
c It solves a parabolic approximation to the mild slope equation
c for the transmitted field of a linear wave. Outputs wave height,
c wave number, and wave angle for each grid point in a 2405 x 4644
c grid array with 3 second x 3 second spacing. Bathymetry is read from
c a formatted 2405 x 4644 real number array called 'ifile'.grd,
c created by the oceanbat.f module, where ifile corresponds to
c the inputted site name. Program uses "oceanrd.inp" input file.
c All output files are named 'ifile' with different extensions,
c ie. 'ifile'.wh1.
c
      parameter (max=10000000)
    character name*8,ifile*12
    character ofile1*12.ofile2*12.ofile3*12.ofile4*12
   character ofile5*12,ofile6*12,ofile7*12
   dimension ccg(max),di(max),hab(max),rlb(max),dib(max),ih(max)
   dimension depth(4644,4644),wht(4644,4644)
   dimension ang(4644,4644)
   dimension depthold(4644,4644)
    real kbar(2,max),kave
c
    complex aa(max),bb(max),cc(max),dd(max),uu(max),aprev(max)
    complex c3,t1,t2,t3,f,alast(max),aphys(max),mim,mip
c
    common /grid/ ny,sy,sx,dely,delx,ndely,freq,dcon,ify,ifx,tide
       common /cut/ dc
```

c

```
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     pi=acos(-1.)
c
   open parameter input file, return error message if missing
c
c
c
c
     open(2,file='oceanrds_socal.inp',status='old',err=900)
     read(2,'(a)') name
        read(2,*) gis
     read(2,*) icoast
        read(2,*) tide_elev
        read(2,*) sx
     read(2,*) sy
        read(2,*) nx
     read(2,*) ny
        read(2,*) persw
       read(2,*) asw
       read(2,*) hsw
c
     tide=tide_elev
     amp=hsw
     dir=asw
     per=persw
c
        freq=1/per
c... change to proper rotation frame, flag if theta not between +/- 45
c
     IF (icoast .EQ. 1) coast=270.0
     IF (icoast .EQ. 2) coast=90.0
     IF (icoast .EQ. 3) coast=0.0
     IF (icoast .EQ. 4) coast=180.0
c
        theta=coast-dir
c... set some variables to constant values
c
      dcon = 1. for nuwave version
c
     dcon=1.
c
      cutoff depth (dc) -5.0 meters
     dc = -5.0
c
```

```
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      breaking wave switch (ibreak) (1=on, 0=off)
c
     ibreak=1
c
      breaking criteria (bc) 0.5 for wave height = (bc)*depth
c
     bc = 0.5
c
      lateral b.c.: ibc=(0) transmitting, ibc=(1) reflective
c
      if trans: (isn=0) straight snell, isn=(1) kirby's improved
c
     isn=1
      idf=(0) small angle diff, idf=(1) large angle dif
c
     idf=1
c
c
       write(*,'(5(/),10x,a)')
   &' KIRBY HIGHER ORDER REFRACTION-DIFFRACTION PROGRAM'
       write(*,'(/,10x,a)')
   &'- based on the parabolic equation method (PEM) of solving'
       write(*,'(10x,a,5(/))')
   &' the mild-slope equation. '
c
       nn=8
       do 3 \text{ m}=8,1,-1
3
       if(name(m:m).eq.'') nn=m-1
c
c
c... open grid file
c
       write(ifile, '(a,a)') name(:nn), '.grd'
     open(20,file=ifile,status='old',err=900)
c
c... open breaker ix,iy,wave height, wave angle, depth file
     write(ofile7,'(a,a)') name(:nn),'.bra'
     open(39,file=ofile7)
c
c... open sea level corrected ascii bathymetry file
     write(ofile1,'(a,a)') name(:nn),'.dep'
     open(31,file=ofile1)
c
c
```

c... open ascii wave number file

```
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     write(ofile2,'(a,a)') name(:nn),'.wvn'
     open(32,file=ofile2)
c
c
c
c... open ascii wave height file
    write(ofile3,'(a,a)') name(:nn),'.wh1'
    open(33,file=ofile3)
c
c
c
c... open ascii wave angle file
     write(ofile4,'(a,a)') name(:nn),'.an1'
     open(34,file=ofile4)
c
c
c
c.... create depth array
     do 111 j=ny,1,-1
     read(20,*) (depthold(j,i),i=1,nx)
      continue
111
     rewind(20)
c 10oct02 *******correct for sea level and change sign if GIS data
     do 711 i=1,nx
     do 811 i=1,ny
     depthold(j,i)=(gis*depthold(j,i))+sealev
811
      continue
711
       continue
c.....write rotated ascii depth file for internal use in ref/dif
      format(2405f12.2)
     DO 101 i=1,nx
     WRITE(31,165)(depthold(j,i),j=1,ny)
101 CONTINUE
     rewind(31)
c
c
     do 119 i=1,nx
     read(31,*) (depth(i,j),j=1,ny)
119
      continue
     rewind(31)
```

```
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c
c
c... read first depth to initialize offshore boundary
       read(31,*) (di(m),m=1,ny)
       ridep=di(ny/2)
       rewind(31)
c.... would like a grid step size on the order of 1/5 wavelength
       call getkcg(10.,wk,cg0)
       wl=2*pi/wk
       ifx=1+sx/(.2*w1)
       ify=1+sy/(.2*w1)
c
       nmx=ifx
       nmy=ify
c
     amp=amp/2.
c... calculate dimensions of interpolated grid
       dely=sy/ify
       delx=sx/ifx
     ndely=(ny-1)*ify
     ndelx=(nx-1)*ifx
c
c
     call getkcg(ridep,wk0,cg0)
c wave length (m)
     wl=2*pi/wk0
c wave frequency (rad/sec)
     sig=2*pi/per
c radder's correction factor
     do 33 j=1, ndely+1
33
      ccg(j) = sqrt(sig*cg0/wk0)
c
c
        ltype=2
c... open binary wave height file
     write(ofile5,'(a,a)') name(:nn),'.bw1'
     open(9,file=ofile5,form='unformatted')
c
```

```
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c... open binary wave angle file
     write(ofile6,'(a,a)') name(:nn),'.ba1'
     open(11,file=ofile6,form='unformatted')
c.. initialize depth array
       do 555 \text{ nn}=1, \text{ndely}+1
       hab(nn)=0.
       ih(nn)=0
555
       continue
c enter initial condition at x=0
c
     call inbc(amp,wk0,theta,ndely,dely,alast)
c
     do 202 j=1,ndely+1
     kbar(1,j)=wk
202
       kbar(2,j)=wk
c scale alast as in radder(1978)
     do 32 j=1,ndely+1
     alast(j)=alast(j)*ccg(j)
      continue
32
c
c
     call wwave(alast,ndely,nmy,dely,kbar)
c
   solution of the parabolic eqn. by the crank-nicholson formulation
c
c
   start x increments
c
c
     c2=1./2./dely**2
     c3=2.*(0,1)/delx
c
c increments in x-direction
c
     ikount=0
     do 100 l=1, ndelx
c... write every 10th step to the screen
       if(10*((l+1)/10).eq.l+1) then
     print *,' column ',l+1,' of ',ndelx+1
       endif
```

```
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c
    lm=l
c
     call intkcg(lm,kbar,ccg,di,ikount)
c correction factor
     do 34 j=1,ndely+1
     ccg(j)=sqrt(sig*ccg(j)/kbar(2,j))
34
     continue
C-----
c increments in y-direction - 1st. round
     do 200 j=2,ndely
c
     kave=(kbar(2,j)+kbar(1,j))/2.
     kave=kbar(2,j)
c
c
     if(idf.eq.0) then
c small angle diffraction approximation
    t1=c2
    t3=c3*kave
    f=2*kave*(kave-wk)+c3/2*(kbar(2,j)-kbar(1,j))
c
     aa(j-1)=t1
     bb(j-1)=t3-2.*t1+f/2.
     cc(i-1)=t1
     dd(j-1)=-t1*alast(j+1)+(t3+2*t1-f/2)*alast(j)-t1*alast(j-1)
c
     endif
c
    if(idf.eq.1) then
c large angle diffraction approximation
     t1=c2/2.*(3.-wk/kave)
    t2=c2*c3/4./kave
     t3=c3*kave
    f=2*kave*(kave-wk)+c3/2*(kbar(2,j)-kbar(1,j))
c
     aa(j-1)=t1+t2
     bb(j-1)=t3-2.*(t1+t2)+f/2.
     cc(j-1)=t1+t2
     dd(j-1)=(t2-t1)*(alast(j+1)+alast(j-1))+(t3-2*(t2-t1)-f/2)
   & *alast(j)
```

```
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     endif
c
     if(j.eq.2) then
     if(ibc.eq.1) cc(1)=aa(1)+cc(1)
c reflective b.c.: a1=a3
     if(ibc.eq.1) bb(1)=aa(1)+bb(1)
c reflective b.c. : a1=a2
     if(ibc.eq.0) then
c transmitting b.c.
     if(isn.eq.0) then
c straight snell
     mip = (0.,1.)*wk*sin(theta)/2.
     mip=(1./dely-mip)/(1./dely+mip)
     else
c kirby's improved
     mip=(alast(2)-alast(1))/(alast(2)+alast(1))
     mip=(1.-mip)/(1.+mip)
     endif
c
     bb(1)=bb(1)+mip*aa(1)
     endif
     aa(j-1)=(0.,0.)
c
     endif
c
     if(j.eq.ndely) then
     if(ibc.eq.1) aa(j-1)=aa(j-1)+cc(j-1)
c reflective b.c.: an=an-2
     if(ibc.eq.1) bb(j-1)=bb(j-1)+cc(j-1)
c reflective b.c.: an=an-1
     if(ibc.eq.0) then
c transmitting b.c.
     if(isn.eq.0) then
c straight snell
     mim = (0.,1.)*wk*sin(theta)/2.
     mim=(1./dely+mim)/(1./dely-mim)
     else
c kirby's improved
     mim = (alast(j+1)-alast(j))/(alast(j+1)+alast(j))
     mim=(1.+mim)/(1.-mim)
     endif
```

```
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c
     bb(j-1)=bb(j-1)+mim*cc(j-1)
     endif
     cc(j-1)=(0.,0.)
     endif
c
200
     continue
c
            ***** compute the solution *****
c
c
     neqs=ndely-1
c
     call tridag(aa,bb,cc,dd,uu,neqs)
c
     do 143 j=2, ndely
      alast(j)=uu(j-1)
c load alast(j)
c
     if(ibc.eq.1) then
     alast(1)=alast(3)
c reflective b.c.: a1=a3
     alast(ndely+1)=alast(ndely-1)
c
           : an=an-2
     alast(1)=alast(2)
c reflective b.c.: a1=a2
     alast(ndely+1)=alast(ndely)
c
           : an=an-1
     endif
c
     if(ibc.eq.0) then
c transmitting b.c.
     alast(1) = mip*alast(2)
     alast(ndely+1)= mim*alast(ndely)
c
     endif
c transform back into phys. height
     do 37 j=1,ndely+1
      aphys(j)=2.*alast(j)/ccg(j)
37
c
         ******
                      check for breaking *******
```

```
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c
       if(ibreak.eq.1) then
    do 54 j=1,ndely+1
    hb=bc*di(j)
    rat=hb/cabs(aphys(j))
        if(di(i).lt.0) rat=0.
    if(rat.lt.1) then
c save point just before wave first breaks
         if(ih(j).eq.0) then
               hab(j)=cabs(aprev(j))
               rlb(j)=(lm-1)/real(ifx)
c calculate direction before breaking
                 if(j.eq.ndely+1.or.ih(j+1).eq.1) then
       xx = aimag((aprev(j)-aprev(j-1))/(dely*kbar(1,j)*aprev(j)))
                  if(xx.gt.1) xx=1.
               if(xx.lt.-1.) xx=-1.
                  dib(j)=asin(xx)
                 else
       xx=aimag((aprev(j+1)-aprev(j))/(dely*kbar(1,j)*aprev(j)))
                  if(xx.gt.1) xx=1.
                  if(xx.lt.-1.) xx=-1.
               dib(i)=asin(xx)
              endif
          dib(j)=270.-dib(j)*57.296-rot
                 if(dib(j).lt.0) dib(j)=360+dib(j)
         ih(i)=1
         endif
    aphys(j)=rat*aphys(j)
    alast(j)=rat*alast(j)
       endif
54
       continue
    endif
c
  *************************
c
c... writing wave field
       if(nmx*(lm/nmx).eq.lm.or.lm.eq.1) then
      call wwave(aphys,ndely,nmy,dely,kbar)
       endif
c
       do 55 nn=1,ndely+1
       aprev(nn)=aphys(nn)
55
       continue
```

```
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c
100
      continue
c... output prebreak heights and directions
       open(24,file='break.dat')
       write(24,'(/,18x,a)') '
                                 REFRACTION - DIFFRACTION
       write(24,'(/,18x,a)') ' WAVE HEIGHT AND DIRECTION BEFORE BREAKING'
       write(24,'(/,15x,a,a,a,f5.1,a,f4.1)')
  & 'site: ',name,' direction: ',dir,' period: ',per
       write(24, (/,20x,a,6x,a,6x,a,9x,a,//)')
   &'row',' H (m) ','col','dir'
       m=1
       write(24,'(18x,i4,f12.2,f12.1,f12.1)') m,hab(m),rlb(m),dib(m)
     alphab=dib(m)-shor
    irlb=NINT(rlb(m))+1
    breakd = (5.0/4.0)*hab(m)
***rasterisze break point file m....ixbra
     ixbra=2405-m
     write(39,'(18x,i4,i4,f12.3,f12.1,f12.3)') ixbra,irlb,hab(m),
   &alphab,breakd
       if(nmy.eq.1) then
       is=2
       else
       is=nmy
       endif
       mm=1
       do 888 m=is,ndely,nmy
       mm=mm+1
       write(24,'(18x,i4,f12.2,f12.1,f12.1)') mm,hab(m),rlb(m),dib(m)
     alphab=dib(m)-shor
     irlb=NINT(rlb(m))+1
    breakd = (5.0/4.0)*hab(m)
    ixbra=2405-mm
     write(39,'(18x,i4,i4,f12.3,f12.1,f12.3)')ixbra,irlb,hab(m),
   &alphab,breakd
888
       continue
c
c
    rewind (9)
    rewind (11)
c
c.....create wave height and wave angle arrays
     do 114 i=1,nx
```

```
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         read(9) (wht(i,j),j=1,ny)
         read(11) (ang(i,j),j=1,ny)
114
       continue
c.... write array values to:
c.... ascii depth, wave number, wave angle, wave height grid files
c
c
c2002
         format(4644f12.2)
cccc Comment out old way of writing arrays
                do 134 i=1,nx
cccccccc
cccccccc
                write(33,2002) (wht(i,j),j=1,ny)
                write(34,2002) (ang(i,j),j=1,ny)
cccccccc
cccccccc134
                  continue
CCCCC Raster way of writing arrays!!!!!!!!!!
    DO 176 j=ny,1,-1
     write(33,6666) (wht(i,j),i=1,nx)
     write(34,6666) (ang(i,j),i=1,nx)
 176 CONTINUE
6666
      format(4644f12.2)
6667
        format(4644f12.6)
c
c
       go to 901
900
       write(*,'(20x,a,a,a)') '**** error **** ',ifile,' missing'
       write(*,'(20x,a)') '
                           press any key to continue '
       read(*,'(a)') idum
901
      continue
     stop
    end
c
c
     subroutine tridag(a,b,c,r,u,n)
c
     parameter (nmax=250002)
     complex gam(nmax),a(n),b(n),c(n),r(n),u(n),bet
c
     bet=b(1)
     u(1)=r(1)/bet
     do 11 j=2,n
```

```
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     gam(j)=c(j-1)/bet
     bet=b(j)-a(j)*gam(j)
     if(bet.eq.0)pause
     u(j)=(r(j)-a(j)*u(j-1))/bet
11
     continue
     do 12 j=n-1,1,-1
     u(j)=u(j)-gam(j+1)*u(j+1)
      continue
12
     return
     end
c
c
     subroutine wwave(aphys,ndely,nmy,dely,kbar)
c
c compute & write wave height and direction, initial breaking point
    imax= max grid size nmax=max no. of steps
c
c
     parameter (nmax=45000000,imax=4500000)
       dimension amod(nmax),alfa(nmax)
       real kbar(2,nmax)
       complex aphys(nmax)
c
       nky=0
     do 350 j=1,ndely+1
       if(nmy*(j/nmy).eq.j.or.j.eq.1) then
       nky=nky+1
c
        amod(nky)=cabs(aphys(j))
        if(amod(nky).gt..05) then
        if(j.eq.ndely+1) then
        xx=aimag((aphys(j)-aphys(j-1))/(dely*kbar(2,j)*aphys(j)))
        if(xx.gt.1) xx=1.
        if(xx.lt.-1.) xx=-1.
        alfa(nky)=asin(xx)
        else
        xx=aimag((aphys(j+1)-aphys(j))/(dely*kbar(2,j)*aphys(j)))
        if(xx.gt.1) xx=1.
        if(xx.lt.-1.) xx=-1.
        alfa(nky)=asin(xx)
        endif
        else
        alfa(nky)=0
```

```
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       endif
       alfa(nky)=alfa(nky)*57.296
       endif
350
      continue
     write(9) (amod(jj),jj=1,nky)
     write(11) (alfa(jj),jj=1,nky)
c
    return
    end
c
c
    subroutine inbc(amp,wk,theta,ndely,dely,aa)
c
    complex aa(*)
c
    real ky
    pi=acos(-1.)
    theta=theta*pi/180.
    ky=wk*sin(theta)
c
    do 3 j=1,ndely+1
    ar=amp*cos(ky*(j-1)*dely)
    ai=amp*sin(ky*(j-1)*dely)
     ar = (cos(ky*(j-1)*dely) + cos(-ky*(j-1)*dely))/2.
c 2 identical
     ai = (\sin(ky*(j-1)*dely) + \sin(-ky*(j-1)*dely))/2.
c waves summed
    aa(j)=cmplx(ar,ai)
3
     continue
c
    return
    end
c
    subroutine getkcg(d,k,cg)
c
c.... depth (meters) and frequency are input
c.... returns wave number (2pi/l) and group velocity
c
    real k
```

```
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    common /grid/ ny,sy,sx,dely,delx,ndely,f,dcon,ify,ifx,tide
    data tpi/6.2831853/
c
    sig=tpi*f
    a=d*sig*sig/9.81
    if(a.ge.1) then
         yhat=a*(1+1.26*exp((-1.84)*a))
         t=exp((-2)*yhat)
         aka=a*(1+2*t*(1+t))
    else
         aka = sqrt(a)*(1+a/6.*(1+a/5.))
    endif
    k=aka/d
    x=2*k*d
    cg=tpi*(f/k)*.5*(1+x/sinh(x))
    return
    end
c
    subroutine intkcg(lm,kbar,ccg,di,ikount)
c
       parameter(max=45000002)
    real kbar(2,max)
    dimension ccg(max),d(2,max),di(max)
c
    save d
        common /cut/ dc
    common /grid/ ny,sy,sx,dely,delx,ndely,f,dcon,ify,ifx,tide
    data istart,ncol,eps/1,0,.0000001/
c
c... if this is the first call, load first two columns
    go to (1,3) istart
      istart=2
1
c... read in first 2 columns
        do 777 i=1,2
777
         read(31,*)(d(i,j),j=1,ny)
     ncol=1
c... interpolate grid points for first row
    do 2 i=1, ndely
    y=1+real(i-1)/real(ify)+eps
    ym=amod(y,1.)
```

```
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    iy=int(y)
     di(i)=d(1,iy)+ym*(d(1,iy+1)-d(1,iy))+tide
     call getkcg(di(i)*dcon,rk,cg)
     ccg(i)=cg
     kbar(2,i)=rk
2
c
c... figure out which bathymetry grid columns should be used
3
     icol=1+real(lm)/real(ifx)+eps
    if(icol.ne.ncol) then
      do 5 i=1,ny
5
      d(1,i)=d(2,i)
      read(31,*,end=999) (d(2,j),j=1,ny)
999
          ncol=icol
     endif
    dd1=d(1,100)
c... bilinear interpolate depths for new column of k and cg
15
      x=1+real(lm)/real(ifx)+eps
    xm=amod(x,1.)
     do 20 i=1, ndely
     y=1+real(i-1)/real(ify)+eps
     ym=amod(y,1.)
    iy=int(y)
     di(i)=d(1,iy)+xm*(d(2,iy)-d(1,iy))+ym*(d(1,iy+1)-d(1,iy))+
   & xm*ym*(d(2,iy+1)-d(1,iy+1)-d(2,iy)+d(1,iy))+tide
    if(di(i).le.01) then
    di(i) = .01
    endif
c
c... shift k and ccg col 2 to 1 and calculate new 2's
        if(di(i).lt.dc) di(i)=dc
    kbar(1,i)=kbar(2,i)
    call getkcg(di(i)*dcon,rk,cg)
    kbar(2,i)=rk
20
     ccg(i)=cg
    kbar(1,ndely+1)=kbar(1,ndely)
    kbar(2,ndely+1)=kbar(2,ndely)
     ccg(ndely+1)=ccg(ndely)
     di(ndely+1)=di(ndely)
c
```

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> return end

For high resolution local refraction/diffraction calculations within the Torrey Pines Sub-Cell, we use the **oceanrds_tp.for** codes on a 441 x 236 raster formatted grid found in Appendix D. The input parameters output files which are required by **oceanrds_tp.for** are:

*bathymetry input file
*(gis) if water values are negative gis= -1.0, if positive gis=1.0
* wave exposure 1=west, 2=north, 3=east, 4=south (icoast)
* sea level adjustment MSL meters (sealev) (+ = deeper water)
* inner grid dimensions in meters perpendicular to coast (sx)
* inner grid dimensions in meters parallel to coast (sy)
* number of grid cells in from deep water perpendicular to coast raster
(nx)
* number of grid cells along coast from top edge (ny)
* wave period in seconds (persw)
* wave direction degress clockwise from true north (asw)
* wave height meters (hsw)
· ·

```
Hydrodynamic Modeling of Storm Drain Discharges in
the Neighborhood of ASBS 29 in La Jolla, California
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c
    Ocean refraction - diffraction module oceanrds_tp.for
c
C
  programmed to run on gis elevation data (tp_subbot50_grd.txt subbot50.grd)
c
  extracted from GIS grid by gis ascii utm-xyz.for
              14 sep, 2004
c
c
c Bathymetry is read from
c a formatted 441 (row) x 236 (col) real number array called 'ifile'.grd,
c created by gis_ascii_utm-xyz.for, where 'ifile' corresponds to
c the inputted site name. Program uses "oceanrd_tp.inp" input file.
c All output files are named 'ifile' with different extensions,
c ie. 'ifile'.wh1.
parameter (max=10000000)
   character name*8,ifile*12
   character ofile1*12,ofile2*12,ofile3*12,ofile4*12
   character ofile5*12,ofile6*12,ofile7*12
   dimension ccg(max),di(max),hab(max),rlb(max),dib(max),ih(max)
   dimension depth(4644,4644),wht(4644,4644)
   dimension ang(4644,4644)
   dimension depthold(4644,4644)
   real kbar(2,max),kave
c
   complex aa(max),bb(max),cc(max),dd(max),uu(max),aprev(max)
    complex c3,t1,t2,t3,f,alast(max),aphys(max),mim,mip
c
   common /grid/ ny,sy,sx,dely,delx,ndely,freq,dcon,ify,ifx,tide
      common /cut/ dc
c
   pi=acos(-1.)
  open parameter input file, return error message if missing
c
c
c
c
   open(2,file='oceanrds_tp.inp',status='old',err=900)
   read(2,'(a)') name
     read(2,*) gis
```

```
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     read(2,*) icoast
       read(2,*) tide_elev
       read(2,*) sx
     read(2,*) sy
       read(2,*) nx
     read(2,*) ny
       read(2,*) persw
       read(2,*) asw
       read(2,*) hsw
c
     tide=tide_elev
     amp=hsw
     dir=asw
     per=persw
c
       freq=1/per
c
c... change to proper rotation frame, flag if theta not between +/- 45
     IF (icoast .EQ. 1) coast=270.0
     IF (icoast .EQ. 2) coast=90.0
     IF (icoast .EQ. 3) coast=0.0
     IF (icoast .EQ. 4) coast=180.0
c
       theta=coast-dir
c... set some variables to constant values
c
      dcon = 1. for nuwave version
c
     dcon=1.
c
      cutoff depth (dc) -5.0 meters
     dc = -5.0
c
      breaking wave switch (ibreak) (1=on, 0=off)
c
     ibreak=1
c
      breaking criteria (bc) 0.5 for wave height = (bc)*depth
     bc=0.5
c
      lateral b.c.: ibc=(0) transmitting, ibc=(1) reflective
     ibc=0
      if trans: (isn=0) straight snell, isn=(1) kirby's improved
```

c

```
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     isn=1
      idf=(0) small angle diff, idf=(1) large angle dif
     idf=1
c
c
        write(*, '(5(/), 10x, a)')
   &' KIRBY HIGHER ORDER REFRACTION-DIFFRACTION PROGRAM'
        write(*,'(/,10x,a)')
   &'- based on the parabolic equation method (PEM) of solving'
        write(*,'(10x,a,5(/))')
   &' the mild-slope equation. '
c
        nn=8
        do 3 \text{ m}=8,1,-1
       if(name(m:m).eq.' ') nn=m-1
3
c
c
c... open grid file
        write(ifile, '(a,a)') name(:nn), '.grd'
     open(20,file=ifile,status='old',err=900)
c
c
c... open breaker ix,iy,wave height, wave angle, depth file
     write(ofile7,'(a,a)') name(:nn),'.bra'
     open(39,file=ofile7)
c
c... open sea level corrected ascii bathymetry file
     write(ofile1,'(a,a)') name(:nn),'.dep'
     open(31,file=ofile1)
c
c
c... open ascii wave number file
     write(ofile2,'(a,a)') name(:nn),'.wvn'
     open(32,file=ofile2)
c
c
c... open ascii wave height file
    write(ofile3,'(a,a)') name(:nn),'.wh1'
    open(33,file=ofile3)
c
```

```
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c
c
c... open ascii wave angle file
     write(ofile4,'(a,a)') name(:nn),'.an1'
     open(34,file=ofile4)
c
c
c
c.... create depth array
     do 111 j=ny,1,-1
     read(20,*) (depthold(j,i),i=1,nx)
111 continue
     rewind(20)
c
c 10oct02 *******correct for sea level and change sign if GIS data
     do 711 i=1,nx
     do 811 j=1,ny
     depthold(j,i)=(gis*depthold(j,i))+sealev
811
      continue
711
       continue
c.....write rotated ascii depth file for internal use in ref/dif
165
      format(2405f12.2)
     DO 101 i=1,nx
     WRITE(31,165)(depthold(j,i),j=1,ny)
101 CONTINUE
     rewind(31)
c
c
     do 119 i=1,nx
     read(31,*) (depth(i,j),j=1,ny)
      continue
     rewind(31)
c
c... read first depth to initialize offshore boundary
c
       read(31,*) (di(m),m=1,ny)
       ridep=di(ny/2)
       rewind(31)
```

c.... would like a grid step size on the order of 1/5 wavelength

```
call getkcg(10.,wk,cg0)
       wl=2*pi/wk
       ifx=1+sx/(.2*w1)
       ify=1+sy/(.2*wl)
c
       nmx=ifx
       nmy=ify
c
     amp=amp/2.
c... calculate dimensions of interpolated grid
c
       dely=sy/ify
       delx=sx/ifx
     ndely=(ny-1)*ify
     ndelx=(nx-1)*ifx
c
c
     call getkcg(ridep,wk0,cg0)
c wave length (m)
     wl=2*pi/wk0
c wave frequency (rad/sec)
     sig=2*pi/per
c radder's correction factor
     do 33 j=1,ndely+1
      ccg(j)=sqrt(sig*cg0/wk0)
33
c
c
        ltype=2
c... open binary wave height file
     write(ofile5,'(a,a)') name(:nn),'.bw1'
     open(9,file=ofile5,form='unformatted')
c
c... open binary wave angle file
     write(ofile6,'(a,a)') name(:nn),'.ba1'
     open(11,file=ofile6,form='unformatted')
c.. initialize depth array
       do 555 nn=1,ndely+1
       hab(nn)=0.
       ih(nn)=0
555
       continue
```

```
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c
  enter initial condition at x=0
c
    call inbc(amp,wk0,theta,ndely,dely,alast)
c
    do 202 i=1,ndely+1
    kbar(1,j)=wk
     kbar(2,j)=wk
202
c scale alast as in radder(1978)
     do 32 j=1,ndely+1
     alast(j)=alast(j)*ccg(j)
      continue
32
c
c
     call wwave(alast,ndely,nmy,dely,kbar)
c
   solution of the parabolic eqn. by the crank-nicholson formulation
c
   start x increments
c
c
    c2=1./2./dely**2
    c3=2.*(0,1)/delx
c
C-----
c increments in x-direction
    ikount=0
    do 100 l=1, ndelx
c... write every 10th step to the screen
c
       if(10*((l+1)/10).eq.l+1) then
     print *,' column ',l+1,' of ',ndelx+1
       endif
c
    lm=l
c
     call intkcg(lm,kbar,ccg,di,ikount)
c correction factor
     do 34 j=1,ndely+1
     ccg(j)=sqrt(sig*ccg(j)/kbar(2,j))
34
      continue
```

```
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c increments in y-direction - 1st. round
     do 200 j=2, ndely
c
     kave=(kbar(2,j)+kbar(1,j))/2.
     kave=kbar(2,j)
c
c
     if(idf.eq.0) then
c small angle diffraction approximation
     t1=c2
     t3=c3*kave
     f=2*kave*(kave-wk)+c3/2*(kbar(2,j)-kbar(1,j))
c
     aa(j-1)=t1
     bb(j-1)=t3-2.*t1+f/2.
     cc(j-1)=t1
     dd(j-1)=-t1*alast(j+1)+(t3+2*t1-f/2)*alast(j)-t1*alast(j-1)
c
     endif
c
     if(idf.eq.1) then
c large angle diffraction approximation
c
     t1=c2/2.*(3.-wk/kave)
     t2=c2*c3/4./kave
     t3=c3*kave
     f=2*kave*(kave-wk)+c3/2*(kbar(2,j)-kbar(1,j))
c
     aa(i-1)=t1+t2
     bb(j-1)=t3-2.*(t1+t2)+f/2.
     cc(j-1)=t1+t2
     dd(j-1)=(t2-t1)*(alast(j+1)+alast(j-1))+(t3-2*(t2-t1)-f/2)
   & *alast(j)
     endif
c
     if(j.eq.2) then
     if(ibc.eq.1) cc(1)=aa(1)+cc(1)
c reflective b.c.: a1=a3
     if(ibc.eq.1) bb(1)=aa(1)+bb(1)
c reflective b.c.: a1=a2
     if(ibc.eq.0) then
c transmitting b.c.
```

```
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c
     if(isn.eq.0) then
c straight snell
     mip = (0.,1.)*wk*sin(theta)/2.
     mip=(1./dely-mip)/(1./dely+mip)
     else
c kirby's improved
     mip=(alast(2)-alast(1))/(alast(2)+alast(1))
     mip=(1.-mip)/(1.+mip)
     endif
c
     bb(1)=bb(1)+mip*aa(1)
     endif
     aa(j-1)=(0.,0.)
c
     endif
c
     if(j.eq.ndely) then
     if(ibc.eq.1) aa(j-1)=aa(j-1)+cc(j-1)
c reflective b.c.: an=an-2
     if(ibc.eq.1) bb(j-1)=bb(j-1)+cc(j-1)
c reflective b.c.: an=an-1
     if(ibc.eq.0) then
c transmitting b.c.
     if(isn.eq.0) then
c straight snell
     mim = (0.,1.)*wk*sin(theta)/2.
     mim=(1./dely+mim)/(1./dely-mim)
     else
c kirby's improved
     mim = (alast(j+1)-alast(j))/(alast(j+1)+alast(j))
     mim=(1.+mim)/(1.-mim)
     endif
c
     bb(j-1)=bb(j-1)+mim*cc(j-1)
     endif
     cc(j-1)=(0.,0.)
     endif
c
200 continue
```

```
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            ****
                    compute the solution *****
c
c
     neqs=ndely-1
c
     call tridag(aa,bb,cc,dd,uu,neqs)
c
     do 143 = 2, ndely
       alast(j)=uu(j-1)
143
c load alast(j)
     if(ibc.eq.1) then
     alast(1)=alast(3)
c
c reflective b.c.: a1=a3
     alast(ndely+1)=alast(ndely-1)
c
           : an=an-2
     alast(1)=alast(2)
c reflective b.c.: a1=a2
     alast(ndely+1)=alast(ndely)
           : an=an-1
c
     endif
c
     if(ibc.eq.0) then
c transmitting b.c.
c
     alast(1) = mip*alast(2)
     alast(ndely+1)= mim*alast(ndely)
c
     endif
c transform back into phys. height
     do 37 i=1,ndely+1
      aphys(j)=2.*alast(j)/ccg(j)
37
c
                      check for breaking *******
         *****
c
c
       if(ibreak.eq.1) then
     do 54 j=1,ndely+1
     hb=bc*di(j)
     rat=hb/cabs(aphys(j))
         if(di(j).lt.0) rat=0.
     if(rat.lt.1) then
c save point just before wave first breaks
         if(ih(j).eq.0) then
```

```
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               hab(j)=cabs(aprev(j))
               rlb(j)=(lm-1)/real(ifx)
c calculate direction before breaking
                if(j.eq.ndely+1.or.ih(j+1).eq.1) then
       xx=aimag((aprev(j)-aprev(j-1))/(dely*kbar(1,j)*aprev(j)))
                 if(xx.gt.1) xx=1.
               if(xx.lt.-1.) xx=-1.
                  dib(j)=asin(xx)
                 else
       xx=aimag((aprev(j+1)-aprev(j))/(dely*kbar(1,j)*aprev(j)))
                  if(xx.gt.1) xx=1.
                  if(xx.lt.-1.) xx=-1.
               dib(j)=asin(xx)
              endif
          dib(j)=270.-dib(j)*57.296-rot
                 if(dib(j).lt.0) dib(j)=360+dib(j)
         ih(j)=1
         endif
    aphys(j)=rat*aphys(j)
    alast(j)=rat*alast(j)
       endif
54
       continue
    endif
  *************************
c... writing wave field
       if(nmx*(lm/nmx).eq.lm.or.lm.eq.1) then
     call wwave(aphys,ndely,nmy,dely,kbar)
       endif
c
       do 55 nn=1,ndely+1
       aprev(nn)=aphys(nn)
55
       continue
100
      continue
c... output prebreak heights and directions
       open(24,file='break.dat')
       write(24,'(/,18x,a)')
                                REFRACTION - DIFFRACTION
       write(24,'(/,18x,a)') ' WAVE HEIGHT AND DIRECTION BEFORE BREAKING'
       write(24,'(/,15x,a,a,a,f5.1,a,f4.1)')
  & 'site: ',name,' direction: ',dir,' period: ',per
```

```
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       write(24, (/,20x,a,6x,a,6x,a,9x,a,//)')
   &'row',' H (m) ','col','dir'
       m=1
       write(24,'(18x,i4,f12.2,f12.1,f12.1)') m,hab(m),rlb(m),dib(m)
     alphab=dib(m)-shor
    irlb=NINT(rlb(m))+1
    breakd = (5.0/4.0)*hab(m)
***rasterisze break point file m....ixbra
     ixbra=2405-m
     write(39,'(18x,i4,i4,f12.3,f12.1,f12.3)') ixbra,irlb,hab(m),
   &alphab,breakd
       if(nmy.eq.1) then
       is=2
       else
       is=nmy
       endif
       mm=1
       do 888 m=is,ndely,nmy
       mm=mm+1
       write(24,'(18x,i4,f12.2,f12.1,f12.1)') mm,hab(m),rlb(m),dib(m)
     alphab=dib(m)-shor
     irlb=NINT(rlb(m))+1
     breakd = (5.0/4.0)*hab(m)
    ixbra=2405-mm
     write(39,'(18x,i4,i4,f12.3,f12.1,f12.3)')ixbra,irlb,hab(m),
   &alphab,breakd
888
       continue
c
c
    rewind (9)
    rewind (11)
c.....create wave height and wave angle arrays
     do 114 i=1,nx
          read(9) (wht(i,j),j=1,ny)
          read(11) (ang(i,j),j=1,ny)
114
       continue
c
c.... write array values to:
c.... ascii depth, wave number, wave angle, wave height grid files
c
c
```

c2002

format(4644f12.2)

```
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c
cccc Comment out old way of writing arrays
                do 134 i=1,nx
cccccccc
                write(33,2002) (wht(i,j),j=1,ny)
cccccccc
                write(34,2002) (ang(i,j),j=1,ny)
cccccccc
cccccccc134
                  continue
CCCCC Raster way of writing arrays!!!!!!!!!!
    DO 176 j=ny,1,-1
     write(33,6666) (wht(i,j),i=1,nx)
     write(34,6666) (ang(i,j),i=1,nx)
 176 CONTINUE
6666
      format(236f12.2)
6667
        format(236f12.6)
c
c
       go to 901
900
       write(*,'(20x,a,a,a)') '**** error **** ',ifile,' missing'
       write(*,'(20x,a)')'
                           press any key to continue '
       read(*,'(a)') idum
901
      continue
    stop
    end
c
     subroutine tridag(a,b,c,r,u,n)
c
     parameter (nmax=250002)
     complex gam(nmax),a(n),b(n),c(n),r(n),u(n),bet
c
    bet=b(1)
     u(1)=r(1)/bet
     do 11 j=2,n
     gam(j)=c(j-1)/bet
     bet=b(j)-a(j)*gam(j)
    if(bet.eq.0)pause
     u(j)=(r(j)-a(j)*u(j-1))/bet
11 continue
     do 12 j=n-1,1,-1
     u(j)=u(j)-gam(j+1)*u(j+1)
12
     continue
    return
```

```
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     end
c
     subroutine wwave(aphys,ndely,nmy,dely,kbar)
c
c compute & write wave height and direction, initial breaking point
    imax= max grid size nmax=max no. of steps
c
     parameter (nmax=45000000,imax=4500000)
       dimension amod(nmax),alfa(nmax)
       real kbar(2,nmax)
       complex aphys(nmax)
c
       nky=0
     do 350 j=1,ndely+1
       if(nmy*(j/nmy).eq.j.or.j.eq.1) then
       nky=nky+1
c
        amod(nky)=cabs(aphys(j))
        if(amod(nky).gt..05) then
        if(j.eq.ndely+1) then
        xx=aimag((aphys(j)-aphys(j-1))/(dely*kbar(2,j)*aphys(j)))
        if(xx.gt.1) xx=1.
        if(xx.lt.-1.) xx=-1.
        alfa(nky)=asin(xx)
        else
        xx=aimag((aphys(j+1)-aphys(j))/(dely*kbar(2,j)*aphys(j)))
        if(xx.gt.1) xx=1.
        if(xx.lt.-1.) xx=-1.
        alfa(nky)=asin(xx)
        endif
        else
        alfa(nky)=0
        endif
        alfa(nky)=alfa(nky)*57.296
       endif
350
       continue
c
     write(9) (amod(jj),jj=1,nky)
     write(11) (alfa(jj), jj=1, nky)
c
     return
```

```
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    end
c
    subroutine inbc(amp,wk,theta,ndely,dely,aa)
c
    complex aa(*)
c
    real ky
    pi=acos(-1.)
    theta=theta*pi/180.
    ky=wk*sin(theta)
c
    do 3 i=1,ndely+1
    ar=amp*cos(ky*(j-1)*dely)
    ai=amp*sin(ky*(j-1)*dely)
     ar = (cos(ky*(j-1)*dely) + cos(-ky*(j-1)*dely))/2.
c 2 identical
     ai = (\sin(ky*(j-1)*dely) + \sin(-ky*(j-1)*dely))/2.
c waves summed
    aa(j)=cmplx(ar,ai)
3
     continue
c
    return
    end
c
    subroutine getkcg(d,k,cg)
c
c.... depth (meters) and frequency are input
c.... returns wave number (2pi/l) and group velocity
c
    real k
    common /grid/ ny,sy,sx,dely,delx,ndely,f,dcon,ify,ifx,tide
    data tpi/6.2831853/
c
    sig=tpi*f
    a=d*sig*sig/9.81
    if(a.ge.1) then
         yhat=a*(1+1.26*exp((-1.84)*a))
         t=exp((-2)*yhat)
```

aka = a*(1+2*t*(1+t))

```
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    else
         aka = sqrt(a)*(1+a/6.*(1+a/5.))
    endif
    k=aka/d
    x=2*k*d
    cg = tpi*(f/k)*.5*(1+x/sinh(x))
    return
    end
c
subroutine intkcg(lm,kbar,ccg,di,ikount)
c
       parameter(max=45000002)
    real kbar(2,max)
    dimension ccg(max),d(2,max),di(max)
c
    save d
        common /cut/ dc
    common /grid/ ny,sy,sx,dely,delx,ndely,f,dcon,ify,ifx,tide
    data istart,ncol,eps/1,0,.0000001/
c... if this is the first call, load first two columns
    go to (1,3) istart
1
      istart=2
c... read in first 2 columns
        do 777 i=1,2
777
         read(31,*)(d(i,j),j=1,ny)
     ncol=1
c... interpolate grid points for first row
    do 2 i=1, ndely
    y=1+real(i-1)/real(ify)+eps
    ym=amod(y,1.)
    iy=int(y)
    di(i)=d(1,iy)+ym*(d(1,iy+1)-d(1,iy))+tide
    call getkcg(di(i)*dcon,rk,cg)
    ccg(i)=cg
2
     kbar(2,i)=rk
c... figure out which bathymetry grid columns should be used
c
3
     icol=1+real(lm)/real(ifx)+eps
```

```
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     if(icol.ne.ncol) then
      do 5 i=1,ny
5
      d(1,i)=d(2,i)
      read(31,*,end=999) (d(2,j),j=1,ny)
999
          ncol=icol
     endif
     dd1=d(1,100)
c
c... bilinear interpolate depths for new column of k and cg
      x=1+real(lm)/real(ifx)+eps
15
     xm=amod(x,1.)
     do 20 i=1, ndely
     y=1+real(i-1)/real(ify)+eps
     ym=amod(y,1.)
     iy=int(y)
     di(i)=d(1,iy)+xm*(d(2,iy)-d(1,iy))+ym*(d(1,iy+1)-d(1,iy))+
   & xm*ym*(d(2,iy+1)-d(1,iy+1)-d(2,iy)+d(1,iy))+tide
     if(di(i).le.01) then
     di(i) = .01
     endif
c
c... shift k and ccg col 2 to 1 and calculate new 2's
c
         if(di(i).lt.dc) di(i)=dc
     kbar(1,i)=kbar(2,i)
     call getkcg(di(i)*dcon,rk,cg)
     kbar(2,i)=rk
20
     ccg(i)=cg
     kbar(1,ndely+1)=kbar(1,ndely)
     kbar(2,ndely+1)=kbar(2,ndely)
     ccg(ndely+1)=ccg(ndely)
     di(ndely+1)=di(ndely)
c
     return
     end
```

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APPENDIX D

CODE FOR THE OCEANBAT BATHYMETRY RETRIEVAL MODULE

Appendix D: Code for the OCEANBAT Bathymetry Retrieval Module

```
c
        Bathymetry data base retrieval module oceanbat.f
c
           Scott A. Jenkins & Joseph Wasyl Jan 10, 2001
c
c
c
  THE LATTITUDE AND LONGITUDE OF THE UPPER LEFT CORNER OF A
  BATHYMETRY GRID REQUIRED.
c
  (The dimensions are specified in number of 3 second lat/lon points)
c
   (we have been using a default value of 200 x 200)
c
c reads packed grid files for the Southern California Bight
 and writes the data to an 200x200 formatted data file, 'site'.grd.
C***********************************
    integer*2 gdata(1201,801),dlist(12,10)
    character name*8,ofile*20,ifile*60,lfile*13,cfile*13
    character name2*60
    dimension ilat(3),ilon(3)
       real*4 latmin,latmax,lonmin,lonmax
    common /param/ ixo,iyo,ix,iy,ixmin
    common /int/ dx,dy,dgx,dgy,ang,ngx,ngy
c... initialize the disk list
c
    call list(dlist)
c
c... get name of the active study area
   open(11,file='oceanbat.inp',status='old')
c
   read(11,'(a)') name
c
    read(11,'(a)') name2
    write(namesub, '(a)') name2
c
    nn=8
    do 2 \text{ m}=8,1,-1
    if(name(m:m).eq.'') nn=m-1
    continue
2
c
    write(lfile, '(a,a)') name(:nn), '.log'
c
```

```
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     open(8,file=lfile)
4
     continue
c
     write(*,'(//,a)')
   &' OCEAN BATHYMETRY MODULE FOR THE CALIFORNIA COASTLINE
DATABASE'
c
c
   read(11,*) ilat(1)
   read(11,*) z
   ilat(2)=int(z)
   ilat(3)=nint(60.*(z-int(z)))
   read(11,*) ilon(1)
   read(11,*) z2
   ilon(2)=int(z2)
   ilon(3) = nint(60.*(z2-int(z2)))
c
     read(11,*) nx
     ngx=nx
c
     read(11,*) ny
     ngy=ny
c
c....read in upper left corner as origin lat lon
c.....write files across from left to right starting at origin
     nrot=3
c
c... figure out dimension of 3 sec by 3 sec grid
     dy = 92.6
     rlat=real(ilat(1))+real(ilat(2))/60+real(ilat(3))/3600
     dx=dy*cos(rlat*.017453)
c
     if(nrot.eq.1.or.nrot.eq.3) then
       dsave=dx
       dx=dy
       dy=dsave
     endif
c
c
     imx=0
     imy=0
c...output information to log file
c
```

write(8,'(6i4,a)') ilat,ilon

```
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     write(8, '(2f5.1,a)') dx, dy
     write(8, '(2i5,a)') ngx, ngy
     write(8,'(a)') ' 0 0 0 0'
c
     ang=0
     ang=pi*ang/180
     if(ang.lt.0) ang=2*pi+ang
c
c... truncate origin to a grid point
     ilat(3)=3*(nint(real(ilat(3))/3.))
     ilon(3)=3*(nint(real(ilon(3))/3.))
c
     iyo = ilat(1)*1200 + ilat(2)*20 + ilat(3)/3 - imy
     ixo=150000-(ilon(1)*1200+ilon(2)*20+ilon(3)/3)-imx
c
c... adjust origin based on rotation
     if(nrot.eq.1) then
          ixo=ixo-ngy+1
          nsave=ngy
          ngy=ngx
          ngx=nsave
     elseif(nrot.eq.2) then
          ixo=ixo-ngx+1
          iyo=iyo-ngy+1
     elseif(nrot.eq.3) then
          iyo=iyo-ngx+1
          nsave=ngx
          ngx=ngy
          ngy=nsave
     endif
c
c... find range of necesssary input data in both lat/lon and
    database units.
c
c
     ixmin=ixo
     ixmax=ixo+ngx-1
     iymin=iyo
     iymax=iyo+ngy-1
c
c... convert back to lat/lon coordinates
     latmin=real(iymin-1)/1200.
     latmax=real(iymax)/1200.
```

```
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     lonmax = (150000.-(ixmin+1))/1200.
     lonmin=(150000.-ixmax)/1200.
c
     print *,' lat/lon range ',latmin,latmax,lonmin,lonmax
c
     do 60 i=latmin,latmax
     nggx=1
     nggy=1
           iy=i*1200
           nys=iymin-iy
           nye=iymax-iy
           if(nys.lt.1) then
            nggy=abs(nys)+2
            nys=1
           endif
           if(nye.gt.1200) then
           if(nye.gt.2400) then
            nye=1200
            else
            nye=1200
            nggy=1
            endif
           endif
     do 60 j=lonmax,lonmin,-1
           ix=150000-(j+1)*1200-1
           nxs=ixmin-ix
           nxe=ixmax-ix
           if(nxs.lt.1) then
            nggx=abs(nxs)+2
            nxs=1
           endif
           if(nxe.gt.1200) then
           if(nxe.gt.2400) then
c
               nxe=1200
c
               nggx=1201
c
               else
c
c
            nggx=1
            nxe=1200
            endif
           endif
c
c... figure out which bathymetry file to read
     il=i-31
    jl=j-116
     ndisk=dlist(il,jl)
c
```

```
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45
      continue
c
c
     write(ifile,1000)name2(1:LSTGDCHR(name2)), i,j
1000 format(a,'/',i2,i3,'pg.dta')
     close(20)
     print *,' searching: ',ifile
     open(20,file=ifile,status='old',form='unformatted',err=50)
     go to 51
50
      write(*,'(a,a,a)')
   + '????? unable to find ',ifile,' on the disk ?????'
     write(*,'(a,\)') ' abort loading ? (y/n) : '
     read(*,*) ans
     if(ans.eq.'y'.or.ans.eq.'Y') then
     stop
     else
     go to 45
     endif
c
51
      continue
c
     call unpack(gdata,nxs,nxe,nys,nye,nggx,nggy)
c
60
      continue
c... output the grid array, one s to n column at a time from w to e
     write(ofile,2000) name(:nn)
2000 format(a,'.grd')
     open(12,file=ofile)
c... output depends on rotation
2009 format(200f8.3)
     if(nrot.eq.1) then
     do 70 j=1,ngy
     write(12,2009) (real(gdata(i,j))*.1+.853,i=ngx,1,-1)
70
      continue
     elseif(nrot.eq.2) then
     do 71 i=ngx,1,-1
     write(12,2009) (real(gdata(i,j))*.1+.853,j=ngy,1,-1)
      continue
71
     elseif(nrot.eq.3) then
     do 72 j = ngy, 1, -1
```

```
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     write(12,2009) (real(gdata(i,j))*.1+.853,i=1,ngx)
72
      continue
    else
    do 73 i=1,ngx
     write(12,2009) (real(gdata(i,j))*.1+.853,j=1,ngy)
73
      continue
    endif
c
c... make the default contour command file for the base map
c... maximum length is 5.5 inches
    rlmax=max(real(ngx)*dx,real(ngy)*dy)
    h=6.5*real(ngy)*dy/rlmax
    w=6.5*real(ngx)*dx/rlmax
    write(cfile,'(a,a)') name(:nn),'.cnt'
    open(7,file=cfile)
     write(7, '(a,a)') 'file
                         ',ofile
     write(7,'(a,a)') 'format binary'
     write(7,*) 'axes ',lonmax,lonmin,latmin,latmax
     write(7,'(a)') 'xlabel Latitude'
     write(7,'(a)') 'ylabel Longitude'
     write(7,'(a)') 'caption'
     write(7,'(a)') 'title Depth Contours (meters from MSL)'
    write(7,'(a)') 'letter .1 .1 .1 .1 .1'
     write(7,'(a,f5.2)') 'width ',w
     write(7,'(a,f5.2)') 'height ',h
     write(7,'(a,i4,i4)') 'read
                             ',ngx,ngy
    write(7,*) 'plot ',.5*(8.5-w),.5*(11.-h)
     write(7,'(a)') 'stop'
c
c.....
c.....write ascii bathymetry
      rewind(12)
c
c
c
c... open ascii bathymetry file
      write(ofile1,'(a,a)') name(:nn),'.dep'
c
     open(17,file=ofile1)
c
c
c
    end
c
C***********************************
c
    subroutine unpack(gdata,nxs,nxe,ns,ne,ngx,ngy)
```

```
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c
c... longitudes are reversed as read in to array since opposite of xy coor.
     integer*2 gdata(1201,801),dum
     integer*2 rec(1200),pdata(1200),idata(1200),dmin,dmax
     common /param/ ixo,iyo,ix,iy,ixmin
     data dmin,dmax/5001,-999/
c
c
c... loop through the files with desired data
c
     read(20) rec
c
c... read records
     ny=ngy
     do 100 i=1,ne
     print *, ' reading rec ',i
c... unpacking data
c... check if record only contains deep water or land
     if(rec(i).lt.0) then
     if(i.lt.ns) go to 100
     if(rec(i).eq.-1) then
          depth=3050
     else
          depth=-20
     endif
          do 33 m=1,1200
33
           idata(m)=depth
          go to 91
     endif
c
c... unpack the row
     icnt=1201
c
     if(i.lt.ns) then
     read(20) dum
     go to 100
     endif
c
     read(20) (pdata(m),m=1,rec(i))
c
     do 90 j=1, rec(i)
```

```
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c
    if(pdata(j).lt.-10000) then
        if(pdata(j).lt.-20000) then
             ncnt=abs(pdata(j)+20000)
             depth=-20
        else
             ncnt=abs(pdata(j)+10000)
             depth=3050
        endif
        do 85 m=1,ncnt
        icnt=icnt-1
85
         idata(icnt)=depth
        go to 90
    endif
c
    icnt=icnt-1
    idata(icnt)=pdata(j)
c
90
     continue
c... place desired data from record into grid array
91
     continue
    nx=ngx
c
    do 92 jj=nxs,nxe
c
    gdata(nx,ny)=idata(jj)
    dmax=max0(gdata(kx,ky),dmax)
c
     dmin=min0(gdata(kx,ky),dmin)
c
92
     nx=nx+1
c
98
     ny=ny+1
c
c.... save rec
100
     continue
    print *,dmin,dmax
c
c
c
    ngx=nx
    ngy=ny
c
    return
    end
c
```

```
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    subroutine list(dlist)
c
c... contains the floppy disk no. for grid files i=lat-31; j=lon-116
    integer*2 dlist(12,10)
c
    do 10 i=1,12
    do 10 j=1,10
    dlist(i,j)=-1
     continue
10
c
    dlist(1,1)=1
    dlist(1,2)=1
    dlist(1,3)=1
    dlist(2,2)=1
    dlist(2,1)=2
    dlist(2,3)=2
    dlist(3,2)=2
    dlist(4,4)=2
    dlist(2,4)=3
    dlist(5,5)=3
    dlist(3,3)=3
    dlist(3,4)=4
    dlist(4,5)=4
    dlist(5,6)=5
c
    return
    end
INTEGER FUNCTION LSTGDCHR(NAMESUB)
   CHARACTER*(*) NAMESUB
   INTEGER II,CLEN,INDEX
   CLEN = LEN(NAMESUB)
   DO 100 \text{ II} = \text{CLEN}, 1, -1
100 IF( ICHAR(NAMESUB(II:II)) .GT. 60 .AND.
       ICHAR(NAMESUB(II:II)) .LT. 127 )GOTO 101
101 INDEX=II
   LSTGDCHR = INDEX
   RETURN
   END
```

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APPENDIX E CODE FOR THE SEDXPORT TRANSPORT MODEL

Appendix E: Code for the SEDXPORT Transport Model

```
c
c sedxport3.for Transport Model - (bottom, surfzone, outfall sources)
    with NULL POINT January. 24, 2005 & selected grid point - 10:00
c
     TIME STEP MODE beach outfall source and canyon sink modules
c
      Littoral Advection/Diffusion
c
      Fine Sediment Dispersion MODEL
c
      written by Scott A.Jenkins & Joseph Wasyl
c
c
c
   parameter (ni=200, nj=200)
c
   real*4 n0,n0_1
c
   character name*8,infil1*12,infil2*12
   character infile3*12,ifile2*12
   Dimension wnum(ni,nj),wht(ni,nj),depth(ni,nj)
   dimension iyb(nj),ixb(nj),bh(nj),ba(nj),bd(nj)
   dimension ubr(nj),dxbr(nj),dxbave(nj),ubave(nj)
   dimension zw(5)
   dimension r7(10,5),revr7(10,5)
   dimension d(10),rcp(10),rcc(10),rcsp(10),rszcp(10)
   dimension rrs(10), rrsp(10)
c******************null point arrays*************
   dimension type(ni,nj)
c ... second outfall arrays
   dimension rrs2(10),rrsp2(10)
   dimension rrivr2(10,5),pnrr2(10,5)
c
c
   dimension rsurf(10,5),rriv(10,5),rbot(10,5)
   dimension pns(10,5),pnr(10,5),pnb(10,5),pnt(10,5)
   dimension sp(10), sp2(10), spp(10)
```

```
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    dimension suml1(ni,nj),suml2(ni,nj)
    dimension suml3(ni,nj),suml4(ni,nj),b(ni,nj),b2(ni,nj)
    dimension ux(ni,nj),uy(ni,nj)
c
c.....arrays unique to selected grid point loop
    dimension sum2(101),zw3(101),dep2(101),slope(101),a4402(101)
    dimension r72(10,101),revr72(10,101),dep(100)
    dimension rsurf2(10,101),rriv2(10,101),rbot2(10,101)
    dimension pns2(10,101),pnr2(10,101),pnb2(10,101),pnt2(10,101)
c
c ... second outfall arrays unique to selected grid point
    dimension rriv2r2(10,101),pnr2r2(10,101)
C**********************
c.. Salinity module arrays
    dimension salmean(5),sal2mean(101)
    dimension sriv1(5), sriv2(5), salr7(5)
   dimension s2riv1(101),s2riv2(101),sal2r7(101)
   dimension sall1(ni,nj),sall2(ni,nj),sall3(ni,nj)
   dimension sall4(ni,nj)
   dimension acdom1(ni,nj),acdom2(ni,nj),acdom3(ni,nj)
   dimension acdom4(ni,nj),a440(101)
c
C
c
c
c
    open(18,file='CEM_sedxport3.inp',status='old')
c
    read(18,'(a)') name
      read(18,*) nx
    read(18,*) ny
    read(18,*) igrdx
      read(18,*) igrdy
      read(18,*) sx
    read(18,*) sy
    read(18,*) persw
    read(18,*) perwin
    read(18,*) hsw
```

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read(18,*) hwin

read(18,*) tanbeta

read(18,*) aks

read(18,*) akb

read(18,*) ak2

read(18,*) rci

read(18,*) rszi

read(18,*) ak

read(18,*) q

read(18,*) irx

read(18,*) iry

read(18,*) dr

read(18,*) rwidth

read(18,*) rrsi

read(18,*) q2

read(18,*) irx2

read(18,*) iry2

read(18,*) dr2

read(18,*) rwidth2

read(18,*) rrsi2

read(18,*) ak3

read(18,*) ak4

read(18,*) ak5

read(18,*) ibins

read(18,*) verdat

read(18,*) numlay

read(18,*) alay1

read(18,*) alay2

read(18,*) alay3

read(18,*) alay4

read(18,*) surf

read(18,*) river

read(18,*) river2

read(18,*) bottom

read(18,*) rhos

read(18,*) rhosr

read(18,*) rhosr2

read(18,*) tcon

read(18,*) tconriv

read(18,*) deltat

read(18,*) timestep

read(18,*) n0

read(18,*) gamma

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read(18,*) salo
read(18,*) w0sal
read(18,*) ak3s
read(18,*) ak5s
read(18,*) ak5s
read(18,*) domslope

read(18,*) dominter read(18,*) domback

read(18,*) tconsal

read(18,*) ak7 read(18,*) ilat1

read(18,*) z

read(18,*) two_layer

read(18,*) v

read(18,*) ak8

read(18,*) p_mbar

read(18,*) dmix_for

read(18,*) dmix

read(18,*) ak2_1

read(18,*) ak_1

read(18,*) ak3_1

read(18,*) ak5_1

read(18,*) tcon_l

read(18,*) tconriv_1

read(18,*) n0_1

read(18,*) gamma_1

read(18,*) delsal_1

read(18,*) w0sal_1

read(18,*) ak3s_1

read(18,*) ak5s_1

read(18,*) aksal 1

read(18,*) tconsal_l

c

c...convert time to seconds

time=deltat*60*60

c......convert tcon by multiplying by 10 E-6 tcon=tcon*0.001

c

if(timestep.NE.0.0)then

open(27,file='dmix_wind.dat',status='old')

read(27,*) dmix_old

read(27,*) epsilonw_old

dmix old=dmix old

```
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    close(27)
    else
c......when no forecast available dmix_old is set to nowcast n0_l at t=0
    dmix_old=n0_l
    endif
c
c.....convert background in mg/l to number of grains
     n0=n0*100
c
c
    ilat2=int(z)
    ilat3=nint(60.*(z-int(z)))
    theta=real(ilat1)+real(ilat2)/60+real(ilat3)/3600
    v=v*0.5148*100.0
    ak8=0.00009/((v**0.5)+1.1103)
    ustar = (ak8*(v**2))**0.5
    rot_e=0.000072685
    rho a=0.001293*(p mbar/760)**0.714285714
    f=2.0*rot_e*SIND(theta)
c
c.....calculate for residual from previous time step
c.....epsilonw is in cgs units
    epsilonw=0.0000043*(v**2)
    if(v.LE.600)epsilonw=0.0000000102*(v**3)
    if(timestep.EQ.0.0)epsilonw old=epsilonw
c..calculate sediment layer depth in meters when dmix_for=0 (no forecast)
    if(dmix_for.EQ.0.0)then
    dmix=0.4*ustar/(f*100)
    endif
c...if a forecast dmix is used do not modify from previous timestep
    if(dmix for.NE.0)go to 1867
    dmix=dmix+((dmix old-dmix)*
   &EXP(-1.0*epsilonw_old*time/(dmix_old*100.0)**2))
1867 continue
c
c.....in a single layer system dmix is set to 500 meters!!
    if(two_layer.EQ.0.0)dmix=500.0
c
c......dmix still in meters prior to writing
```

```
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     open(27,file='dmix_wind.dat',status='unknown')
     write(27,8030)dmix
     write(27,8030)epsilonw
8030
        format(f20.10)
c
c......convert dmix back to centimeters for subsequent calculations
     dmix=dmix*100.0
c
     alay3=(dmix-100)
     alay4 = (dmix + 100)
c
     open(19,file='bulk_density.dat',status='old')
     read(19,*) rci1
     read(19,*) rci2
     read(19,*) rci3
     read(19,*) rci4
     read(19,*) rci5
     read(19,*) rci6
     read(19,*) rci7
     read(19,*) rci8
     read(19,*) rci9
     read(19,*) rci10
     read(19,*) rci11
     read(19,*) rci12
     read(19,*) rci13
     read(19,*) rci14
c
     ires=101
     riv2=river2
     riv=river
     bot=bottom
c
c
     salmax=(domback-dominter)/domslope*(-1.0)
c...divide n0 grains among the first 5 grain size bins
    an0b1=n0*.767
    an0b2=n0*.179
    an0b3=n0*.0376
    an0b4=n0*.00886
    an0b5=n0*.00609
    an0b6=n0*.00142
```

```
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    an0b7=n0*.000248
    an0b8=n0*.000121
    an0b9=n0*.0000484
    if(an0b1.LE.2.0)an0b1=2.0
    if(an0b2.LE.2.0)an0b2=2.0
    if(an0b3.LE.2.0)an0b3=2.0
    if(an0b4.LE.2.0)an0b4=2.0
    if(an0b5.LE.2.0)an0b5=2.0
    if(an0b6.LE.2.0)an0b6=2.0
    if(an0b7.LE.2.0)an0b7=2.0
    if(an0b8.LE.2.0)an0b8=2.0
    if(an0b9.LE.2.0)an0b9=2.0
c
c
     per=persw
c
     numlay=4
     if(numlay.GE.4)numlay=4
c
c....open array of bulk density values from previous time step
c
     OPEN(2, FILE='timestep.sed', ACCESS='DIRECT', RECL=24,
   & FORM='UNFORMATTED')
c
c.....open planview of grain number output files
c
c... open ascii file layer 1 grain number / cc
     open(41,file='layer1.sed')
     open(81,file='layer1.sal')
     open(85,file='layer1.dom')
     open(91,file='layer1.slp')
c
c... open ascii file layer 1 grain number / cc
     open(42,file='layer2.sed')
     open(82,file='layer2.sal')
     open(86,file='layer2.dom')
     open(92,file='layer2.slp')
c
```

if(two layer.NE.0)then

```
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c... open ascii file layer 1 grain number / cc
     open(43,file='layer3.sed')
     open(83,file='layer3.sal')
     open(87,file='layer3.dom')
c
c... open ascii file layer 1 grain number / cc
     open(44,file='layer4.sed')
     open(84,file='layer4.sal')
     open(88,file='layer4.dom')
c
     endif
c
c
c
        freq=1/per
     pi=acos(-1.)
     g = 980.0
     ro = 1.03
    omega=2*pi/per
c
c
        nn=8
        do 3 \text{ m}=8,1,-1
        if(name(m:m).eq.' ') nn=m-1
3
c
c
c... open grain size and mass percent file 'masstotal.dat'
c
     open(26,file='masstotal.dat',status='old')
c
c... read data from file
c
     nfact=1
     do 44 i=1,ibins
     read(26,*)d(i),rcp(i),rcsp(i),rrsp(i),rrsp2(i)
c ..... convert microns to centimeters
     d(i)=d(i)*.0001
     rc(i)=rcp(i)*rci
     rszcp(i)=rcsp(i)*rszi
     rrs(i)=rrsp(i)*rrsi
     rrs2(i)=rrsp2(i)*rrsi2
     nfact=nfact*i
44 continue
```

```
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c
c...open selected point file at selected grid point(igrdx,igrdy)
    open(99,file='profile.sed',status='unknown')
c
c
c... open bottom type classification file (from grass)
    open(29,file='bottom_type.dat',status='old')
c
c
c
c...open bottom distribution file at selected crossection(igrdy)
c.....this part of program commented out for integrated cwc model
      open(50,file='nullpt.dat',status='unknown')
c
c....open bottom distribution file at selected point(igrdx,igrdy)
c.....this part commented out for integrated model
      open(88,file='nullpt.igrdx')
c.
c... open ascii wave number file from swell component
    write(ifile2,'(a,a)') name(:nn),'.wvn'
    open(32,file=ifile2,status='old')
c
c... open ascii depth file corrected to tide
       write(infil1,'(a,a)') name(:nn),'.dep'
    open(20,file=infil1,status='old')
c
c.....open total wave height file
       write(infil2,'(a,a)') name(:nn),'.wht'
    open(9,file=infil2,status='old')
c... open ascii file x-component of current
    open(45,file='xcur.xpt')
c
c... open ascii file y-component of current
    open(46,file='ycur.xpt')
c
c... read bathymetry, wave height, swell wave number x1, y=1,200
c....read x-component of current, y-component of current
```

```
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c....x1, y1 is bottom left corner looking at page by convention
c
c.....read into arrays depth, wave height, wave number
     do 111 i=1,nx
    read(20,*) (depth(i,j),j=1,ny)
    read(9,*)(wht(i,j),j=1,ny)
    read(32,*)(wnum(i,j),j=1,ny)
      continue
111
c.....read in xcur, ycur, bot type verticle system reversed (raster)
     do 168 j=ny,1,-1
     read(45,*)(ux(i,j),i=1,nx)
     read(46,*)(uy(i,j),i=1,nx)
    read(29,*) (type(i,j),i=1,nx)
c
168
      continue
c
c.... open breaker input file
       write(infile3,'(a,a)') name(:nn),'.bra'
     open(38,file=infile3)
c.... read x,y, breaker height, breaker angle, breaker depth
     do 150 n=1,ny
     read(38,*) iyb(n),ixb(n),bh(n),ba(n),bd(n)
c.....determine wave height, wnum at the break point
     ibpx=ixb(n)-1
     whtbc=wht(ibpx,n)*100
    wnumbc=wnum(ibpx,n)/100
    depthbc=depth(ibpx,n)*100
     wndepb=wnumbc*depthbc
c
c...create ix by 200 arrays of dxbr's and ubr's at the break point
     dxbr(n)=whtbc/SINH(wndepb)
     ubr(n)=dxbr(n)*omega
c
150
      continue
c.....calculate average values dxbave and ubave
     do 151 i=1,200
c
    im1=i-1
```

```
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    im2=i-2
    ip1=i+1
    ip2=i+2
    if(im1.LT.1)im1=1
    if(im2.LT.1)im2=1
    if(ip1.GT.200)ip1=200
    if(ip2.GT.200)ip2=200
    dxbave(i)=(dxbr(i)+dxbr(im1)+dxbr(im2)+dxbr(ip1)+dxbr(ip2))/5
    ubave(i)=(ubr(i)+ubr(im1)+ubr(im2)+ubr(ip1)+ubr(ip2))/5
c
151
       continue
c
c.....initilize icount to 0 outside of iy,ix, and ipart loops
c.....and rtold,rsold,qsold,qs2old,salold,salold2 for timestep=0
     icount=0
     rtold=0.0
     rsold=0.0
     qsold=0.0
     qs2old=0.0
     salold=0.0
     salold2=0.0
c
c.....determine wave height, wnum at the outfall
     whtrm=wht(irx,iry)*100
c
     wnumrm=wnum(irx,iry)/100
c
     depthrm=depth(irx,iry)*100
c
     wndeprm=wnumrm*depthrm
c
c
c
c...... at the outfall
     drm=whtrm/SINH(wndeprm)
c
c
     urm=drm*omega
c
    do 1800 iy=1,ny
    allxbrk=0
c
    bd1=bd(iy)*100
    if(bd1.LT.10.0)bd1=10.0
    ba1=ba(iy)
    bh1 = bh(iy) * 100
c
c.....determine wave height, wnum at the break point
     ibpx=ixb(iy)-1
```

```
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    whtbc=wht(ibpx,iy)*100
    wnumbc=wnum(ibpx,iy)/100
    depthbc=depth(ibpx,iy)*100
    wndepb=wnumbc*depthbc
c
c..... at the break point
    dxb=whtbc/SINH(wndepb)
    ub=dxb*omega
c
c
c
     stopdep=bd(iy)
c
    do 1801 ix=1,nx
c
if(type(ix,iy).LE.100)go to 7006
c
    if(type(ix,iy).LE.200)then
    rci=(rci2-rci1)*(type(ix,iy)-100)/100+rci1
    go to 7007
    endif
c
    if(type(ix,iy).LE.300)then
    rci=(rci3-rci2)*(type(ix,iy)-200)/100+rci2
    go to 7007
    endif
c
    if(type(ix,iy).LE.400)then
    rci=(rci4-rci3)*(type(ix,iy)-300)/100+rci3
    go to 7007
    endif
c
    if(type(ix,iy).LE.500)then
    rci=(rci5-rci4)*(type(ix,iy)-400)/100+rci4
    go to 7007
    endif
c
    if(type(ix,iy).LE.200)then
    rci=(rci2-rci1)*(type(ix,iy)-100)/100+rci1
    go to 7007
    endif
c
```

```
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     if(type(ix,iy).LE.300)then
     rci=(rci3-rci2)*(type(ix,iy)-200)/100+rci2
     go to 7007
     endif
c
     if(type(ix,iy).LE.400)then
     rci=(rci4-rci3)*(type(ix,iy)-300)/100+rci3
     go to 7007
     endif
c
     if(type(ix,iy).LE.500)then
     rci=(rci5-rci4)*(type(ix,iy)-400)/100+rci4
     go to 7007
     endif
c
     if(type(ix,iy).LE.600)then
     rci=(rci6-rci5)*(type(ix,iy)-500)/100+rci5
     go to 7007
     endif
c
     if(type(ix,iy).LE.700)then
     rci=(rci7-rci6)*(type(ix,iy)-600)/100+rci6
     go to 7007
     endif
c
     if(type(ix,iy).LE.800)then
     rci=(rci8-rci7)*(type(ix,iy)-700)/100+rci7
     go to 7007
     endif
     if(type(ix,iy).LE.900)then
     rci=(rci9-rci8)*(type(ix,iy)-800)/100+rci8
     go to 7007
     endif
c
     if(type(ix,iy).LE.1000)then
     rci=(rci10-rci9)*(type(ix,iy)-900)/100+rci9
     go to 7007
     endif
c
     if(type(ix,iy).LE.1100)then
     rci=(rci11-rci10)*(type(ix,iy)-1000)/100+rci10
     go to 7007
```

Hydrodynamic Modeling of Storm Drain Discharges in the Neighborhood of ASBS 29 in La Jolla, California Revised: 13 March 2013 endif c if(type(ix,iy).LE.1200)then rci=(rci12-rci11)*(type(ix,iy)-1100)/100+rci11 go to 7007 endif c if(type(ix,iy).LE.1300)then rci=(rci13-rci12)*(type(ix,iy)-1200)/100+rci12 go to 7007 endif c if(type(ix,iy).LE.1400)then rci=(rci14-rci13)*(type(ix,iy)-1300)/100+rci13 go to 7007 endif c if(type(ix,iy).GT.1400)then rci=rci14 go to 7007 endif c 7006 rci=rci1 7007 continue distbp = ((201-ix)-ixb(iy))*sx*100c uy1=uy(ix,iy)+0.000001ux1=ux(ix,iy)+0.000001c c c.....ADD resident sediment volume c...... initialize total grain variable at each x,y location suml1(ix,iy)=n0suml2(ix,iy)=n0sum13(ix,iy)=n0sum14(ix,iy)=n0c sall1(ix,iy)=salo sall2(ix,iy)=salo

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sall3(ix,iy)=salo

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     sall4(ix,iy)=salo
c
     b(ix,iy)=gamma
     b2(ix,iy)=gamma
c
c
    if(depth(ix,iy).LE.stopdep)then
    allxbrk=1.0
     do 478 irt=ix,nx
     suml1(irt,iy)=-1.0
     sum12(irt,iy)=-1.0
     sum13(irt,iy)=-1.0
     sum14(irt,iy)=-1.0
c
     sall1(irt,iy)=-1.0
     sall2(irt,iy)=-1.0
     sall3(irt,iy)=-1.0
     sall4(irt,iy)=-1.0
c
     acdom1(irt,iy)=-1.0
     acdom2(irt,iy)=-1.0
     acdom3(irt,iy)=-1.0
     acdom4(irt,iy)=-1.0
c
c
     b(irt,iy)=-1.0
     b2(irt,iy) = -1.0
478
      continue
     go to 479
     endif
c
c
c ..... convert wave hieght, water depth, and wave number into cgs
     whtc=wht(ix,iy)*100
     wnumc=wnum(ix,iy)/100
     depthc=depth(ix,iy)*100
    wndep=wnumc*depthc
c
     d0=whtc/SINH(wndep)
     u=omega*d0
```

c

```
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c
c
c
    do 1776 ipart=1,ibins
c
    dip=d(ipart)
    rrsfix2=rrs2(ipart)
    rrsfix=rrs(ipart)
    rcspfix=rszcp(ipart)
c**********************null point begin*****************
    d0np=d0
    dm=ak7*d0np
    unp=rszi/(rhos*pi*dm)
    unpd1=unp*d(1)
    if(unpd1.GT.ibins)unp=ibins/d(1)
    if(unp.LT.ibins/(100*d(1)))unp=ibins/(100*d(1))
    unpdip=unp*dip
    rc(ipart)=rci*6.092*(((unp*dip)**ibins)/nfact)*EXP(-1*unp*dip)
    if(rc(ipart).LT.0.0000000000000001)rc(ipart)=0.0000000000000001
c
c.....nullpt selected grid write commented out
     if(iy.NE.igrdy)go to 8001
c
     if(ix.NE.igrdx)go to 8002
c
     write(88,8003)dip,rc(ipart),depth(ix,iy),unpdip
c8003 format(4e10.3)
c8002 continue
       continue
c8001
c********************null point end********************
    rcfix=rc(ipart)
c
c
     icount=icount+1
c
    if(timestep.EQ.0.0)go to 81
    read(2, REC=icount)rtold,rsold,qsold,qs2old,salold,salold2
81
      continue
c
    if(ipart.EQ.1)w0=.00011
    if(ipart.EQ.2)w0=.00043
    if(ipart.EQ.3)w0=.0017
    if(ipart.EQ.4)w0=.0050
```

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    if(ipart.EQ.5)w0=.0108
    if(ipart.EQ.6)w0=.044
    if(ipart.EQ.7)w0=.17
    if(ipart.EQ.8)w0=.38
    if(ipart.EQ.9)w0=.98
C*****************************
    if(iy.NE.igrdy)go to 4000
    if(ix.NE.igrdx)go to 4001
c
    ipart2=ipart
c
c
    fw=EXP(5.2*((2.5*dip/d0)**0.2)-6.0)
c
    delta=0.72*d0*(2.5*dip/d0)**0.25
    deltab=0.72*dxb*(2.5*dip/dxb)**0.25
    deltar=0.72*dxbave(iy)*(2.5*dip/dxbave(iy))**0.25
    deltas=0.72*dxbave(iy)*(0.000006625/dxbave(iy))**0.25
c
c.....for the outfall sediment flux use running mean values of ub *dxb
   shieldr=ubave(iy)**2/((rhos/ro-1.0)*g*dip)
   epsilonr_l=0.00035*ak3_l*((ubave(iy)/w0)**0.68)*
   &(shieldr**0.4)*deltar*g*per
   if(epsilonr 1.LT.0.0000000001)epsilonr 1 = 0.0000000001
   epsilonr=epsilonr l+epsilonw
c.....for the outfall plume salinity use running mean values of ub *dxb
   shieldrs=ubave(iy)**2/(.03*g*dr)
   epsilonrs l=0.00035*ak3s*((ubave(iy)/w0sal)**0.68)*
   &(shieldrs**0.4)*deltas*g*per
   if(epsilonrs_1.LT.0.0000000001)epsilonrs_1 = 0.0000000001
   epsilonrs_l+epsilonw
c
c....at the local break point
    shieldb=ub**2/((rhos/ro-1.0)*g*dip)
    epsilonb l=0.00035*ak3*((ub/w0)**0.68)*(shieldb**0.4)
  &*deltab*g*per
    if(epsilonb_l.EQ.0)epsilonb_l=0.0000000001
    epsilonb=epsilonb_l+epsilonw
c
c....at the local break point for disequilibrium profile
```

```
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     epsilonbs_l=0.00035*ak3s*((ub/w0sal)**0.68)*(shieldrs**0.4)
   &*deltas*g*per
     if(epsilonbs_1.EQ.0.0000000001)epsilonbs_1 = 0.0000000001
    epsilonbs=epsilonbs_l+epsilonw
c
c.....offshore
     shield=u**2/((rhos/ro-1.0)*g*dip)
     epsilon_l=0.00035*ak2*((u/w0)**0.68)*(shield**0.4)*delta*g*per
     if(epsilon_1.EQ.0.0000000001)epsilon_1 = 0.0000000001
     epsilon=epsilon l+epsilonw
c
c.....outfall shields and epsilon calculated in RIVERM subroutine
c**
c**
    rrsfix2=rrs2(ipart2)
    rrsfix=rrs(ipart2)
     rcspfix=rszcp(ipart2)
    rcfix=rc(ipart2)
c
    if(riv.EQ.1)then
     CALL RIVERM(epsilonr,dr,rwidth,q,ux1,uy1,ix,iy,irx,iry,sx,
   &sy,rrsfix,ak5,w0,deltab,rhosr,ro,dip,g,per,ak3,time,qsold,
   &depthc,tconriv,qs)
     CALL SALM1(epsilonrs,dr,rwidth,q,ux1,uy1,ix,iy,irx,iry,sx,
   &sy,salo,ak5s,w0sal,deltas,rhosr,ro,dip,g,per,ak3,time,salold,
   &depthc,tconsal,sal)
c
     else
    continue
     endif
c
     if(riv2.EQ.1)then
     CALL RIVER2M(epsilonr,dr2,rwidth2,q2,ux1,uy1,ix,iy,irx2,iry2,sx,
   &sy,rrsfix2,ak5,w0,deltab,rhosr2,ro,dip,g,per,ak3,time,qs2old,
   &depthc,tconriv,qs2)
c**
c**
     CALL SALM2(epsilonrs,dr2,rwidth2,q2,ux1,uy1,ix,iy,irx2,iry2,sx,
   &sy,salo,ak5s,w0sal,deltas,rhosr2,ro,dip,g,per,ak3,time,salold2,
   &depthc,tconsal,sal2)
c
     else
     continue
```

```
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     endif
c
     if(surf.EQ.1)then
     CALL SURFLM(epsilonb,bd1,ba1,bh1,ibpx,ix,iy,
   &ak4,aks,akb,dip,w0,pi,g,rhos,ro,tanbeta,rcspfix,time,rsold,
   &depthc,tcon,rs)
     else
    continue
     endif
c
c
     if(bot.EQ.1)then
c**
c**
     CALL BOTTOMM(ix,iy,u,shield,fw,ak,pi,wndep,rcfix,time,rtold,
   &depthc,tcon,rt,w0,epsilon)
     else
     continue
     endif
c
     do 2002 k2=1,ires
     zw3(k2) = depthc/100*(k2-1)
     dep2(k2)=depthc/100*(ires-k2)
     if(k2.EQ.1)zw3(k2)=delta
    if(k2.EQ.1)dep2(k2)=depthc-delta
c
c ....sediment transport calculations
c.....initialize sal2mean(k2)
    sal2mean(k2)=salo
     a4402(k2)=dominter-domslope*sal2mean(k2)
    if(sal2mean(k2).GE.salmax)a4402(k2)=domback
     if(dmix.GE.dep2(k2))then
     sal2r7(k2)=(-1.0)*dep2(k2)*aksal*w0sal/epsilonbs
     else
     sal2r7(k2)=(-1.0)*dep2(k2)*aksal*((dimx/4.21)**2.4)
   &*w0sal/epsilonbs_1
     endif
c
     if((depthc-dmix).GE.zw3(k2))then
     r72(ipart2,k2)=(-1.0)*zw3(k2)*w0/epsilon_l
     else
    r72(ipart2,k2)=(-1.0)*zw3(k2)*w0*(dmix/dip)**2.4/epsilon
     endif
```

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     if((depthc-dmix).LT.0.0)r72(ipart2,k2)=(-1.0)
   &*zw3(k2)*w0/epsilon
c
     if(dmix.GE.dep2(k2))then
     revr72(ipart2,k2)=(-1.0)*dep2(k2)*w0/epsilonb
     else
     revr72(ipart2,k2)=(-1.0)*dep2(k2)*w0*
   &(dmix/dip)**2.4/epsilonb_l
   endif
c**
c**
     if(surf.EQ.1)then
     rsurf2(ipart2,k2)=rs*EXP(r72(ipart2,k2))
     pns2(ipart2,k2)=(6.0/(pi*dip**3))*rsurf2(ipart2,k2)/rhos
     else
    pns2(ipart2,k2)=0
     endif
C
    if(riv.EQ.1)then
     s2riv1(k2)=sal*EXP(sal2r7(k2))
     rivsum=s2riv1(k2)
     sal2mean(k2)=salo-s2riv1(k2)
     a4402(k2)=dominter-domslope*sal2mean(k2)
     if(sal2mean(k2).GE.salmax)a4402(k2)=domback
     rriv2(ipart2,k2)=qs*EXP(revr72(ipart2,k2))
     pnr2(ipart2,k2)=(6.0/(pi*dip**3))*rriv2(ipart2,k2)/rhosr
     else
     pnr2(ipart2,k2)=0
     endif
C**
C^{**}
     if(riv2.EQ.1)then
     s2riv2(k2)=sal2*EXP(sal2r7(k2))
    rivsum=s2riv1(k2)+s2riv2(k2)
    if(rivsum.GT.salo)then
     rivsum=salo
     endif
     sal2mean(k2)=salo-rivsum
     a4402(k2)=dominter-domslope*sal2mean(k2)
     if(sal2mean(k2).GE.salmax)a4402(k2)=domback
     rriv2r2(ipart2,k2)=qs2*EXP(revr72(ipart2,k2))
     pnr2r2(ipart2,k2)=(6.0/(pi*dip**3))*rriv2r2(ipart2,k2)/rhosr2
     else
```

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    pnr2r2(ipart2,k2)=0
    endif
c
    if(bot.EQ.1)then
    rbot2(ipart2,k2)=rt*EXP(r72(ipart2,k2))
    pnb2(ipart2,k2)=(6.0/(pi*dip**3))*rbot2(ipart2,k2)/rhos
    else
    pnb2(ipart2,k2)=0
    endif
C**
C^{**}
    pnt2(ipart2,k2)=pns2(ipart2,k2)+pnr2(ipart2,k2)+pnb2(ipart2,k2)
  &+pnr2r2(ipart2,k2)
    if(pnt2(ipart2,k2).GT.9999999)pnt2(ipart2,k2)=9999999
    if(pnt2(ipart2,k2).LT.-1000)pnt2(ipart2,k2)=-55555
c
2002 continue
2005 continue
c
4001 continue
4000 continue
c
c
    fw=EXP(5.2*((2.5*dip/d0)**0.2)-6.0)
c
    delta=0.72*d0*(2.5*dip/d0)**0.25
    deltab=0.72*dxb*(2.5*dip/dxb)**0.25
    deltar=0.72*dxbave(iy)*(2.5*dip/dxbave(iy))**0.25
    deltas=0.72*dxbave(iy)*(0.000006625/dxbave(iy))**0.25
c
c.....for the outfall sediment flux use running mean values of ub *dxb
   shieldr=ubave(iy)**2/((rhos/ro-1.0)*g*dip)
   epsilonr 1=0.00035*ak3 1*((ubave(iy)/w0)**0.68)*
   &(shieldr**0.4)*deltar*g*per
   if(epsilonr_1.LT.0.0000000001)epsilonr_1 = 0.0000000001
   epsilonr=epsilonr_l+epsilonw
c.....for the accretion wave use running mean values of ub *dxb
```

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   shieldrs=ubave(iy)**2/(.03*g*dr)
   epsilonrs 1=0.00035*ak3s*((ubave(iy)/w0sal)**0.68)*
   &(shieldrs**0.4)*deltas*g*per
   if(epsilonrs_1.LT.0.0000000001)epsilonrs_1 = 0.0000000001
   epsilonrs_l+epsilonw
c
c
    shieldb=ub**2/((rhos/ro-1.0)*g*dip)
     epsilonb_l=0.00035*ak3*((ub/w0)**0.68)*
   &(shieldb**0.4)*deltab*g*per
    if(epsilonb_1.EQ.0)epsilonb_1=0.0000000001
   epsilonb=epsilonb_l+epsilonw
c
c.....offshore
    shield=u**2/((rhos/ro-1.0)*g*dip)
    epsilon_l=0.00035*ak2*((u/w0)**0.68)*
   &(shield**0.4)*delta*g*per
    if(epsilon 1.LT.0.0000000001)epsilon 1 = 0.0000000001
   epsilon=epsilon_l+epsilonw
c
c....at the local break point for salinity
     epsilonbs_l=0.00035*ak3s*((ub/w0sal)**0.68)
   &*(shieldrs**0.4)*deltas*g*per
    if(epsilonbs_1.LT.0000000001)epsilonbs_1 = 0.0000000001
   epsilonbs=epsilonbs_l+epsilonw
c
c.....outfall shields and epsilon calculated in RIVERM subroutine
c
c
c
c
    if(riv.EQ.1)then
     CALL RIVERM(epsilonr,dr,rwidth,q,ux1,uy1,ix,iy,irx,iry,sx,
   &sy,rrsfix,ak5,w0,delta,rhosr,ro,dip,g,per,ak3,time,qsold,
   &depthc,tconriv,qs)
c
    CALL SALM1(epsilonrs,dr,rwidth,q,ux1,uy1,ix,iy,irx,iry,sx,
   &sy,salo,ak5s,w0sal,deltas,rhosr,ro,dip,g,per,ak3,time,salold,
   &depthc,tconsal,sal)
c
    else
    continue
```

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     endif
c
\mathbf{C}
     if(riv2.EQ.1)then
     CALL RIVER2M(epsilonr,dr2,rwidth2,q2,ux1,uy1,ix,iy,irx2,iry2,sx,
   &sy,rrsfix2,ak5,w0,deltab,rhosr2,ro,dip,g,per,ak3,time,qs2old,
   &depthc,tconriv,qs2)
c
     CALL SALM2(epsilonrs,dr2,rwidth2,q2,ux1,uy1,ix,iy,irx2,iry2,sx,
   &sy,salo,ak5s,w0sal,deltas,rhosr2,ro,dip,g,per,ak3,time,salold2,
   &depthc,tconsal,sal2)
c
     else
     continue
     endif
c
c
     if(surf.EQ.1)then
     CALL SURFLM(epsilonb,bd1,ba1,bh1,ibpx,ix,iy,
   &ak4,aks,akb,dip,w0,pi,g,rhos,ro,tanbeta,rcspfix,time,rsold,
   &depthc,tcon,rs)
     else
     continue
     endif
c
c
     if(bot.EQ.1)then
     CALL BOTTOMM(ix,iy,u,shield,fw,ak,pi,wndep,rcfix,time,rtold,
   &depthc,tcon,rt,w0,epsilon)
     else
     continue
     endif
c
c
c
    if(verdat.EQ.1)datum=depthc
    if(verdat.NE.1)datum=delta
c
    if(verdat.EQ.1)datmod=-1.0
    if(verdat.NE.1)datmod=1.0
c
     do 200 k=1,4
     if(k.EQ.1)zw(k)=depthc-100.0
```

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     if(k.EQ.2)zw(k)=delta+100.0
    if(k.EQ.3)zw(k)=depthc-alay3
    if(k.EQ.4)zw(k)=depthc-alay4
c
    if(zw(k)).GT.depthc)zw(k)=depthc
    if(zw(k).LT.delta)zw(k)=delta
    dep(k)=depthc-zw(k)
c
c ....erosion calculations
     salmean(k)=salo
     a440(k)=dominter-domslope*salmean(k)
     if(salmean(k).GE.salmax)a440(k)=domback
     if(dmix.GE.dep(k))then
     salr7(k)=(-1.0)*dep(k)*aksal*w0sal/epsilonbs
     else
     salr7(k)=(-1.0)*dep(k)*aksal*((dimx/4.21)**2.4)
   &*w0sal/epsilonbs_1
     endif
c
     if((depthc-dmix).GE.zw(k))then
     r7(ipart,k)=(-1.0)*zw(k)*w0/epsilon_l
    r7(ipart,k)=(-1.0)*zw(k)*w0*(dmix/dip)**2.4/epsilon
c*************my fix 10-21-94********
     if(k.EQ.2)r7(ipart,k)=(-1.0)*zw(k)*w0/epsilon_l
c*************
     endif
     if((depthc-dmix).LT.0.0)r7(ipart,k)=(-1.0)
   &*zw(k)*w0/epsilon
c
     if(dmix.GE.dep(k))then
     revr7(ipart,k)=(-1.0)*dep(k)*w0/epsilonb
     revr7(ipart,k)=(-1.0)*dep(k)*w0*
   &(dmix/dip)**2.4/epsilonb_l
   endif
c**
c
    if(surf.EQ.1)then
     rsurf(ipart,k)=rs*EXP(r7(ipart,k))
     pns(ipart,k)=(6.0/(pi*dip**3))*rsurf(ipart,k)/rhos
     else
     pns(ipart,k)=0
```

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     endif
C
     if(riv.EQ.1)then
     sriv1(k)=sal*EXP(salr7(k))
     rivsum=sriv1(k)
     salmean(k)=salo-sriv1(k)
     a440(k)=dominter-domslope*salmean(k)
     if(salmean(k).GE.salmax)a440(k)=domback
     rriv(ipart,k)=qs*EXP(revr7(ipart,k))
     pnr(ipart,k)=(6.0/(pi*dip**3))*rriv(ipart,k)/rhosr
     else
    pnr(ipart,k)=0
    endif
C
\mathbf{C}
    if(riv2.EQ.1)then
     sriv2(k)=sal2*EXP(salr7(k))
    rivsum=sriv1(k)+sriv2(k)
    if(rivsum.GT.salo)then
     rivsum=salo
     endif
     salmean(k)=salo-rivsum
     a440(k)=dominter-domslope*salmean(k)
     if(salmean(k).GE.salmax)a440(k)=domback
     rrivr2(ipart,k)=qs2*EXP(revr7(ipart,k))
     pnrr2(ipart,k)=(6.0/(pi*dip**3))*rrivr2(ipart,k)/rhosr2
    else
    pnrr2(ipart,k)=0
     endif
c
\mathbf{C}
    if(bot.EQ.1)then
     rbot(ipart,k)=rt*EXP(r7(ipart,k))
     pnb(ipart,k)=(6.0/(pi*dip**3))*rbot(ipart,k)/rhos
     else
    pnb(ipart,k)=0
    endif
C
     pnt(ipart,k)=pns(ipart,k)+pnr(ipart,k)+pnb(ipart,k)
   &+pnrr2(ipart,k)
      CONTINUE
200
c
```

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1095
       FORMAT(F8.5,3x,F8.3,3x,F17.4,3x,F12.8,f12.8)
c
499
      continue
c.....sum numbers from each size bin together
    suml1(ix,iy)=pnt(ipart,1)+suml1(ix,iy)
    suml2(ix,iy)=pnt(ipart,2)+suml2(ix,iy)
    suml3(ix,iy)=pnt(ipart,3)+suml3(ix,iy)
    suml4(ix,iy)=pnt(ipart,4)+suml4(ix,iy)
c
c
    write(2, REC=icount)rt,rs,qs,qs2,sal,sal2
1776 continue
c
c
c*****************null point write*******************
c.....null point write commented out
c.....
        if(iy.NE.igrdy)go to 6005
      write(50,3000)rc(1),rc(2),rc(3),rc(4),rc(5),rc(6),rc(7),
c..
     &rc(8),rc(9),unp
c..
c..3000 format(10e10.3)
c..6005
        continue
c
c....put salinity values int x-y array
    sall1(ix,iy)=salmean(1)
    sall2(ix,iy)=salmean(2)
    sall3(ix,iy)=salmean(3)
    sall4(ix,iy)=salmean(4)
    acdom1(ix,iy)=a440(1)
    acdom2(ix,iy)=a440(2)
    acdom3(ix,iy)=a440(3)
    acdom4(ix,iy)=a440(4)
c
c
    if(suml1(ix,iy).GT.9999999)suml1(ix,iy)=9999999
    if(suml2(ix,iy).GT.9999999)suml2(ix,iy)=9999999
    if(suml3(ix,iy).GT.9999999)suml3(ix,iy)=9999999
    if(suml4(ix,iy).GT.9999999)suml4(ix,iy)=9999999
c
c
    if(suml1(ix,iy).LT.-1000)suml1(ix,iy)=-1000
```

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     if(suml2(ix,iy).LT.-1000)suml2(ix,iy)=-1000
     if(suml3(ix,iy).LT.-1000)suml3(ix,iy)=-1000
     if(sum14(ix,iy).LT.-1000)sum14(ix,iy)=-1000
c
c
c...add fractional grains to the first 5 bins at surface & bottom
     pnt(1,1)=pnt(1,1)+an0b1
    pnt(2,1)=pnt(2,1)+an0b2
    pnt(3,1)=pnt(3,1)+an0b3
     pnt(4,1)=pnt(4,1)+an0b4
    pnt(5,1)=pnt(5,1)+an0b5
    pnt(6,1)=pnt(6,1)+an0b6
    pnt(7,1)=pnt(7,1)+an0b7
    pnt(8,1)=pnt(8,1)+an0b8
    pnt(9,1)=pnt(9,1)+an0b9
c
c
     pnt(1,2)=pnt(1,2)+an0b1
    pnt(2,2)=pnt(2,2)+an0b2
    pnt(3,2)=pnt(3,2)+an0b3
     pnt(4,2)=pnt(4,2)+an0b4
    pnt(5,2)=pnt(5,2)+an0b5
     pnt(6,2)=pnt(6,2)+an0b6
     pnt(7,2)=pnt(7,2)+an0b7
    pnt(8,2)=pnt(8,2)+an0b8
     pnt(9,2)=pnt(9,2)+an0b9
c
c
c
    kounts=0
    sumsp=0
    do 1790 is=1,3
    isp1=is+1
    sp(is)=ABS((LOG(pnt(isp1,1)/pnt(is,1)))/(LOG(d(isp1)/d(is))))
    sumsp=sumsp+sp(is)
    if(sp(is).LT.0.1)go to 1789
    kounts=kounts+1
1789 continue
1790 continue
c
    kountb=0
    sumsp2=0
    do 1788 is=1,3
```

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    isp1=is+1
    sp2(is) = ABS((LOG(pnt(isp1,2)/pnt(is,2)))/(LOG(d(isp1)/d(is))))
    sumsp2=sumsp2+sp2(is)
    if(sp2(is).LT.0.1)go to 1787
    kountb=kountb+1
1787 continue
1788 continue
    b(ix,iy)=sumsp/(kounts+1)
    b2(ix,iy)=sumsp2/(kountb+1)
    if(b(ix,iy).GT.gamma)b(ix,iy)=gamma
    if(b(ix,iy).LT.1.0)b(ix,iy)=1.0
    if(b2(ix,iy).GT.gamma)b2(ix,iy)=gamma
    if(b2(ix,iy).LT.1.0)b2(ix,iy)=1.0
c
1791 continue
c
c*************selected grid loop start with ix,iy EQ igrdx igrdy***
    if(iy.NE.igrdy)go to 4002
    if(ix.NE.igrdx)go to 4003
     do 2006 k3=1,ires
c....initialize sum2(k3) to background level n0
    sum2(k3)=n0
     do 2007 ipart3=1,ibins
c
c-----Slope Calculation for selected grid point-----
c...add fractional grains to the first 5 bins at each point in water
     pnt2(1,k3)=pnt2(1,k3)+an0b1
    pnt2(2,k3)=pnt2(2,k3)+an0b2
    pnt2(3,k3)=pnt2(3,k3)+an0b3
    pnt2(4,k3)=pnt2(4,k3)+an0b4
    pnt2(5,k3)=pnt2(5,k3)+an0b5
    pnt2(6,k3)=pnt2(6,k3)+an0b6
    pnt2(7,k3)=pnt2(7,k3)+an0b7
    pnt2(8,k3)=pnt2(8,k3)+an0b8
    pnt2(9,k3)=pnt2(9,k3)+an0b9
c
c
c.....sum numbers from each size bin together for selected location
     sum2(k3)=pnt2(ipart3,k3)+sum2(k3)
    if(sum2(k3).GT.9999999)sum2(k3)=9999999
```

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2007
       continue
c
c
2006 continue
c
   do 1462 k4=ires,1,-1
c.....calculate slope of profile.....
   sumspp=0
   kountp=0
c
   do 1795 is=1,3
   isp1=is+1
   spp(is)=ABS((LOG(pnt2(isp1,k4)/
   &pnt2(is,k4)))/(LOG(d(isp1)/d(is)))
   sumspp=sumspp+spp(is)
   if(spp(is).LT.0.1)go to 1794
   kountp=kountp+1
1794 continue
1795 continue
   slope(k4)=sumspp/(kountp+1)
   if(slope(k4).GT.gamma)slope(k4)=gamma
   if(slope(k4).LT.1.0)slope(k4)=1.0
   write(99,3001)dep2(k4),char(9),sum2(k4),char(9),
  &slope(k4),char(9),sal2mean(k4),char(9),a4402(k4)
1462 continue
4003
       continue
4002
       continue
c
1801
      continue
479
      continue
    ixdum=100
    write(*,*)ixdum,iy,suml1(100,iy)
   write(*,*)epsilonb
c
1800 continue
1096 format(200f8.4)
1097 format(200f10.0)
1098 format(200f8.3)
```

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3001 format(f9.3,1a,f10.0,1a,f8.3,1a,f8.3,1a,f8.4)
c
    do 1935 iy=ny,1,-1
    write(81,1098) (sall1(ix,iy),ix=1,nx)
    write(85,1096) (acdom1(ix,iy),ix=1,nx)
    write(82,1098) (sall2(ix,iy),ix=1,nx)
    write(86,1096) (acdom2(ix,iy),ix=1,nx)
    if(two_layer.NE.0)then
    write(83,1098) (sall3(ix,iy),ix=1,nx)
    write(87,1096) (acdom3(ix,iy),ix=1,nx)
    write(84,1098) (sall4(ix,iy),ix=1,nx)
    write(88,1096) (acdom4(ix,iy),ix=1,nx)
    endif
1935
       continue
c
    do 1802 ix=1,nx
    write(41,1097) (suml1(ix,iy),iy=1,ny)
    write(91,1098) (b(ix,iy),iy=1,ny)
    write(42,1097) (suml2(ix,iy),iy=1,ny)
    write(92,1098) (b2(ix,iy),iy=1,ny)
c
    if(two_layer.NE.0)then
    write(43,1097) (suml3(ix,iy),iy=1,ny)
    write(44,1097) (suml4(ix,iy),iy=1,ny)
    endif
c
1802 continue
c
    stop
    END
SUBROUTINE RIVERM(epsilonr,dr,rwidth,q,ux1,uy1,ix,iy,irx,iry,sx,
   &sy,rrsfix,ak5,w0,deltab,rhosr,ro,dip,g,per,ak3,time,qsold,
   &depthc,tconriv,qs)
c
c
    drc=dr*100.0
    rwidthc=rwidth*100.0
    qc=q*1000000.0
c
```

```
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c...modification to produce sediment plots using variable current
c.....(RADIANS).....
c
    uvdir=ATAN2(uy1,ux1)
    rxyr = ((iry-iy+.000001)*sy)/(ABS(irx-ix+.000001)*sx)
   thta=uvdir+ATAN(rxyr)
   ut=qc/(rwidthc*drc)
   ur=ut*COS(thta)-((ux1**2+uy1**2)**0.5)*SIN(thta)
c.....
c
c
   e2r=ak5*(ur-(ur**2+4*w0*epsilonr)**0.5)/(2*epsilonr)
c
   rsr1=(ABS(ut*rrsfix))/(ut**2+4*w0*epsilonr)**0.5
   rsr3=e2r*((((irx-ix)*sx)**2)+(((iry-iy)*sy)**2))**0.5
   rsr2=EXP(rsr3)
   timedec=(time*tconriv*w0/depthc)
   if(timedec.GT.1.0)timedec=1.0
   timedec2=0.03116*(ux1**2+uy1**2)**0.5
   qsnew=rsr1*rsr2
   qfosl=qsold*(1-timedec)
   if(qsnew.GT.qfosl)then
   qs=qsnew+qfosl
   else
   qs=qsnew+qfosl*EXP(-1.0*timedec2)
   endif
   return
   end
c
c
c
   SUBROUTINE SURFLM(epsilonb,bd1,ba1,bh1,ibpx,ix,iy,
  &ak4,aks,akb,dip,w0,pi,g,rhos,ro,tanbeta,rcspfix,time,
  &rsold,depthc,tcon,rs)
c
c******************Begin Surfl Subroutine*************
c
   us=ABS((g*bd1)**0.5*COS(ba1)**2)
   e2=(us-(us**2+4*ak4*w0*epsilonb)**0.5)/(2*epsilonb)
   denom=pi*dip**3*(rhos-ro)*akb*bd1**2*9240.0/tanbeta
```

rn=(24*aks*ro*bh1**2*(g*bd1)**0.5)/denom

```
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   rr=pi*dip**3*rn*rhos/6
   rs1 = (rr/(us**2+4*w0*epsilonb)**0.5)
   rs3=(e2*ABS(ibpx-ix)*77.5)
   rs2=EXP(rs3)
   rs=rs1*rs2*rcspfix+rsold*(1-(EXP(-1.0*depthc*tcon/(2*time*w0))))
c
   return
   end
c
c
   SUBROUTINE BOTTOMM(ix,iy,u,shield,fw,ak,pi,wndep,rcfix,time,
  &rtold,depthc,tcon,rt,w0,epsilon)
c
c
    if (u.LT..000001) then
    nomo=1
    rt1=0.0
    go to 399
    endif
c
   rt5=shield*fw
   if(rt5.le..05)then
   nomo=1
   rt1=0.0
   go to 399
   endif
c
c
   rt2=(0.05/(rt5))**.5
   rt4=ACOS(rt2)
   rt1=ak*(fw*shield-0.05)*(2/pi)*rt4
399 continue
   rt=rt1+(rtold*(1-(EXP(-1.0*depthc*tcon/(2*time*w0)))))
c*********rcfix section was commented out Sept 12, 1994 10:00
   if(rt.GT.rcfix)then
   rt1=rcfix
   rt=rt1+(rtold*(1-(EXP(-1.0*depthc*tcon/(2*time*w0)))))
   endif
c
```

```
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c
   return
   end
SUBROUTINE RIVER2M(epsilonr,dr2,rwidth2,q2,ux1,uy1,ix,iy,irx2,iry2
  &,sx,sy,rrsfix2,ak5,w0,deltab,rhosr2,ro,dip,g,per,ak3,time,
  &qs2old,depthc,tconriv,qs2)
c
c
   drc=dr2*100.0
   rwidthc=rwidth2*100.0
    qc=q2*1000000.0
c
c...modification to produce sediment plots using variable current
c.....(RADIANS).....
c
   uvdir=ATAN2(uy1,ux1)
   rxyr = ((iry2-iy+.000001)*sy)/(ABS(irx2-ix+.000001)*sx)
   thta=uvdir+ATAN(rxyr)
   ut=qc/(rwidthc*drc)
   ur=ut*COS(thta)-((ux1**2+uy1**2)**0.5)*SIN(thta)
c.....
   e2r=ak5*(ur-(ur**2+4*w0*epsilonr)**0.5)/(2*epsilonr)
c
   rsr1=(ABS(ut*rrsfix2))/(ut**2+4*w0*epsilonr)**0.5
   rsr3=e2r*((((irx2-ix)*sx)**2)+(((iry2-iy)*sy)**2))**0.5
   rsr2=EXP(rsr3)
   timedec=(time*tconriv*w0/depthc)
   if(timedec.GT.1.0)timedec=1.0
   timedec2=0.03116*(ux1**2+uy1**2)**0.5
   qsnew=rsr1*rsr2
   qfosl=qs2old*(1-timedec)
   if(qsnew.GT.qfosl)then
   qs2=qsnew+qfosl
   else
   qs2=qsnew+qfos1*EXP(-1.0*timedec2)
   endif
c
```

return

```
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   end
c
c
SUBROUTINE SALM1(epsilonrs,dr,rwidth,q,ux1,uy1,ix,iy,irx,iry,sx,
  &sy,salo,ak5s,w0sal,deltas,rhosr,ro,dip,g,per,ak3,time,salold,
  &depthc,tconsal,sal)
c
c
    drc=dr*100.0
    rwidthc=rwidth*100.0
    qc=q*1000000.0
c
c...modification to produce sediment plots using variable current
c.....(RADIANS).....
    uvdir=ATAN2(uy1,ux1)
    rxyr = ((iry-iy+.000001)*sy)/(ABS(irx-ix+.000001)*sx)
   thta=uvdir+ATAN(rxyr)
   ut=qc/(rwidthc*drc)
   ur=ut*COS(thta)-((ux1**2+uy1**2)**0.5)*SIN(thta)
c
c
   e2r=ak5s*(ur-(ur**2+4*w0sal*epsilonrs)**0.5)/(2*epsilonrs)
c
   rsr1=(ABS(ut*salo))/(ut**2+4*w0sal*epsilonrs)**0.5
   rsr3=e2r*((((irx-ix)*sx)**2)+(((iry-iy)*sy)**2))**0.5
   rsr2=EXP(rsr3)
   timedec=(time*tconsal*w0sal/depthc)
   if(timedec.GT.1.0)timedec=1.0
   timedec2=0.03116*(ux1**2+uy1**2)**0.5
   snew=rsr1*rsr2
   sfosl=salold*(1-timedec)
   if(snew.GT.sfosl)then
   sal=snew+sfosl
   else
   sal=snew+sfosl*EXP(-1.0*timedec2)
   endif
   return
   end
```

```
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c
c
c*******End Accretion Outfall 1 Subroutine**********
c
c************Salinity Outfall2 Subroutine***************
   SUBROUTINE SALM2(epsilonrs,dr2,rwidth2,q2,ux1,uy1,ix,iy,irx2,iry2
  &,sx,sy,salo,ak5s,w0sal,deltas,rhosr2,ro,dip,g,per,ak3,time,
  &salold2,depthc,tconsal,sal2)
c
c
    drc=dr2*100.0
    rwidthc=rwidth2*100.0
    qc=q2*1000000.0
c
c...modification to produce sediment plots using variable current
c.....(RADIANS).....
    uvdir=ATAN2(uy1,ux1)
    rxyr = ((iry2-iy+.000001)*sy)/(ABS(irx2-ix+.000001)*sx)
   thta=uvdir+ATAN(rxyr)
   ut=qc/(rwidthc*drc)
   ur=ut*COS(thta)-((ux1**2+uy1**2)**0.5)*SIN(thta)
c.....
c
c
   e2r=ak5s*(ur-(ur**2+4*w0sal*epsilonrs)**0.5)/(2*epsilonrs)
c
   rsr1=(ABS(ut*salo))/(ut**2+4*w0sal*epsilonrs)**0.5
   rsr3=e2r*((((irx2-ix)*sx)**2)+(((iry2-iy)*sy)**2))**0.5
   rsr2=EXP(rsr3)
   timedec=(time*tconsal*w0sal/depthc)
   if(timedec.GT.1.0)timedec=1.0
   timedec2=0.03116*(ux1**2+uy1**2)**0.5
   snew=rsr1*rsr2
   sfosl=salold2*(1-timedec)
   if(snew.GT.sfosl)then
   sal2=snew+sfosl
   else
   sal2=snew+sfosl*EXP(-1.0*timedec2)
   endif
c
   return
```

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APPENDIX F CODE FOR THE MULTINODE DILUTION MODEL

Appendix F: Code for the Multinode Dilution Model

```
multinode.for - 18 June 2004
c**with verticle loop
c reads in raster 200 row 200 column arrays of:
c depths, wave heights, wave numbers, x- and y-directed current components
c Surface and Bottom Salinity & caluculates Dilution from these
          written by Scott A. Jenkins & Joseph Wasyl
c
c
    character*26 infile1,infile2,infile4,infile5
c
    DIMENSION depth(200,200), wht(200,200), wvn(200,200)
    DIMENSION xcur(200,200),ycur(200,200),col(200,200)
c
   dimension z(5),s(5),ps1(5),ps1b(5),ps1bav(5)
   integer ipt(5), ix(5), iy(5)
   dimension pnx(200,200),pny(200,200),pn(200,200),pn_l(200,200)
    dimension brx(200,200),bry(200,200),brn(200,200),dmix(200,200)
    dimension salsur(200,200),salbrn(200,200),saldep(200,200)
   dimension dilriv(200,200),dildep(200,200),colriv(200,200)
   dimension dilbrn(200,200),dbrnav(200,200),sbrnav(200,200)
   dimension brxav(200,200),bryav(200,200),brnav(200,200)
   character*12 ifile,ofile1,ofile2,ofile3,ofile4,ofile5,ofile6
   character*12 ofile7,ofile8,ofile9,ofil10,ofil11,ofil12,ofil13
c
   open(42,file='multinode_agua_3.inp',status='old')
    read(42,3000)ifile
    read(42,3000)infile1
    read(42,3111)infile2
    read(42,3200)infile4
    read(42,3200)infile5
    read(42,3000)ofile1
    read(42,3000)ofile2
    read(42,3000)ofile3
    read(42,3000)ofile4
    read(42,3000)ofile5
    read(42,3000)ofile6
    read(42,3000)ofile7
    read(42,3000)ofile8
```

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read(42,3000)ofile9

read(42,3000)ofil10

read(42,3000)ofil11

read(42,3000)ofil12

read(42,3000)ofil13

read(42,*)num

read(42,*)imax

read(42,*)jmax

read(42,*)srivx1

read(42,*)srivx2

read(42,*)srivx3

read(42,*)srivx4

read(42,*)srivx5

read(42,*)srivy1

read(42,*)srivy2

read(42,*)srivy3

read(42,*)srivy4

read(42,*)srivy5

read(42,*)ub

read(42,*)vb

read(42,*)u2

read(42,*)v2

read(42,*)sx

read(42,*)sy

read(42,*)akx1_1

read(42,*)aky1_1

read(42,*)akx1_2

read(42,*)aky1_2

read(42,*)akx1_3

read(42,*)aky1_3

read(42,*)akx1 4

read(42,*)aky1_4

read(42,*)akx1_5

read(42,*)aky1_5

read(42,*)ak2_1

read(42,*)ak2_2

read(42,*)ak2_3

read(42,*)ak2 4

read(42,*)ak2_5

read(42,*)akz_1

read(42,*)akz_2

read(42,*)akz_3

read(42,*)akz 4

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read(42,*)akz_5

read(42,*)thresh

read(42,*)back

read(42,*)salo

read(42,*)cline

read(42,*)isal

read(42,*)pnman

read(42,*)upper

read(42,*)brine

read(42,*)upperb

read(42,*)ibrn

read(42,*)brnman

read(42,*)per

read(42,*)ruf

read(42,*)rhos

read(42,*)wind

read(42,*)deglat

read(42,*)ibadx

read(42,*)ibady

read(42,*)qsa

read(42,*)qtc

read(42,*)widsa

read(42,*)widtc

read(42,*)akr_3

read(42,*)akr_4

read(42,*)u_blend

read(42,*)aksal

read(42,*)surdz

read(42,*)botdz

read(42,*)shrdec

read(42,*)swrdec

read(42,*)surmix

read(42,*)aksur

read(42,*)surhb

read(42,*)thermo

read(42,*)xmag

read(42,*)ymag

read(42,*)akocs

read(42,*)depo

read(42,*)thrmdz

read(42,*)coli

read(42,*)deep

read(42,*)akzav

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c
    rawbrn=2.0*salo
C**read in oceanbat .grd, oceanrds_200x200_raster and tidecur_agua arrays
   OPEN(UNIT=51,FILE=infile1,STATUS='OLD')
   OPEN(UNIT=52,FILE=infile2,STATUS='OLD')
   OPEN(UNIT=54,FILE=infile4,STATUS='OLD')
   OPEN(UNIT=55,FILE=infile5,STATUS='OLD')
\mathbf{C}
   DO 8001 J=1,200
   READ(51,*)(depth(I,J),I=1,200)
   READ(52,*)(wht(I,J),I=1,200)
   READ(54,*)(xcur(I,J),I=1,200)
   READ(55,*)(ycur(I,J),I=1,200)
8001 CONTINUE
c
3000 format(a12)
3111
      format(a26)
3200
      format(a22)
c
    surhb=surhb*100.0
    surdz=surdz*100.0
    botdz=botdz*100.0
    cline=cline*100.0
    depo=depo*100.0
    thrmdz=thrmdz*100.0
    freq=1.0/per
    pi=ACOS(-1.0)
    g = 980.0
    rho=1.03
    sigma=2.0*pi*freq
    deepl=2.0*pi*g/(sigma**2.0)
    wind=wind*0.5148*100.0
    ak8=0.00009/((wind**0.5)+1.1103)
    ustar=(ak8*(wind**2))**0.5
    rot_e=0.000072685
    f=2.0*rot_e*SIND(deglat)
    dmix test=4.0*ustar/(f*100.0)
c.....epsilonw is in cgs units
    epsilonw=0.0000043*(wind**2.0)
    if(wind.LE.600)epsilonw=0.0000000102*(wind**3.0)
c
c
```

```
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   open(20,file=ifile,status='old')
    open(21,file=ofile1,status='unknown')
    open(22,file=ofile2,status='unknown')
   open(23,file=ofile3,status='unknown')
    open(24,file=ofile4,status='unknown')
    open(25,file=ofile5,status='unknown')
    open(26,file=ofile6,status='unknown')
   open(27,file=ofile7,status='unknown')
   open(28,file=ofile8,status='unknown')
   open(29,file=ofile9,status='unknown')
   open(30,file=ofil10,status='unknown')
    open(31,file=ofil11,status='unknown')
    open(32,file=ofil12,status='unknown')
   open(33,file=ofil13,status='unknown')
c
c
c... open node file
     do 100 i=1,num
    read(20,1000) ipt(i),ix(i),iy(i),z(i)
100
       continue
1000
       format(i2,2i4,f7.2)
c..covert x and y coordinates to nearest integer
c..calculate or assign strength to each source
c**initialize particle output array
     do 870 iay=1,jmax
     do 860 iax=1,imax
     pn(iax,iay)=0.0
     pnx(iax,iay)=0.0
     pny(iax,iay)=0.0
     brn(iax,iay)=0.0
     brx(iax,iay)=0.0
     bry(iax,iay)=0.0
     brnav(iax,iay)=0.0
     brxav(iax,iay)=0.0
     bryav(iax,iay)=0.0
     xcur(iax,iay)=xmag*xcur(iax,iay)
     ycur(iax,iay)=ymag*ycur(iax,iay)
c
     depth(iax,iay)=depth(iax,iay)*100.0
```

c**make land in bays negative in increasing iax direction

if(iland.EQ.1)depth(iax,iay)=-200.0

```
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    if(depth(iax,iay).LE.0.0)then
    iland=1
    depth(iax,iay)=-200.0
    endif
c
c**dispersion routine for wave number
c.....initialize wave number array
     wvn(iax,iay)=-1.0
     amp2=0.05*deepl
     if(depth(iax,iay).LE.0)go to 1090
     b=(sigma**2*depth(iax,iay))/g
     if(depth(iax,iay).LT.amp2)then
      wvn(iax,iay)=sigma/(g*depth(iax,iay))**0.5
      go to 1090
      else
      endif
      a=b
     do 1030 k5=1,100
     h=tanh(a)
     fjw=b-a*h
     if (abs(fjw) .lt. 0.000001) go to 1040
     fd=-1.0*h-(a/cosh(a)**2)
     a=a-(fjw/fd)
1030 continue
     write(*,1050)
      format(' subroutine disp does not converge!!! ')
1050
        wvn(iax,iay)=a/depth(iax,iay)
1040
1090
       continue
CEND DISPERSION SUBROUTINE
    wht(iax,iay)=wht(iax,iay)*100.0
c
860
      continue
    iland=0
870
      continue
c&&&&&&&&&
    do 200 i=1,num
    ps1b(i)=0.0
    ps1bav(i)=0.0
200
      continue
cPick each Source, use x and y location arrays ix(i), iy(i) of source
    do 799 i=1,num
```

```
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cPick each x location of grid(iax) solve dx relative to x location of source ix(i)
     do 800 iax=1,imax
     dx = (iax - ix(i))*sx
     do 850 iay=1,jmax
     dmix(iax,iay)=4.0*ustar/(f*100.0)
    if(dmix(iax,iay).LE.cline)dmix(iax,iay)=cline
     if(depth(iax,iay).GT.0.0.AND.xcur(iax,iay).EQ.0.0)
   &xcur(iax,iay)=0.00001
     if(depth(iax,iay).GT.0.0.AND.ycur(iax,iay).EQ.0.0)
   &ycur(iax,iay)=0.00001
     dy=(iay-iy(i))*sy
     dr = ((dx**2.0) + (dy**2.0))**0.5
    drxy = ((sx**2.0) + (sy**2.0))**0.5
    if(dr.LT.drxy)dr=drxy
     theta=ABS(dx/dr)
     d0=wht(iax,iay)/SINH(wvn(iax,iay)*depth(iax,iay))+0.001
     uwav=sigma*d0
     ucur = (xcur(iax,iay)**2.0+ycur(iax,iay)**2.0)**0.5
     u=ucur+uwav
C----
    fw=EXP(5.2*((2.5*ruf/d0)**0.2)-6.0)
     delta=0.72*d0*(2.5*ruf/d0)**0.25
    shield=(u**2/((rhos/rho-1.0)*g*ruf))*fw
C*********SIO_1 OUTFALL X-LOOP (ipt=1) lat=33 39' 19"N, lon=117 58' 57"W*****
     if(ipt(i).EQ.1)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
    ps1b(i)=0.0
    ps1bav(i)=0.0
    etr=0.0
    etrb=0.0
     go to 4521
    endif
     s(i)=srivx1
     ak1=akx1_1
     ak2=ak2_1
     surdec=-1.0*((depth(iax,iay)/surhb)**aksur)
     epsur=surmix*(EXP(surdec))
c*U_BLEND logic
     uave=(u blend*ucur+u)/(u blend+1)
     dxb=uave*per
```

```
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     deltar=0.72*dxb*(2.5*ruf/dxb)**0.25
    fw=EXP(5.2*((2.5*ruf/dxb)**0.2)-6.0)
     shield=(uave**2/((rhos/rho-1.0)*g*ruf))*fw
    epsilon_l=0.00035*ak2*(uave**0.68)*(shield**0.4)*deltar*g*per
    if(xcur(iax,iay).LT.0.0)then
    uwav=-1.0*uave
    else
    uwav=uave
    endif
    u0=xcur(iax,iay)+uwav+ub
    u0abs = ABS(u0)
c
    if(epsilon_1.EQ.0.0)epsilon_1=0.0000000001
    eps=epsilon l+epsilonw
     depmdz=depth(iax,iay)-botdz
    botx = -1.0*akz_1*(botdz**1.0)/eps
     botav=-1.0*akzav*(depth(iax,iay)**1.0)/eps
    decbot=EXP(botx)
    decav=EXP(botav)
    if(dmix(iax,iay).GE.depth(iax,iay).AND.depth(iax,iay).GT.0.0)
   &dmix(iax,iay)=depth(iax,iay)
    if(dmix(iax,iay).GT.depmdz)decbot=decbot**shrdec
    ps1b(i)=s(i)*decbot/((uave**2+4.0*ruf*(eps+epsur))**0.5)
    ps1bav(i)=ps1b(i)*decav/decbot
c SIO_1 Inshore
    if(iax.EQ.237.AND.iay.EQ.133)write(*,*)iax,iay,epsilon l,epsur,
   &ps1b(i),etrb
c SIO_1 Inshore
    if(iax.EQ.173.AND.iay.EQ.96)write(*,*)iax,iay,epsilon_l,epsur,
   &ps1b(i),etrb
c SIO 1 Inshore
    if(iax.EQ.179.AND.iay.EQ.94)write(*,*)iax,iay,epsilon_l,epsur,
   &ps1b(i),etrb
c SIO 1 Inshore
    if(iax.EQ.163.AND.iay.EQ.97)write(*,*)iax,iay,epsilon l,epsur,
   &ps1b(i),etrb
    if(u0.GE.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*((u0-theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.GE.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*((u0+theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.LT.0.0)then
```

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    etr=-1.0*ak1*(-1.0*(u0-theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0+theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    au0=ABS(u0)
    if(dx.EQ.0.0)then
    etr=-1.0*ak1*(au0+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
c
    if(u0.GE.0.0.AND.dx.GT.0.0)then
    etrb=-1.0*ak1*((u0-theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    if(u0.GE.0.0.AND.dx.LT.0.0)then
    etrb=-1.0*ak1*((u0+theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.LT.0.0)then
    etrb=-1.0*ak1*(-1.0*(u0-theta*u0)+(4.0*ruf*eps)**0.5)/
  \&(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.GT.0.0)then
    etrb=-1.0*ak1*(-1.0*(u0+theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    au0=ABS(u0)
    if(dx.EQ.0.0)then
    etrb=-1.0*ak1*(au0+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    endif
4521 continue
c************************END
                                            SIO_1
                                                                                        X-
                                                                OUTFALL
LOOP****************
C*******SIO_3 OUTFALL X-LOOP (ipt=2) *********************
    if(ipt(i).EQ.2)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
```

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    ps1b(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4522
    endif
    if(xcur(iax,iay).LT.0.0)uwav=-1.0*uwav
    u0=xcur(iax,iay)+uwav+u2
    s(i)=srivx2
    ak1=akx1_2
     ak2=ak2 2
    epsilon_l=0.00035*ak2*(u**0.68)*(shield**0.4)*delta*g*per
    if(epsilon_l.EQ.0.0)epsilon_l=0.0000000001
    eps=epsilon_l+epsilonw
    dmxmdz=dmix(iax,iay)-thrmdz
    if(thrmdz.NE.0)then
    if(dmxmdz.GT.depth(iax,iay).AND.depth(iax,iay).GT.0.0)
   &thrmdz=dmxmdz-depth(iax,iay)+thrmdz
    endif
    thrmx = -1.0*akz_2*(thrmdz**1.0)
    dcthrm=EXP(thrmx)
    if(depth(iax,iay).LE.depo)then
     ps1(i)=(s(i)*dcthrm/((u**2+4.0*ruf*eps)**0.5))*
   &((depth(iax,iay)/depo)**akocs)
    else
     ps1(i)=(s(i)*dcthrm/((u**2+4.0*ruf*eps)**0.5))*
   &((depth(iax,iay)/depo)**(akocs/deep))
    endif
    if(u0.GE.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*((u0-theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.GE.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*((u0+theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0-theta*u0)+((4.0*ruf*eps)**0.5))/
   \&(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0+theta*u0)+((4.0*ruf*eps)**0.5))/
   \&(2.0*eps)
    endif
    au0=ABS(u0)
```

if(dx.EQ.0.0)then

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    etr=-1.0*ak1*(au0+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    endif
4522 continue
c******END
                                                                OUTFALL
                                                                                        X-
                                            SIO 3
LOOP******************
C********SIO 2 Outfall X-LOOP (ipt=3) ***********************
    if(ipt(i).EQ.3)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
    ps1b(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4523
    endif
    drc=z(i)*100.0
    wid=widsa*100.0
    qc=qsa*92903.0
    uvdir=ATAN2(ycur(iax,iay),xcur(iax,iay))
    ut0=qc/(drc*wid)
    ut=ut0/((((dx+0.00001)**2+(dy+0.00001)**2)**0.5)**akr_3)
    u0=xcur(iax,iay)-ut*COS(uvdir)
    uave=(u_blend*ucur+u)/(u_blend+1)
    dxb=uave*per
    deltar=0.72*dxb*(2.5*ruf/dxb)**0.25
    fw=EXP(5.2*((2.5*ruf/dxb)**0.2)-6.0)
    shield=(uave**2/((rhos/rho-1.0)*g*ruf))*fw
    s(i)=srivx3
    ak1=akx1 3
    ak2=ak2_3
    epsilon_l=0.00035*ak2*(uave**0.68)*(shield**0.4)*deltar*g*per
    if(epsilon 1.EQ.0.0)epsilon 1=0.0000000001
    eps=epsilon_l+epsilonw
c***diagnostic
CCCCCCC
               if(iay.EQ.118)write(*,*)iax,iay,ur,shield,deltar,eps
c***diagnostic
    surx=-1.0*akz 3*(surdz**1.0)
    decsur=EXP(surx)
    if(dmix(iax,iay).GE.depth(iax,iay).AND.depth(iax,iay).GT.0.0)
  &dmix(iax,iay)=depth(iax,iay)
    if(dmix(iax,iay).LT.surdz)decsur=decsur**thermo
```

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    ps1(i)=s(i)*decsur/((u0**2+4.0*ruf*eps)**0.5)
    if(u0.GE.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*((u0-theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    if(u0.GE.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*((u0+theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0-theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0+theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    au0=ABS(u0)
    if(dx.EQ.0.0)then
    etr=-1.0*ak1*(au0+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    endif
4523 continue
C*******SIO 4 OUTFALL X-LOOP (ipt=4) ************************
    if(ipt(i).EQ.4)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
    ps1b(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4524
    endif
    drc=z(i)*100.0
    wid=widtc*100.0
    qc=qtc*92903.0
    uvdir=ATAN2(ycur(iax,iay),xcur(iax,iay))
    ut0=qc/(drc*wid)
    ut=ut0/((((dx+0.00001)**2+(dy+0.00001)**2)**0.5)**akr 4)
    u0=xcur(iax,iay)-ut*COS(uvdir)
    uave=(u_blend*ucur+u)/(u_blend+1)
    dxb=uave*per
    deltar=0.72*dxb*(2.5*ruf/dxb)**0.25
```

```
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    fw=EXP(5.2*((2.5*ruf/dxb)**0.2)-6.0)
    shield=(uave**2/((rhos/rho-1.0)*g*ruf))*fw
    s(i)=srivx4
    ak1=akx1_4
    ak2=ak2 4
    epsilon 1=0.00035*ak2*(uave**0.68)*(shield**0.4)*deltar*g*per
    if(epsilon_1.EQ.0.0000000001)epsilon_1=0.0000000001
    eps=epsilon_l+epsilonw
c***diagnostic
CCCCCCC
               if(iay.EQ.118)write(*,*)iax,iay,ur,shield,deltar,eps
c***diagnostic
    surx = -1.0*akz_4*(surdz**2.0)/eps
    decsur=EXP(surx)
    if(dmix(iax,iay).GE.depth(iax,iay).AND.depth(iax,iay).GT.0.0)
  &dmix(iax,iay)=depth(iax,iay)
    if(dmix(iax,iay).LT.surdz)decsur=decsur**thermo
    ps1(i)=s(i)*decsur/((u0**2+4.0*ruf*eps)**0.5)
    if(u0.GE.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*((u0-theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.GE.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*((u0+theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0-theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0+theta*u0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    au0=ABS(u0)
    if(dx.EQ.0.0)then
    etr=-1.0*ak1*(au0+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    endif
4524 continue
C*******SIO_ Offshore OUTFALL X-LOOP (ipt=5) ***********************************
    if(ipt(i).EQ.5)then
    if(depth(iax,iay).LT.10.0)then
```

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    ps1(i)=0.0
    ps1b(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4525
    endif
    if(xcur(iax,iay).LT.0.0)uwav=-1.0*uwav
    u0=xcur(iax,iay)+uwav
    s(i)=srivx5
    ak1=akx1 5
    ak2=ak2 5
    epsilon_l=0.00035*ak2*(u**0.68)*(shield**0.4)*delta*g*per
    if(epsilon_1.EQ.0.0000000001)epsilon_1=0.0000000001
    eps=epsilon l+epsilonw
    ps1(i)=s(i)/((u**2+4.0*ruf*eps)**0.5)
    if(u0.GE.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*((u0-theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.GE.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*((u0+theta*u0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.LT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0-theta*u0)+((4.0*ruf*eps)**0.5))/
   \&(2.0*eps)
    endif
    if(u0.LT.0.0.AND.dx.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(u0+theta*u0)+((4.0*ruf*eps)**0.5))/
   \&(2.0*eps)
    endif
    au0=ABS(u0)
    if(dx.EQ.0.0)then
    etr=-1.0*ak1*(au0+((4.0*ruf*eps)**0.5))/
   \&(2.0*eps)
    endif
    endif
4525 continue
c******END
                                        SIO
                                                    Offshore
                                                                     OUTFALL
                                                                                        X-
LOOP****************
c
    ps3=etr*((dx**2)+(dy**2))**0.5
    ps3b=etrb*((dx**2)+(dy**2))**0.5
c***
    ps2=EXP(ps3)
```

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    ps2b=EXP(ps3b)
    pnx(iax,iay)=pnx(iax,iay)+(ps1(i)*ps2)
    brx(iax,iay)=brx(iax,iay)+(ps1b(i)*ps2b)
    brxav(iax,iay)=brxav(iax,iay)+(ps1bav(i)*ps2b)
c
c
850
      continue
      continue
800
799
      continue
C******** LOOPS *******
cPick each Source, use x and y location arrays ix(i), iy(i) of source
     do 1799 i=1,num
cPick each x location of grid(iax) solve dx relative to x location of source ix(i)
    do 1800 iay=1,jmax
    dy=(iay-iy(i))*sy
c
    do 1850 iax=1,imax
    dmix(iax,iay)=4.0*ustar/(f*100.0)
    if(dmix(iax,iay).LE.cline)dmix(iax,iay)=cline
    if(iax.EQ.ibadx.AND.iay.EQ.ibady)then
    ps1(i)=0.0
    ps1b(i)=0.0
    ps1bav(i)=0.0
    etr=0.0
    etrb=0.0
    go to 1777
    endif
    if(depth(iax,iay).GT.0.0.AND.xcur(iax,iay).EQ.0.0)
   &xcur(iax,iay)=0.00001
    if(depth(iax,iay).GT.0.0.AND.ycur(iax,iay).EQ.0.0)
   &ycur(iax,iay)=0.00001
    dx=(iax-ix(i))*sx
    dr = ((dx**2.0) + (dy**2.0))**0.5
    drxy = ((sx**2.0) + (sy**2.0))**0.5
    if(dr.LT.drxy)dr=drxy
    beta=ABS(dy/dr)
c
    d0=wht(iax,iay)/SINH(wvn(iax,iay)*depth(iax,iay))+0.001
     uwav=sigma*d0
     ucur = (xcur(iax,iay)**2.0+ycur(iax,iay)**2.0)**0.5
    u=ucur+uwav
```

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C----
    fw=EXP(5.2*((2.5*ruf/d0)**0.2)-6.0)
    delta=0.72*d0*(2.5*ruf/d0)**0.25
    shield=(u**2/((rhos/rho-1.0)*g*ruf))*fw
C----
c**********SIO 1 OUTFALL Y-LOOP************************
    if(ipt(i).EQ.1)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
    ps1b(i)=0.0
    ps1bav(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4530
    endif
    s(i)=srivy1
    ak1=aky1_1
    ak2=ak2 1
    surdec=-1.0*((depth(iax,iay)/surhb)**aksur)
    epsur=surmix*(EXP(surdec))
c* U BLEND logic
    uave=(u_blend*ucur+u)/(u_blend+1)
    dxb=uave*per
    deltar=0.72*dxb*(2.5*ruf/dxb)**0.25
    fw=EXP(5.2*((2.5*ruf/dxb)**0.2)-6.0)
    shield=(uave**2/((rhos/rho-1.0)*g*ruf))*fw
    epsilon_l=0.00035*ak2*(uave**0.68)*(shield**0.4)*deltar*g*per
    if(ycur(iax,iay).LT.0.0)then
    uwav=-1.0*uave
    else
    uwav=uave
    endif
    v0=ycur(iax,iay)+uwav+vb
    v0abs = ABS(v0)
c
    if(epsilon_1.EQ.0.0)epsilon_1=0.0000000001
    eps=epsilon_l+epsilonw
    depmdz=depth(iax,iay)-botdz
    botx=-1.0*akz_1*(botdz**1.0)/eps
    botav=-1.0*akzav*(depth(iax,iay)**1.0)/eps
    decbot=EXP(botx)
    decav=EXP(botav)
```

```
if(dmix(iax,iay).GE.depth(iax,iay).AND.depth(iax,iay).GT.0.0)
&dmix(iax,iay)=depth(iax,iay)
 if(dmix(iax,iay).GT.depmdz)decbot=decbot**shrdec
 ps1b(i)=s(i)*decbot/((uave**2+4.0*ruf*(eps+epsur))**0.5)
 ps1bav(i)=ps1b(i)*decav/decbot
 if(v0.GE.0.0.AND.dy.GT.0.0)then
 etr=-1.0*ak1*((v0-beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
 endif
 if(v0.GE.0.0.AND.dy.LT.0.0)then
 etr=-1.0*ak1*((v0+beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
 endif
 if(v0.LT.0.0.AND.dy.LT.0.0)then
 etr=-1.0*ak1*(-1.0*(v0-beta*v0)+((4.0*ruf*eps)**0.5))/
\&(2.0*eps)
 endif
 if(v0.LT.0.0.AND.dy.GT.0.0)then
 etr=-1.0*ak1*(-1.0*(v0+beta*v0)+((4.0*ruf*eps)**0.5))/
\&(2.0*eps)
 endif
 av0=ABS(v0)
 if(dy.EQ.0.0)then
 etr=-1.0*ak1*(av0+((4.0*ruf*eps)**0.5))/
\&(2.0*eps)
 endif
 if(v0.GE.0.0.AND.dy.GT.0.0)then
 etrb=-1.0*ak1*((v0-beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
 endif
 if(v0.GE.0.0.AND.dy.LT.0.0)then
 etrb=-1.0*ak1*((v0+beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
 endif
 if(v0.LT.0.0.AND.dy.LT.0.0)then
 etrb=-1.0*ak1*(-1.0*(v0-beta*v0)+((4.0*ruf*eps)**0.5))/
\&(2.0*eps)
 endif
 if(v0.LT.0.0.AND.dy.GT.0.0)then
 etrb = -1.0*ak1*(-1.0*(v0+beta*v0)+(4.0*ruf*eps)**0.5)/
\&(2.0*eps)
 endif
 av0=ABS(v0)
 if(dy.EQ.0.0)then
 etrb=-1.0*ak1*(av0+(4.0*ruf*eps)**0.5)/
\&(2.0*eps)
```

c

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    endif
    endif
4530 continue
c*********SIO 3 OUTFALL Y-LOOP*******************
    if(ipt(i).EQ.2)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
    ps1b(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4531
    endif
    if(ycur(iax,iay).LT.0.0)uwav=-1.0*uwav
    v0=ycur(iax,iay)+uwav+v2
    s(i)=srivy2
    ak1=aky1 2
    ak2=ak2 2
    epsilon_l=0.00035*ak2*(u**0.68)*(shield**0.4)*delta*g*per
    if(epsilon_l.EQ.0.0000000001)epsilon_l=0.0000000001
    eps=epsilon l+epsilonw
    dmxmdz=dmix(iax,iay)-thrmdz
    if(thrmdz.NE.0)then
    if(dmxmdz.GT.depth(iax,iay).AND.depth(iax,iay).GT.0.0)
  &thrmdz=dmxmdz-depth(iax,iay)+thrmdz
    endif
    thrmx = -1.0*akz_2*(thrmdz**1.0)
    dcthrm=EXP(thrmx)
    if(depth(iax,iay).LE.depo)then
    ps1(i)=(s(i)/((u**2+4.0*ruf*eps)**0.5))*
  &((depth(iax,iay)/depo)**akocs)
    else
    ps1(i)=(s(i)/((u**2+4.0*ruf*eps)**0.5))*
  &((depth(iax,iay)/depo)**(akocs/deep))
    endif
    if(v0.GE.0.0.AND.dy.GT.0.0)then
    etr=-1.0*ak1*((v0-beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(v0.GE.0.0.AND.dy.LT.0.0)then
    etr=-1.0*ak1*((v0+beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(v0.LT.0.0.AND.dy.LT.0.0)then
```

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    etr=-1.0*ak1*(-1.0*(v0-beta*v0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    if(v0.LT.0.0.AND.dy.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(v0+beta*v0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    av0=ABS(v0)
    if(dy.EQ.0.0)then
    etr=-1.0*ak1*(av0+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    endif
4531 continue
c**********SIO 2 Outfall Y-LOOP*******************
    if(ipt(i).EQ.3)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
    ps1b(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4532
    endif
    drc=z(i)*100.0
    wid=widsa*100.0
    qc=qsa*92903.0
    uvdir=ATAN2(ycur(iax,iay),xcur(iax,iay))
    ut0=qc/(drc*wid)
    ut=ut0/((((dx+0.00001)**2+(dy+0.00001)**2)**0.5)**akr_3)
    v0=ycur(iax,iay)+ut*SIN(uvdir)
    uave=(u_blend*ucur+u)/(u_blend+1)
    dxb=uave*per
    deltar=0.72*dxb*(2.5*ruf/dxb)**0.25
    fw=EXP(5.2*((2.5*ruf/dxb)**0.2)-6.0)
    shield=(uave**2/((rhos/rho-1.0)*g*ruf))*fw
    s(i)=srivy3
    ak1=aky1_3
    ak2=ak2_3
    epsilon_l=0.00035*ak2*(uave**0.68)*(shield**0.4)*deltar*g*per
    if(epsilon_l.EQ.0.0)epsilon_l=0.0000000001
```

```
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    eps=epsilon_l+epsilonw
    surx=-1.0*akz_3*(surdz**1.0)
    decsur=EXP(surx)
    if(dmix(iax,iay).GE.depth(iax,iay).AND.depth(iax,iay).GT.0.0)
  &dmix(iax,iay)=depth(iax,iay)
    if(dmix(iax,iay).LT.surdz)decsur=decsur**thermo
    ps1(i)=s(i)*decsur/((v0**2+4.0*ruf*eps)**0.5)
    if(v0.GE.0.0.AND.dy.GT.0.0)then
    etr=-1.0*ak1*((v0-beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(v0.GE.0.0.AND.dy.LT.0.0)then
    etr=-1.0*ak1*((v0+beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(v0.LT.0.0.AND.dy.LT.0.0)then
    etr=-1.0*ak1*(-1.0*(v0-beta*v0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    if(v0.LT.0.0.AND.dy.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(v0+beta*v0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    av0=ABS(v0)
    if(dy.EQ.0.0)then
    etr=-1.0*ak1*(av0+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    endif
4532 continue
c**********SIO 4 Y-LOOP********************
    if(ipt(i).EQ.4)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
    ps1b(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4533
    endif
    drc=z(i)*100.0
    wid=widtc*100.0
```

qc=qtc*92903.0

```
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     uvdir=ATAN2(ycur(iax,iay),xcur(iax,iay))
    ut0=qc/(drc*wid)
    ut=ut0/((((dx+0.00001)**2+(dy+0.00001)**2)**0.5)**akr_4)
     v0=ycur(iax,iay)+ut*SIN(uvdir)
     uave=(u_blend*ucur+u)/(u_blend+1)
     dxb=uave*per
    deltar=0.72*dxb*(2.5*ruf/dxb)**0.25
    fw=EXP(5.2*((2.5*ruf/dxb)**0.2)-6.0)
    shield=(uave**2/((rhos/rho-1.0)*g*ruf))*fw
    s(i)=srivy4
    ak1=aky1_4
    ak2=ak2 4
    epsilon_l=0.00035*ak2*(uave**0.68)*(shield**0.4)*deltar*g*per
    if(epsilon 1.EQ.0.0000000001)epsilon 1=0.0000000001
     eps=epsilon_l+epsilonw
    surx = -1.0*akz_4*(surdz**2.0)/eps
    decsur=EXP(surx)
    if(dmix(iax,iay).GE.depth(iax,iay).AND.depth(iax,iay).GT.0.0)
   &dmix(iax,iay)=depth(iax,iay)
    if(dmix(iax,iay).LT.surdz)decsur=decsur**thermo
    ps1(i)=s(i)*decsur/((v0**2+4.0*ruf*eps)**0.5)
    if(v0.GE.0.0.AND.dy.GT.0.0)then
    etr=-1.0*ak1*((v0-beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(v0.GE.0.0.AND.dy.LT.0.0)then
     etr=-1.0*ak1*((v0+beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(v0.LT.0.0.AND.dy.LT.0.0)then
     etr=-1.0*ak1*(-1.0*(v0-beta*v0)+((4.0*ruf*eps)**0.5))/
   \&(2.0*eps)
    endif
    if(v0.LT.0.0.AND.dy.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(v0+beta*v0)+((4.0*ruf*eps)**0.5))/
   \&(2.0*eps)
    endif
    av0=ABS(v0)
    if(dy.EQ.0.0)then
    etr=-1.0*ak1*(av0+((4.0*ruf*eps)**0.5))/
```

&(2.0*eps)
endif
endif
4533 continue

c

```
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the Neighborhood of ASBS 29 in La Jolla, California
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c**********SIO Offshore OUTFALL Y-LOOP**********************
    if(ipt(i).EQ.5)then
    if(depth(iax,iay).LT.10.0)then
    ps1(i)=0.0
    ps1b(i)=0.0
    etr=0.0
    etrb=0.0
    go to 4534
    endif
    if(ycur(iax,iay).LT.0.0)uwav=-1.0*uwav
    v0=ycur(iax,iay)+uwav
    s(i)=srivy5
    ak1=aky1_5
    ak2=ak2_5
    epsilon_l=0.00035*ak2*(u**0.68)*(shield**0.4)*delta*g*per
    if(epsilon_1.EQ.0.0000000001)epsilon_1=0.0000000001
    eps=epsilon_l+epsilonw
    ps1(i)=s(i)/((u**2+4.0*dl*eps)**0.5)
    if(v0.GE.0.0.AND.dy.GT.0.0)then
    etr=-1.0*ak1*((v0-beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(v0.GE.0.0.AND.dy.LT.0.0)then
    etr=-1.0*ak1*((v0+beta*v0)+((4.0*ruf*eps)**0.5))/(2.0*eps)
    endif
    if(v0.LT.0.0.AND.dy.LT.0.0)then
    etr=-1.0*ak1*(-1.0*(v0-beta*v0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    if(v0.LT.0.0.AND.dy.GT.0.0)then
    etr=-1.0*ak1*(-1.0*(v0+beta*v0)+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    av0=ABS(v0)
    if(dy.EQ.0.0)then
    etr=-1.0*ak1*(av0+((4.0*ruf*eps)**0.5))/
  \&(2.0*eps)
    endif
    endif
4534 continue
```

```
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c
1777
       continue
    ps3=etr*((dx**2)+(dy**2))**0.5
    ps3b=etrb*((dx**2)+(dy**2))**0.5
c***
    ps2=EXP(ps3)
    ps2b=EXP(ps3b)
    pny(iax,iay)=pny(iax,iay)+(ps1(i)*ps2)
    bry(iax,iay)=bry(iax,iay)+(ps1b(i)*ps2b)
    bryav(iax,iay)=bryav(iax,iay)+(ps1bav(i)*ps2b)
c
1850
      continue
       continue
1800
1799
       continue
c
c sum pnx and pny together
    do 2800 iax=1,imax
    do 2850 iay=1,jmax
     if(pnx(iax,iay).LE.0.00000001)pnx(iax,iay)=0.00000001
c
     if(pny(iax,iay).LE.0.00000001)pny(iax,iay)=0.00000001
c
     if(pnx(iax,iay).GE.1000000000.0)pnx(iax,iay)=1000000000.0
c
     if(pny(iax,iay).GE.1000000000.0)pny(iax,iay)=1000000000.0
c
    pn(iax,iay)=(pnx(iax,iay)+pny(iax,iay))/2.0
     pn_l(iax,iay)=LOG(pn(iax,iay))
c
    brn(iax,iay)=(brx(iax,iay)+bry(iax,iay))/2.0
    brnav(iax,iay)=(brxav(iax,iay)+bryav(iax,iay))/2.0
c
c
2850 continue
2800
      continue
c
    if(isal.EQ.1)then
c find maximum pnmax
    pnmax=0.0
    brnmax=0.0
    do 2900 iax=1,imax
    do 2950 iay=1,jmax
    if(pn(iax,iay).GT.pnmax)pnmax=pn(iax,iay)
    if(brn(iax,iay).GT.brnmax)brnmax=brn(iax,iay)
2950 continue
2900 continue
    else
```

```
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    brnmax=0.0
    do 2902 iax=1,imax
    do 2953 iay=1,jmax
    if(brn(iax,iay).GT.brnmax)brnmax=brn(iax,iay)
2953 continue
2902 continue
    pnmax=pnman
    endif
c
    if(ibrn.EQ.0)brnmax=brnman
    pnmx_l=LOG(pnmax)
    write(*,*)pnmax,brnmax
c*****SALINITY LOOP
    do 3100 iax=1,imax
    do 3150 iay=1,jmax
c*salinity at depth increment surdz below surface
     salsur(iax,iay)=salo*(1.0-((pn(iax,iay)/pnmax)**aksal))
    if(salsur(iax,iay).LT.0.0)salsur(iax,iay)=0.0
    if(depth(iax,iay).LE.0.0)salsur(iax,iay)=0.0
c**salinity at elevation botdz above the bottom
     salbrn(iax,iay)=salo*(1-(brn(iax,iay)/brnmax))+brine*
   &(brn(iax,iay)/brnmax)
c**depth averaged brine salinity
     sbrnav(iax,iay)=salo*(1-(brnav(iax,iay)/brnmax))+brine*
   &(brnav(iax,iay)/brnmax)
    if(iax.EQ.150.AND.iay.EQ.92)write(*,*)salbrn(iax,iay)
    if(iax.EQ.150.AND.iay.EQ.92)write(*,*)sbrnav(iax,iay)
c*salinity of sewage at depth increment thrmdz above thermocline
     saldep(iax,iay)=salo*(1.0-((pn(iax,iay)/pnmax)**aksal))
    if(saldep(iax,iay).LT.0.0)saldep(iax,iay)=0.0
    if(depth(iax,iay).LE.0.0)saldep(iax,iay)=0.0
c SIO 1 Inshore
    if(iax.EQ.174.AND.iay.EQ.94)write(*,*)iax,iay,pn(iax,iay),
   &salsur(iax,iay),salbrn(iax,iay),dmix(iax,iay)
c upper left (NW) corner of grid
    if(iax.EQ.1.AND.iay.EQ.1)write(*,*)iax,iay,pn(iax,iay),
   &salsur(iax,iay),salbrn(iax,iay),dmix(iax,iay)
c SIO_3 Inshore
    if(iax.EQ.173.AND.iay.EQ.96)write(*,*)iax,iay,pn(iax,iay),
   &salsur(iax,iay),salbrn(iax,iay),dmix(iax,iay)
c SIO 2
```

```
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    if(iax.EQ.193.AND.iay.EQ.100)write(*,*)iax,iay,pn(iax,iay),
   &salsur(iax,iay),salbrn(iax,iay),dmix(iax,iay)
c SIO 4 Outfall
    if(iax.EQ.204.AND.iay.EQ.109)write(*,*)iax,iay,pn(iax,iay),
   &salsur(iax,iay),salbrn(iax,iay),dmix(iax,iay)
3150 continue
3100 continue
C****SALINITY MAXIMUM AND MINIMUM LOOP
    salsmx=0.0
    saldmx=0.0
    salbmn=33.52
    do 3160 iax=1,imax
    do 3170 iay=1,jmax
    if(salsur(iax,iay).GT.salsmx)salsmx=salsur(iax,iay)
    if(saldep(iax,iay).GT.saldmx)saldmx=saldep(iax,iay)
    if(salbrn(iax,iay).LT.salbmn)salbmn=salbrn(iax,iay)
3170 continue
3160 continue
    write(*,*)salsmx,saldmx,salbmn
c****DILUTION LOOP
    do 3180 iax=1,imax
    do 3190 iay=1,jmax
c dilution of storm water sources at depth increment surdz below surface
     dilriv(iax,iay)=LOG10(salsmx/(salsmx-salsur(iax,iay)+back))
c**storm waterdilution at elevation botdz above the bottom
   dilbrn(iax,iay)=LOG10((rawbrn-salbmn)/
   &(salbrn(iax,iay)-salbmn+back))
c**depth averaged brine dilution
   dbrnav(iax,iay)=LOG10((rawbrn-salbmn)/
   &(sbrnav(iax,iay)-salbmn+back))
    if(iax.EQ.150.AND.iay.EQ.92)write(*,*)dilbrn(iax,iay)
    if(iax.EQ.150.AND.iay.EQ.92)write(*,*)dbrnav(iax,iay)
c dilution of storm water at depth increment thrmdz above thermocline
     dildep(iax,iay)=LOG10(saldmx/(saldmx-saldep(iax,iay)+back))
c TSS counts of storm water at depth thrmdz above thermocline
     col(iax,iay)=LOG10(coli)-dildep(iax,iay)
    if(col(iax,iay).LT.0.0)col(iax,iay)=0.0
c TSS counts of outfall source at depth surdz below surface
     colriv(iax,iay)=LOG10(coli)-dilriv(iax,iay)
    if(colriv(iax,iay).LT.0.0)colriv(iax,iay)=0.0
```

```
Hydrodynamic Modeling of Storm Drain Discharges in
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c
3190 continue
3180
       continue
c
c
     do 901 iay=1,jmax
    write(21,999)(pn(iax,iay),iax=1,imax)
901
      continue
     close(21)
c
     do 902 iay=1,jmax
     write(22,999)(pn_l(iax,iay),iax=1,imax)
902
      continue
c
     close(22)
c
     do 888 iax=1,imax
     do 887 iay=1,jmax
     if(depth(iax,iay).LT.surdz)salsur(iax,iay)=-2.0
    if(depth(iax,iay).LE.0.0)salsur(iax,iay)=-2.0
    if(depth(iax,iay).LT.botdz)salbrn(iax,iay)=-2.0
     if(depth(iax,iay).LE.0.0)salbrn(iax,iay)=-2.0
    if(depth(iax,iay).LE.0.0)dilbrn(iax,iay)=-2.0
     if(depth(iax,iay).LE.0.0)dildep(iax,iay)=-2.0
    if(depth(iax,iay).LE.0.0)saldep(iax,iay)=-2.0
     if(depth(iax,iay).LE.0.0)col(iax,iay)=-2.0
     if(depth(iax,iay).LE.0.0)colriv(iax,iay)=-2.0
    if(depth(iax,iay).LE.0.0)sbrnav(iax,iay)=-2.0
     if(depth(iax,iay).LE.0.0)dbrnav(iax,iay)=-2.0
      continue
887
888
      continue
     do 903 iay=1,jmax
     write(23,995)(salsur(iax,iay),iax=1,imax)
903
      continue
c
     close(23)
c
     do 905 iay=1,jmax
     write(25,995)(salbrn(iax,iay),iax=1,imax)
```

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905
      continue
c
     close(25)
c
c
     do 906 iay=1,jmax
     write(26,995)(dilbrn(iax,iay),iax=1,imax)
      continue
906
c
     close(26)
c
     do 904 iay=1,jmax
     write(24,999)(dilriv(iax,iay),iax=1,imax)
904
      continue
c
     close(24)
cc
     do 907 iay=1,jmax
     write(27,995)(saldep(iax,iay),iax=1,imax)
      continue
907
c
     close(27)
c
c
     do 908 iay=1,jmax
     write(28,995)(dildep(iax,iay),iax=1,imax)
908
      continue
c
     close(28)
c
c
     do 909 iay=1,jmax
     write(29,995)(col(iax,iay),iax=1,imax)
909
      continue
c
     close(29)
c
     do 910 iay=1,jmax
     write(30,995)(dmix(iax,iay),iax=1,imax)
910
      continue
c
     close(30)
c
```

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```
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     do 911 iay=1,jmax
     write(31,995)(colriv(iax,iay),iax=1,imax)
911
      continue
c
     close(31)
c
c
     do 912 iay=1,jmax
     write(32,995)(sbrnav(iax,iay),iax=1,imax)
912
      continue
c
     close(32)
c
c
     do 913 iay=1,jmax
     write(33,995)(dbrnav(iax,iay),iax=1,imax)
913
      continue
c
     close(33)
      format(200e12.3)
999
995
      format(200e15.6)
c
     stop
```

end

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APPENDIX G AMEC MONITORING DATA

Appendix G: AMEC Monitoring Data

Station Code	Sample Date	Time	Units	Result Collection Device Name	Agency
906SDL062OD	03/Nov/11	8:45 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	9:00 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	9:15 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	9:30 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	9:45 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	10:00 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	10:15 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	10:30 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	10:45 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	11:00 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	11:15 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	11:30 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	11:45 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	12:00 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	12:15 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	12:30 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	12:45 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	13:00 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	13:15 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	13:30 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	13:45 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	14:00 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	14:15 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	14:30 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	14:45 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	15:00 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	15:15 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	15:30 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	15:45 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	16:00 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	16:15 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	16:30 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	16:45 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	17:00 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	17:15 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	17:30 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	17:45 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	18:00 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	18:15 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	18:30 PST	inches	Nach tipping bucket rain gauge Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	18:45 PST	inches	Nach tipping bucket rain gauge Hach tipping bucket rain gauge	AMEC
		19:00 PST		D Hach tipping bucket rain gauge	
906SDL062OD	03/Nov/11	19:00 PST	inches		AMEC
906SDL062OD	03/Nov/11	19:15 PST 19:30 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11		inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	19:45 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	20:00 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	20:15 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	20:30 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	20:45 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	21:00 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	21:15 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	21:30 PST	inches	0 Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	21:45 PST	inches	Hach tipping bucket rain gauge	AMEC
906SDL062OD	03/Nov/11	22:00 PST	inches	0 Hach tipping bucket rain gauge	AMEC

Otation Code	Cample Date	T: A	1	Danult	Callastian Davisa Nama
Station Code	Sample Date	Time Agency	Location	Result	Collection Device Name
906SDL062OD	20/Nov/11	14:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	14:15 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	14:30 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	14:45 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	15:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	15:15 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	15:30 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	15:45 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	16:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	16:15 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	16:30 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	16:45 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	17:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	17:15 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	17:30 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	17:45 AMEC	Pipe	1.29 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	18:00 AMEC	Pipe	1.76 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	18:15 AMEC	Pipe	5.48 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	18:30 AMEC	Pipe	8.77 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	18:45 AMEC	Pipe	9.44 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	19:00 AMEC	Pipe	10.44 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	19:15 AMEC	Pipe	9.33 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	19:30 AMEC	Pipe	6.54 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	19:45 AMEC	Pipe	4.83 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	20:00 AMEC	Pipe	4.45 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	20:15 AMEC	Pipe	4.05 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	20:30 AMEC	Pipe	3.62 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	20:45 AMEC	Pipe	3.7 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	21:00 AMEC	Pipe	4.5 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	21:15 AMEC	Pipe	3.67 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	21:30 AMEC	Pipe	3.52 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	21:45 AMEC	Pipe	3.39 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	22:00 AMEC	Pipe	3.14 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	22:15 AMEC	Pipe	2.63 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	22:30 AMEC	Pipe	2.09 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	22:45 AMEC	Pipe	1.66 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	23:00 AMEC	Pipe	1.39 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	23:15 AMEC	Pipe	1.32 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	23:30 AMEC	Pipe	1.62 cfs	Hach 950 Flow Meter
906SDL062OD	20/Nov/11	23:45 AMEC	Pipe	4.59 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	0:00 AMEC	Pipe	6.12 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	0:15 AMEC	Pipe	3.99 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	0:30 AMEC	Pipe	3.16 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	0:45 AMEC	Pipe	3.12 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	1:00 AMEC	Pipe	2.75 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	1:15 AMEC	Pipe	2.15 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	1:30 AMEC	Pipe	1.59 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	1:45 AMEC	Pipe	1.11 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	2:00 AMEC	Pipe	0.761 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	2:15 AMEC	Pipe	0.53 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	2:30 AMEC	Pipe	0.339 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	2:45 AMEC	Pipe	0.249 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	3:00 AMEC	Pipe	0.204 cfs	Hach 950 Flow Meter
906SDL062OD	21/Nov/11	3:15 AMEC	Pipe	0.146 cfs	Hach 950 Flow Meter

Station Code	Sample Date	Time Agency	Location	Result Units	Collection Device Name
906SDL062OD	07/Feb/12	14:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	14:15 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	14:30 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	14:45 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	15:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	15:15 AMEC	Pipe	0.01 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	15:30 AMEC	Pipe	0.095 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	15:45 AMEC	Pipe	0.28 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	16:00 AMEC	Pipe	1.9 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	16:15 AMEC	Pipe	2.2 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	16:30 AMEC	Pipe	1.33 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	16:45 AMEC	Pipe	2.23 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	17:00 AMEC	Pipe	4.44 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	17:15 AMEC	Pipe	6.56 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	17:30 AMEC	Pipe	5.14 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	17:45 AMEC	Pipe	2.47 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	18:00 AMEC	Pipe	2.04 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	18:15 AMEC	Pipe	1.02 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	18:30 AMEC	Pipe	0.293 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	18:45 AMEC	Pipe	0.047 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	19:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	19:15 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	19:30 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	19:45 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	20:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	20:15 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	20:30 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	20:45 AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL062OD	07/Feb/12	21:00 AMEC	Pipe	0 cfs	Hach 950 Flow Meter

Station Code	Sample Date	Time Age	ncy Location	Result	Units	Collection Device Name
906SDL062OD	16/Mar/12	17:45 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	18:00 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	18:15 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	18:30 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	18:45 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	19:00 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	19:15 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	19:30 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	19:45 AME		0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	20:00 AME		0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	20:15 AME	C Pipe	0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	20:30 AME		0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	20:45 AME		0	cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	21:00 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	21:15 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	21:30 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	21:45 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	22:00 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	22:15 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	22:30 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	22:45 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	23:00 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	23:15 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	23:30 AME			cfs	Hach 950 Flow Meter
906SDL062OD	16/Mar/12	23:45 AME			cfs	Hach 950 Flow Meter
906SDL062OD	17/Mar/12	0:00 AME			cfs	Hach 950 Flow Meter
906SDL062OD	17/Mar/12	0:15 AME			cfs	Hach 950 Flow Meter
906SDL062OD	17/Mar/12	0:30 AME			cfs	Hach 950 Flow Meter
906SDL062OD	17/Mar/12	0:45 AME			cfs	Hach 950 Flow Meter
906SDL062OD	17/Mar/12	1:00 AME			cfs	Hach 950 Flow Meter
906SDL062OD	17/Mar/12	1:15 AME		0.053		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	1:30 AME		0.428		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	1:45 AME		0.252		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	2:00 AME		0.065		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	2:15 AME		0.011		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	2:30 AME		0.031		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	2:45 AME		0.031		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	3:00 AME		0.017		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	3:15 AME		0.017		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	3:30 AME		0.014		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	3:45 AME		0.01		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	4:00 AME		0.01		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	4:15 AME		0.225		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	4:30 AME		0.44		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	4:45 AME		0.361		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	5:00 AME		0.493		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	5:15 AME		1.74		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	5:30 AME		1.68		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	5:45 AME		2.23		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	6:00 AME		2.23		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	6:15 AME		4.25		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	6:30 AME		1.62		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	6:45 AME		1.78		Hach 950 Flow Meter
				2.13		Hach 950 Flow Meter
906SDL062OD	17/Mar/12	7:00 AME	- ⊢ripe	2.13	UIS	I IACIT 300 FIOW MELEI

Station Code	Sample Date	Time	Agency	Location	Result	Units	Collection Device Name
906SDL157OD	20/Nov/11		AMEC	Pipe		cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	9:30	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	9:45	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	10:00	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	10:15	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	10:30	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	10:45	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	11:00	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	11:15	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	11:45	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	12:15	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe		cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	13:00	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	13:15	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	13:45	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	14:00	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	14:15	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	14:30	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	14:45	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	15:00	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	15:15	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	15:30	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	16:00	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11	16:30	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe		cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe		cfs	Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.003		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.56		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	1.26		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	1.25		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	2.73		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	2.96		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	3.22		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	2.26		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	1.76		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	1.49		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.96		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.94		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.92		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	1.72		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	1.68		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	1.08		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.93		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.97		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.95		Hach 950 Flow Meter
906SDL157OD	20/Nov/11		AMEC	Pipe	0.84		Hach 950 Flow Meter
906SDL157OD	20/Nov/11	22:30	AMEC	Pipe	0.75	cts	Hach 950 Flow Meter

AMEC, Environment & Infrastructure, Inc. Hydrodynamic Modeling of Storm Drain Discharges in the Neighborhood of ASBS 29 in La Jolla, California Revised: 13 March 2013

Ctation Call	Cample Detal	Time	A	1 41	Desuit	11!4-	Callantian Davida Nama
Station Code		Time	Agency	Location	Result	Units	Collection Device Name
906SDL157OD			AMEC	Pipe		cfs	Hach 950 Flow Meter
906SDL157OD			AMEC	Pipe		cfs	Hach 950 Flow Meter
906SDL157OD			AMEC	Pipe		cfs	Hach 950 Flow Meter
906SDL157OD			AMEC	Pipe			Hach 950 Flow Meter
906SDL157OD			AMEC	Pipe	0.001		Hach 950 Flow Meter
906SDL157OD	07/Feb/12	15:15	AMEC	Pipe	0.11		Hach 950 Flow Meter
906SDL157OD	07/Feb/12	15:30	AMEC	Pipe	0.13	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	15:45	AMEC	Pipe	0.16	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	16:00	AMEC	Pipe	0.45	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	16:15	AMEC	Pipe	1.31	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	16:30	AMEC	Pipe	1.34	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	16:45	AMEC	Pipe	2.52	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	17:00	AMEC	Pipe	2.88	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	17:15	AMEC	Pipe	3.48	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	17:30	AMEC	Pipe	3.15	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	17:45	AMEC	Pipe	3.17	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	18:00	AMEC	Pipe	2.73	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	18:15	AMEC	Pipe	2.07	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	18:30	AMEC	Pipe	1.64	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	18:45	AMEC	Pipe	1.27	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	19:00	AMEC	Pipe	0.77	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	19:15	AMEC	Pipe	0.51	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	19:30	AMEC	Pipe	0.24	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	19:45	AMEC	Pipe	0.1	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	20:00	AMEC	Pipe	0.022	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	20:15	AMEC	Pipe .	0.003	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	20:30	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	20:45	AMEC	Pipe	0	cfs	Hach 950 Flow Meter
906SDL157OD	07/Feb/12	21:00	AMEC	Pipe		cfs	Hach 950 Flow Meter

Station Code	Sampl eDate	Time	Agency	Location	Result Units	Collection Device Name
906SDL157OD	16/Mar/12	17:45	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12	18:00	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12	18:15	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC		0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
			AMEC	Pipe		Hach 950 Flow Meter
906SDL157OD	16/Mar/12			Pipe	0 cfs	
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	16/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	3:30	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	3:45	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	4:00	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	4:15	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	4:30	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	4:45	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	5:00	AMEC	Pipe	0 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	5:15	AMEC	Pipe	0.001 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	5:30	AMEC	Pipe	0.045 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	5:45	AMEC	Pipe	0.19 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12	6:00	AMEC	Pipe	0.51 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0.9 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	1.14 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0.78 cfs	Hach 950 Flow Meter
906SDL157OD	17/Mar/12		AMEC	Pipe	0.39 cfs	Hach 950 Flow Meter

						TSS	TSS Load
Station Code	Date	Time	Result		Units	(mg/L)	(kg/min)
906SDL062OD	20/Nov/11	14:00			cfs	260	
906SDL062OD	20/Nov/11	14:15			cfs	260	
906SDL062OD	20/Nov/11	14:30			cfs	260	
906SDL062OD	20/Nov/11	14:45			cfs	260	
906SDL062OD	20/Nov/11	15:00			cfs	260	
906SDL062OD	20/Nov/11	15:15			cfs	260	
906SDL062OD	20/Nov/11	15:30			cfs	260	
906SDL062OD	20/Nov/11	15:45			cfs	260	
906SDL062OD	20/Nov/11	16:00			cfs	260	
906SDL062OD	20/Nov/11	16:15			cfs	260	
906SDL062OD	20/Nov/11	16:30			cfs	260	
906SDL062OD	20/Nov/11	16:45			cfs	260	
906SDL062OD	20/Nov/11	17:00			cfs	260	
906SDL062OD	20/Nov/11	17:15			cfs	260	
906SDL062OD	20/Nov/11	17:30			cfs	260	
906SDL062OD	20/Nov/11	17:45		1.29		260	
906SDL062OD	20/Nov/11	18:00		1.76		260	
906SDL062OD	20/Nov/11	18:15		5.48		260	
906SDL062OD	20/Nov/11	18:30		8.77		260	
906SDL062OD	20/Nov/11	18:45		9.44		260	
906SDL062OD	20/Nov/11	19:00		10.44		260	
906SDL062OD	20/Nov/11	19:15		9.33		260	
906SDL062OD	20/Nov/11	19:30		6.54		260	
906SDL062OD	20/Nov/11	19:45		4.83		260	
906SDL062OD	20/Nov/11	20:00		4.45		260	
906SDL062OD	20/Nov/11	20:15		4.05		260	
906SDL062OD	20/Nov/11	20:30		3.62		260	
906SDL062OD	20/Nov/11	20:45		3.7		260	
906SDL062OD	20/Nov/11	21:00		4.5		260	
906SDL062OD	20/Nov/11	21:15		3.67		260	
906SDL062OD	20/Nov/11	21:30		3.52		260	
906SDL062OD	20/Nov/11 20/Nov/11	21:45		3.39		260	
906SDL062OD		22:00		3.14		260	
906SDL062OD 906SDL062OD	20/Nov/11	22:15		2.63		260	
906SDL062OD	20/Nov/11 20/Nov/11	22:30		2.09		260	
	20/Nov/11 20/Nov/11	22:45		1.66		260 260	
906SDL062OD 906SDL062OD	20/Nov/11 20/Nov/11	23:00 23:15		1.39 1.32		260	
906SDL062OD	20/Nov/11 20/Nov/11	23:30		1.62		260	
906SDL062OD	20/Nov/11 20/Nov/11	23:45		4.59		260	
906SDL062OD	20/Nov/11 21/Nov/11	0:00		6.12		260	
906SDL062OD	21/Nov/11 21/Nov/11	0:00		3.99		260	
906SDL062OD	21/Nov/11 21/Nov/11	0:13		3.16		260	
906SDL062OD	21/Nov/11 21/Nov/11	0:30		3.12		260	
906SDL062OD	21/Nov/11 21/Nov/11	1:00		2.75		260	
906SDL062OD	21/Nov/11 21/Nov/11	1:15		2.15		260	
906SDL062OD	21/Nov/11 21/Nov/11	1:30		1.59		260	
906SDL062OD	21/Nov/11 21/Nov/11	1:45		1.11		260	
906SDL062OD	21/Nov/11 21/Nov/11	2:00		0.761		260	
906SDL062OD	21/Nov/11 21/Nov/11	2:15		0.761		260	
906SDL062OD	21/Nov/11 21/Nov/11	2:30		0.339		260	
906SDL062OD	21/Nov/11 21/Nov/11	2:45		0.249		260	
906SDL062OD	21/Nov/11 21/Nov/11	3:00		0.204		260	
	,	0.00		5.20 T		_00	3.0001

Station Code	Date	Time	Result	Units	TSS (mg/L)	TSS Load (kg/min)
906SDL062OD	07/Feb/12	14:00	0	cfs	130	0.0000
906SDL062OD	07/Feb/12	14:15	0	cfs	130	0.0000
906SDL062OD	07/Feb/12	14:30	0	cfs	130	0.0000
906SDL062OD	07/Feb/12	14:45	0	cfs	130	0.0000
906SDL062OD	07/Feb/12	15:00	0	cfs	130	0.0000
906SDL062OD	07/Feb/12	15:15	0.01	cfs	130	0.0022
906SDL062OD	07/Feb/12	15:30	0.095	cfs	130	0.0210
906SDL062OD	07/Feb/12	15:45	0.28	cfs	130	0.0618
906SDL062OD	07/Feb/12	16:00	1.9	cfs	130	0.4197
906SDL062OD	07/Feb/12	16:15	2.2	cfs	130	0.4859
906SDL062OD	07/Feb/12	16:30	1.33		130	0.2938
906SDL062OD	07/Feb/12	16:45	2.23	cfs	130	0.4925
906SDL062OD	07/Feb/12	17:00	4.44	cfs	130	0.9807
906SDL062OD	07/Feb/12	17:15	6.56	cfs	130	1.4489
906SDL062OD	07/Feb/12	17:30	5.14		130	
906SDL062OD	07/Feb/12	17:45	2.47	cfs	130	0.5456
906SDL062OD	07/Feb/12	18:00	2.04		130	0.4506
906SDL062OD	07/Feb/12	18:15	1.02	cfs	130	0.2253
906SDL062OD	07/Feb/12	18:30	0.293		130	
906SDL062OD	07/Feb/12	18:45	0.047	cfs	130	
906SDL062OD	07/Feb/12	19:00		cfs	130	
906SDL062OD	07/Feb/12	19:15	0	cfs	130	0.0000
906SDL062OD	07/Feb/12	19:30		cfs	130	
906SDL062OD	07/Feb/12	19:45		cfs	130	
906SDL062OD	07/Feb/12	20:00		cfs	130	
906SDL062OD	07/Feb/12	20:15		cfs	130	
906SDL062OD	07/Feb/12	20:30	0	cfs	130	
906SDL062OD	07/Feb/12	20:45		cfs	130	
906SDL062OD	07/Feb/12	21:00	0	cfs	130	
						99.57
			27,050 Total Flow (cf)		130	
				Total Load (kg/event)		

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Station Code	Date	Time	Result	Units	TSS (mg/L)	TSS Load (kg/min)
906SDL062OD	16/Mar/12	17:45		cfs	81	0.0000
906SDL062OD	16/Mar/12	18:00		cfs	81	0.0000
906SDL062OD	16/Mar/12	18:15		cfs	81	0.0000
906SDL062OD	16/Mar/12	18:30		cfs	81	0.0000
906SDL062OD	16/Mar/12	18:45		cfs	81	0.0000
906SDL062OD	16/Mar/12	19:00		cfs	81	0.0000
906SDL062OD	16/Mar/12	19:15		cfs	81	0.0000
906SDL062OD	16/Mar/12	19:30		cfs	81	0.0000
906SDL062OD	16/Mar/12	19:45		cfs	81	0.0000
906SDL062OD	16/Mar/12	20:00		cfs	81	0.0000
906SDL062OD	16/Mar/12	20:15		cfs	81	0.0000
906SDL062OD	16/Mar/12	20:30		cfs	81	0.0000
906SDL062OD	16/Mar/12	20:45		cfs	81	0.0000
906SDL062OD	16/Mar/12	21:00		cfs	81	0.0000
906SDL062OD	16/Mar/12	21:15		cfs	81	0.0000
906SDL062OD	16/Mar/12	21:30		cfs	81	0.0000
906SDL062OD	16/Mar/12	21:45		cfs	81	0.0000
906SDL062OD	16/Mar/12	22:00		cfs	81	0.0000
906SDL062OD	16/Mar/12	22:15		cfs	81	0.0000
906SDL062OD	16/Mar/12	22:30		cfs	81	0.0000
906SDL062OD	16/Mar/12	22:45		cfs	81	0.0000
906SDL062OD	16/Mar/12	23:00		cfs	81	0.0000
906SDL062OD	16/Mar/12	23:15		cfs	81	0.0000
906SDL062OD	16/Mar/12	23:30		cfs	81	0.0000
906SDL062OD	16/Mar/12	23:45		cfs	81	0.0000
906SDL062OD	17/Mar/12	0:00		cfs	81	0.0000
906SDL062OD	17/Mar/12	0:15		cfs	81	0.0000
906SDL062OD	17/Mar/12	0:30		cfs	81	0.0000
906SDL062OD	17/Mar/12	0:45		cfs	81	0.0000
906SDL062OD	17/Mar/12	1:00		cfs	81	0.0000
906SDL062OD	17/Mar/12	1:15	0.053	cfs	81	0.0073
906SDL062OD	17/Mar/12	1:30	0.428		81	0.0589
906SDL062OD	17/Mar/12	1:45	0.252	cfs	81	0.0347
906SDL062OD	17/Mar/12	2:00	0.065	cfs	81	0.0089
906SDL062OD	17/Mar/12	2:15	0.011	cfs	81	0.0015
906SDL062OD	17/Mar/12	2:30	0.031	cfs	81	0.0043
906SDL062OD	17/Mar/12	2:45	0.031	cfs	81	0.0043
906SDL062OD	17/Mar/12	3:00	0.017	cfs	81	0.0023
906SDL062OD	17/Mar/12	3:15	0.017	cfs	81	0.0023
906SDL062OD	17/Mar/12	3:30	0.014	cfs	81	0.0019
906SDL062OD	17/Mar/12	3:45	0.01		81	0.0014
906SDL062OD	17/Mar/12	4:00	0.01	cfs	81	0.0014
906SDL062OD	17/Mar/12	4:15	0.225		81	0.0310
906SDL062OD	17/Mar/12	4:30	0.44	cfs	81	0.0606
906SDL062OD	17/Mar/12	4:45	0.361	cfs	81	0.0497
906SDL062OD	17/Mar/12	5:00	0.493	cfs	81	0.0678
906SDL062OD	17/Mar/12	5:15	1.74	cfs	81	0.2395
906SDL062OD	17/Mar/12	5:30	1.68		81	0.2312
906SDL062OD	17/Mar/12	5:45	2.23		81	0.3069
906SDL062OD	17/Mar/12	6:00	2.18		81	0.3000
906SDL062OD	17/Mar/12	6:15	4.25		81	
906SDL062OD	17/Mar/12	6:30	1.62		81	0.2229
906SDL062OD	17/Mar/12	6:45	1.78		81	0.2450
906SDL062OD	17/Mar/12	7:00	2.13	cfs	81	0.2931

Station Code	Date	Time	Result	Units	TSS (mg/L)	TSS Load (kg/min)	
906SDL157OD	20/Nov/11	9:15	0	cfs] 1 00 (g, <u>_</u>)	340	0.0000
906SDL157OD	20/Nov/11	9:30		cfs		340	0.0000
906SDL157OD	20/Nov/11	9:45		cfs		340	0.0000
906SDL157OD	20/Nov/11	10:00		cfs		340	0.0000
906SDL157OD	20/Nov/11	10:15		cfs		340	0.0000
906SDL157OD	20/Nov/11	10:30		cfs		340	0.0000
906SDL157OD	20/Nov/11	10:45		cfs		340	0.0000
906SDL157OD	20/Nov/11	11:00		cfs		340	0.0000
906SDL157OD	20/Nov/11	11:15		cfs		340	0.0000
906SDL157OD	20/Nov/11	11:30		cfs		340	0.0000
906SDL157OD	20/Nov/11	11:45		cfs		340	0.0000
906SDL157OD	20/Nov/11	12:00		cfs		340	0.0000
906SDL157OD	20/Nov/11	12:15		cfs		340	0.0000
906SDL157OD	20/Nov/11	12:30		cfs		340	0.0000
906SDL157OD	20/Nov/11	12:45		cfs		340	0.0000
906SDL157OD	20/Nov/11	13:00		cfs		340	0.0000
906SDL157OD	20/Nov/11	13:15		cfs		340	0.0000
906SDL157OD	20/Nov/11	13:30		cfs		340	0.0000
906SDL157OD	20/Nov/11	13:45		cfs		340	0.0000
906SDL157OD	20/Nov/11	14:00		cfs		340	0.0000
906SDL157OD	20/Nov/11	14:15		cfs		340	0.0000
906SDL157OD	20/Nov/11	14:30		cfs		340	0.0000
906SDL157OD	20/Nov/11	14:45		cfs		340	0.0000
906SDL157OD	20/Nov/11	15:00		cfs		340	0.0000
906SDL157OD	20/Nov/11	15:15		cfs		340	0.0000
906SDL157OD	20/Nov/11	15:30		cfs		340	0.0000
906SDL157OD	20/Nov/11	15:45		cfs		340	0.0000
906SDL157OD	20/Nov/11	16:00		cfs		340	0.0000
906SDL157OD	20/Nov/11	16:15		cfs		340	0.0000
906SDL157OD	20/Nov/11	16:30		cfs		340	0.0000
906SDL157OD	20/Nov/11	16:45		cfs		340	0.0000
906SDL157OD	20/Nov/11	17:00		cfs		340	0.0000
906SDL157OD	20/Nov/11	17:15		cfs		340	0.0000
906SDL157OD	20/Nov/11	17:30	0.003			340	0.0017
906SDL157OD	20/Nov/11	17:45	0.56			340	0.3235
906SDL157OD	20/Nov/11	18:00	1.26			340	0.7279
906SDL157OD	20/Nov/11	18:15	1.25	-		340	0.7221
906SDL157OD	20/Nov/11	18:30	2.73			340	1.5770
906SDL157OD	20/Nov/11	18:45	2.96			340	1.7099
906SDL157OD	20/Nov/11	19:00	3.22			340	1.8601
906SDL157OD	20/Nov/11	19:15	2.26			340	1.3055
906SDL157OD	20/Nov/11	19:30	1.76			340	1.0167
906SDL157OD	20/Nov/11	19:45	1.49			340	0.8607
906SDL157OD	20/Nov/11	20:00	0.96			340	0.5546
906SDL157OD	20/Nov/11	20:15	0.94			340	0.5430
906SDL157OD	20/Nov/11	20:30	0.92			340	0.5315
906SDL157OD	20/Nov/11	20:45	1.72			340	0.9936
906SDL157OD	20/Nov/11	21:00	1.68			340	0.9705
906SDL157OD	20/Nov/11	21:15	1.08		1	340	0.6239
906SDL157OD	20/Nov/11	21:30	0.93			340	0.5372
906SDL157OD	20/Nov/11	21:45	0.97			340	0.5603
906SDL157OD	20/Nov/11	22:00	0.95			340	0.5488
906SDL157OD	20/Nov/11	22:15	0.84			340	0.4852
30000010100	_5/145V/11	10	0.07	5.5	I	3.10	J. 7002

Station Code	Date	Time	Result	Units	TSS (mg/L)	TSS Load (kg/min)
906SDL157OD	07/Feb/12	14:00	0	cfs	230	0.0000
906SDL157OD	07/Feb/12	14:15	0	cfs	230	0.0000
906SDL157OD	07/Feb/12	14:30	0	cfs	230	0.0000
906SDL157OD	07/Feb/12	14:45	0	cfs	230	0.0000
906SDL157OD	07/Feb/12	15:00	0.001	cfs	230	0.0004
906SDL157OD	07/Feb/12	15:15	0.11	cfs	230	0.0430
906SDL157OD	07/Feb/12	15:30	0.13	cfs	230	0.0508
906SDL157OD	07/Feb/12	15:45	0.16	cfs	230	0.0625
906SDL157OD	07/Feb/12	16:00	0.45	cfs	230	0.1758
906SDL157OD	07/Feb/12	16:15	1.31	cfs	230	0.5119
906SDL157OD	07/Feb/12	16:30	1.34	cfs	230	0.5236
906SDL157OD	07/Feb/12	16:45	2.52	cfs	230	0.9847
906SDL157OD	07/Feb/12	17:00	2.88	cfs	230	1.1254
906SDL157OD	07/Feb/12	17:15	3.48	cfs	230	1.3599
906SDL157OD	07/Feb/12	17:30	3.15	cfs	230	
906SDL157OD	07/Feb/12	17:45	3.17	cfs	230	
906SDL157OD	07/Feb/12	18:00	2.73	cfs	230	
906SDL157OD	07/Feb/12	18:15	2.07	cfs	230	
906SDL157OD	07/Feb/12	18:30	1.64	cfs	230	
906SDL157OD	07/Feb/12	18:45	1.27	cfs	230	
906SDL157OD	07/Feb/12	19:00	0.77		230	
906SDL157OD	07/Feb/12	19:15	0.51	cfs	230	0.1993
906SDL157OD	07/Feb/12	19:30	0.24		230	
906SDL157OD	07/Feb/12	19:45		cfs	230	
906SDL157OD	07/Feb/12	20:00	0.022		230	
906SDL157OD	07/Feb/12	20:15	0.003	cfs	230	
906SDL157OD	07/Feb/12	20:30		cfs	230	
906SDL157OD	07/Feb/12	20:45		cfs	230	
906SDL157OD	07/Feb/12	21:00	0	cfs	230	
						164.45
25,250				230		
		Total Flow (cf)			Total Load (kg/event)	

Station Code	Date	Time	Result Units	TSS (mg/L)	TSS Load (kg/min)
906SDL157OD	16/Mar/12	17:45	0 cfs	170	0.0000
906SDL157OD	16/Mar/12	18:00	0 cfs	170	0.0000
906SDL157OD	16/Mar/12	18:15	0 cfs	170	0.0000
906SDL157OD	16/Mar/12	18:30	0 cfs	170	0.0000
906SDL157OD	16/Mar/12	18:45	0 cfs	170	0.0000
906SDL157OD	16/Mar/12	19:00	0 cfs	170	0.0000
906SDL157OD	16/Mar/12	19:15	0 cfs	170	
906SDL157OD	16/Mar/12	19:30	0 cfs	170	
906SDL157OD	16/Mar/12	19:45	0 cfs	170	
906SDL157OD	16/Mar/12	20:00	0 cfs	170	
906SDL157OD	16/Mar/12	20:15	0 cfs	170	
906SDL157OD	16/Mar/12	20:30	0 cfs	170	
906SDL157OD	16/Mar/12	20:45	0 cfs	170	
906SDL157OD	16/Mar/12	21:00	0 cfs	170	
906SDL157OD	16/Mar/12	21:15	0 cfs	170	
906SDL157OD	16/Mar/12	21:30	0 cfs	170	
906SDL157OD	16/Mar/12	21:45	0 cfs	170	
906SDL157OD	16/Mar/12	22:00	0 cfs	170	
906SDL157OD	16/Mar/12	22:15	0 cfs	170	
906SDL157OD	16/Mar/12	22:30	0 cfs	170	
906SDL157OD	16/Mar/12	22:45	0 cfs	170	
906SDL157OD	16/Mar/12	23:00	0 cfs	170	
906SDL157OD	16/Mar/12	23:15	0 cfs	170	
906SDL157OD	16/Mar/12	23:30	0 cfs	170	
906SDL157OD	16/Mar/12	23:45	0 cfs	170	
906SDL157OD	17/Mar/12	0:00	0 cfs	170	
906SDL157OD	17/Mar/12	0:15	0 cfs	170	
906SDL157OD	17/Mar/12	0:30	0 cfs	170	
906SDL157OD	17/Mar/12	0:45	0 cfs	170	
906SDL157OD	17/Mar/12	1:00	0 cfs	170	
906SDL157OD	17/Mar/12	1:15	0 cfs	170	
906SDL157OD	17/Mar/12	1:30	0 cfs	170	
906SDL157OD	17/Mar/12	1:45	0 cfs	170	
906SDL157OD	17/Mar/12	2:00	0 cfs	170	
906SDL157OD	17/Mar/12	2:15	0 cfs	170	
906SDL157OD	17/Mar/12	2:30	0 cfs	170	
906SDL157OD	17/Mar/12	2:45	0 cfs	170	0.0000
906SDL157OD	17/Mar/12	3:00	0 cfs	170	
906SDL157OD	17/Mar/12	3:15	0 cfs	170	
906SDL157OD	17/Mar/12	3:30	0 cfs	170	
906SDL157OD	17/Mar/12	3:45	0 cfs	170	
906SDL157OD	17/Mar/12	4:00	0 cfs	170	
906SDL157OD	17/Mar/12	4:15	0 cfs	170	
906SDL157OD	17/Mar/12	4:30	0 cfs	170	
906SDL157OD	17/Mar/12	4:45	0 cfs	170	
906SDL157OD	17/Mar/12	5:00	0 cfs	170	
906SDL157OD	17/Mar/12	5:15	0.001 cfs	170	
906SDL157OD	17/Mar/12	5:30	0.045 cfs	170	
906SDL157OD	17/Mar/12	5:45	0.19 cfs	170	
906SDL157OD	17/Mar/12	6:00	0.51 cfs	170	
906SDL157OD	17/Mar/12	6:15	0.9 cfs	170	
906SDL157OD	17/Mar/12	6:30	1.14 cfs	170	
906SDL157OD	17/Mar/12	6:45	0.78 cfs	170	
906SDL157OD	17/Mar/12	7:00	0.39 cfs	170	
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